



Bicester Office Park

June 2018 ES Addendum

Trium Environmental Consulting LLP
68-85 Tabernacle Street
London
EC2A 4BD
+44 (0) 20 3887 7118
hello@triumenv.co.uk
www.triumenvironmental.co.uk

Project Reference: TEC001

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ES Addendum

Introduction

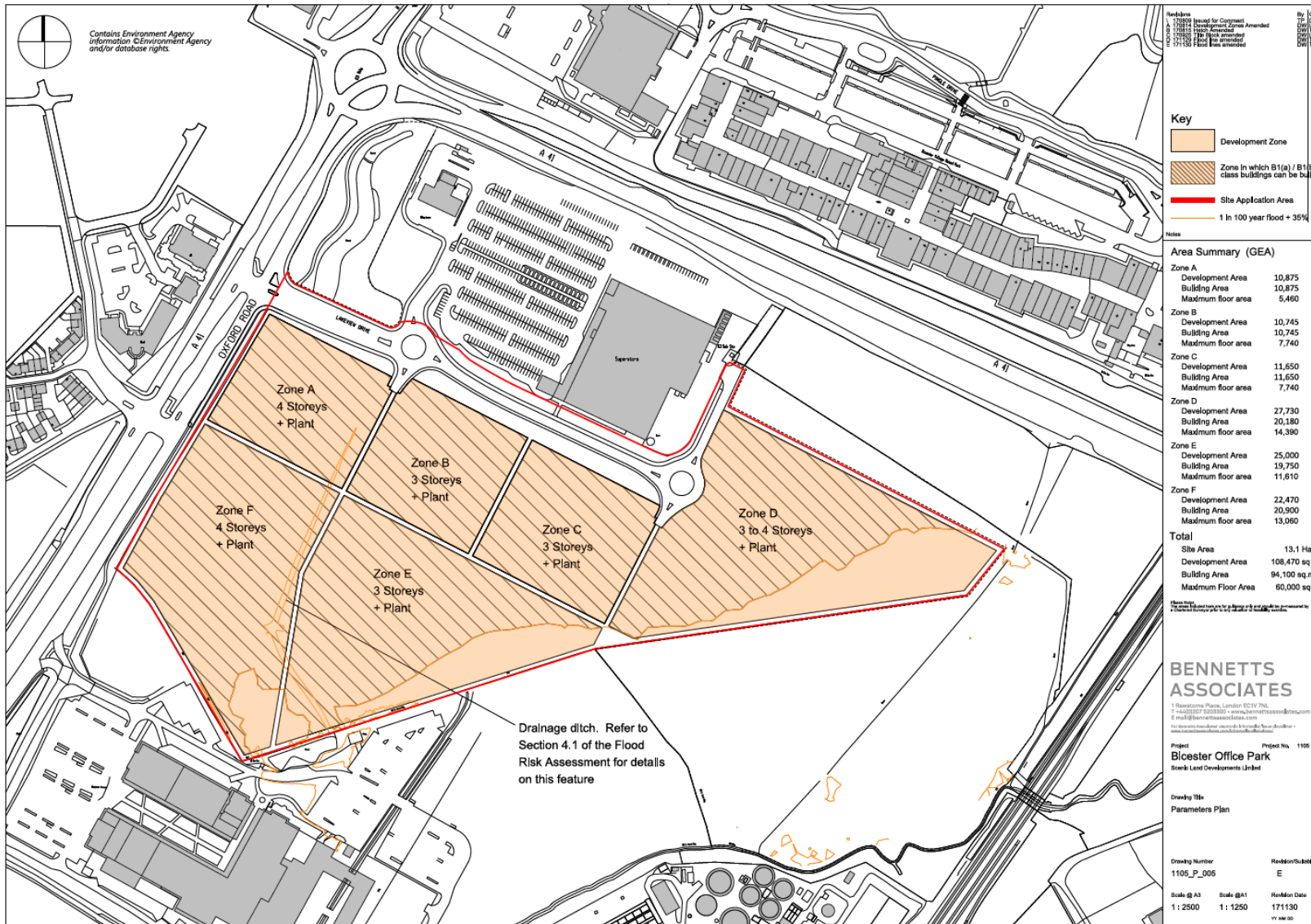
1. In September 2017, Scenic Land Developments Ltd ('the Applicant') applied to Cherwell District Council (CDC) for outline planning permission for the construction of a residential led mixed-use development (reference number 17/02534/OUT). This application included an Environmental Statement¹ prepared by Trium Environmental Consulting LLP (hereafter referred to as the Original ES) which was produced in accordance with the Town and Country (Environmental Impact Assessment) (England and Wales) Regulations 2011 (the EIA Regulations) as amended in 2015² ('the EIA Regulations') (Ref. 1).
2. This ES Addendum prepared by Trium Environmental Consulting LLP (hereafter referred to as the 'June 2018 ES Addendum') has been prepared to accompany Original ES as a result of some minor amendments to the parameters plan, further assessment and consultation comments from CDC since the Original ES. Its purpose is to assess whether or not these changes will result in any new or materially different likely significant environmental effects when compared to those considered within the ES (as amended).

The Original ES

3. The Application seeks outline permission for a set of parameters that define the use, amount of development, zones of development and scale of development that could come forward on this site. All matters are reserved except for access.
4. The combined parameter plan for the outline application contains information on:
 - Use – uses proposed for the development (B1(a) and B1(b) Office);
 - Amount of development – the maximum floor area proposed (60,000 square metres (m²) Gross External Area (GEA));
 - Zones – indicating the maximum floor area and height for buildings within each zone; and
 - Scale – an indication of the upper and lower limits for the height of the proposed buildings within the zones.
5. The Proposed Development, comprises the construction of a business park of up to 60,000 m² GEA of Class B1(a) and B1(b) office floorspace, parking for up to 2,000 cars and associated highways, infrastructure and earthworks. The Bicester Office Park will be made up of differently sized buildings which will vary in height between 2 and 4 storeys and located within a landscaped space. The site will be accessed from Lakeview Drive via the signaled controlled junction with the A41 Oxford Road. Figure 1 illustrates the combined parameter plan which was submitted as part of the Original Planning application and assessed in the Original ES.
6. Since the submission of the 2017 ES, additional hydraulic modelling has been undertaken to better inform the fluvial flood risk at the site. This ES Addendum therefore provides a review of the Water Resources and Flood Risk Chapter as a result of a Revised Flood Risk Assessment and a review of a Revised Combined Parameter Plan which has been amended to take into account the revised fluvial flood risk of the site.
7. The ES Addendum also provides a response to various comments from CDC regarding Transport and Ecology related issues.

¹ Environmental Statement (Original ES) can be found via a search of Cherwell District Council's online planning portal, using the planning reference number: 17/02534/OUT

Figure 1 Combined Parameter Plan



Water Resources and Flood Risk

9. Since the submission of the 2017 ES, additional hydraulic modelling has been undertaken to better inform the fluvial flood risk at the site. This has involved updating the EA's 2010 hydraulic model with 2017 and 2018 topographic surveys for the site and adjacent floodplain. The details of the hydraulic modelling undertaken can be found appended to the updated Flood Risk Assessment (FRA) (Appendix A).
10. The updated hydraulic modelling has resulted in some amendments to the flood risk section of Chapter 13 of the 2017 ES. The amended section is presented below.

Flood Risk

Risk of Fluvial Flooding

11. Fluvial flooding occurs when sustained or intense rainfall events increase the flow in rivers causing water level to rise above the level of the banks and into surrounding areas.
12. The Flood Zone map produced by the EA shows that the majority of the site lies within Flood Zone 1 which is considered at low risk of flooding. However, land along the south east boundary lies within Flood Zone 2 and 3a, considered at a medium and high risk of flooding respectively, due to the Langford Brook approximately 180m from the site. There are also localised areas of Flood Zone 3b, classified as functional floodplain which has more than a 1 in 20 annual probability of flooding in any one year.
13. Figure 2 shows that along the south eastern boundary, there are areas that are defined at 'Very low hazard'.

Figure 2: Fluvial flooding hazard map for 1 in 100 year storm event + 35% climate change



(Contains Environment Agency Information © Environment Agency and/or database right) Imagery © Google 2017, Map data © Google 2017)

Risk of Flooding from Surface Water

14. There is no change to the risk of flooding from surface water as described in Chapter 13 and the FRA

of the 2017 ES.

Risk of Flooding from Sewers

- 15. There is no change to the risk of flooding from sewers as described in Chapter 13 and the FRA of the 2017 ES.

Risk of Flooding from Artificial Sources

- 16. There is no change to the risk of flooding from artificial sources as described in Chapter 13 and the FRA of the 2017 ES.

Additional Receptors

- 17. One of the comments raised regarding the Water Resources and Flood Risk chapter was the potential for Bicester Wetland Reserve Local Wildlife Site (LWS) to be hydrologically connected to the proposed development site. The LWS is located downstream from the application site and comprises priority habitats of grazing marsh and reed beds, both of which are dependent on hydrological flows. The LWS is fed from the Thames Water treatment works and runs parallel to, but does not appear to be connected to Langford Brook which will carry drainage water from the proposed development site. Therefore, it is not believed that there is a hydrological link between the site and the LWS under normal environmental circumstances. However, given that the possibility of hydrological connection cannot be ruled out, and this receptor has been included in the updated impact assessment below and is considered a low sensitivity due to no obvious hydraulic connection to the site and due to it not being protected under statutory designation.

Scheme Amendments Relevant to the Assessment

- 18. The amendments to the scheme relate to the change in flood risk and the consideration of an additional receptor. The sections below update the construction and operation effects from the 2017 ES.

Updated Residual Effects and Conclusions

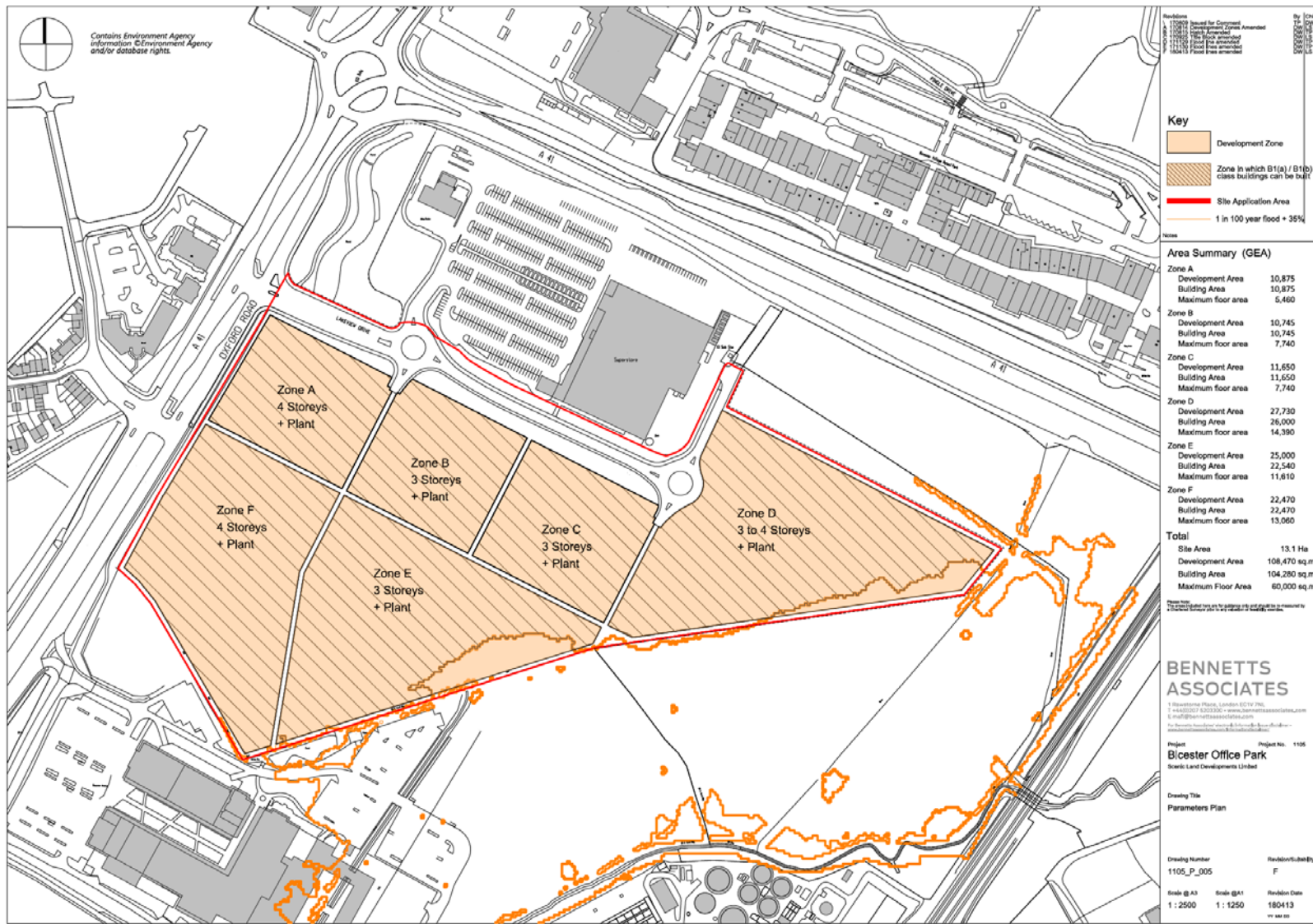
Phase	Receptor	Effect	2017 ES Assessment	Amended Proposed Development Residual Effects		
			Residual Effect Significance	Further Supplementary Mitigation	Effect Significance	
Construction	Garden Centre Pond – water quality	Dust and debris	Negligible	None, the measures already specified in 2017 ES are valid	Negligible	
	Bicester Wetland Reserve LWS	Dust and debris	Not considered in 2017 ES		Negligible	
	Langford Brook – water quality	Increased sediment loads	Minor adverse			Minor adverse
		Accidental release of hydrocarbons	Negligible			Negligible
		Accidental release of hazardous materials	Negligible			Negligible
		Dust and debris	Negligible			Negligible
		Leak or breakage of the temporary sewerage system	Negligible			Negligible
	Water services infrastructure – drainage sewer	Increased sediment loads	Negligible			Negligible
Surface water capacity		Negligible		Negligible		

Phase	Receptor	Effect	2017 ES Assessment	Amended Proposed Development Residual Effects	
			Residual Effect Significance	Further Supplementary Mitigation	Effect Significance
	Site users (construction workers and plant)	Flood risks to site workers	Major		Negligible
Operation	Langford Brook – water quality	Pollutants contained in surface water	Negligible	No further measures required, beyond those set out in the 2017 ES and specified in the Phase specific FRAs and Drainage Strategies that have since come forward	Negligible
	Water services infrastructure (surface water) - capacity	Decreased flood risk	Minor adverse		Minor adverse
	Water services infrastructure (supply)	Increased water demand	Minor adverse		Minor adverse
	Water services infrastructure (foul)	Increased foul water discharge	Minor adverse		Minor adverse
	Site users	Increased flood risk	Minor adverse		Minor adverse

Combined Parameter Plan

19. The combined parameter plan has been amended to take account of additional hydraulic modelling that has been undertaken to better inform the fluvial flood risk at the site. The revised flood extents and flood levels (1 in 100 Year flood (+35%)) have illustrated a lesser extent of the site that would be affected than previously envisaged. As a result, the Original Combined Parameter Plan zone in which B1(a)/B1(b) class buildings can be built has been extended (Figure 3 Revised Combined Parameter Plan). Whilst the footprint of the developable area has extended the maximum quantum of flood area is unchanged.
20. The proposed amendments to the combined parameter plan are considered marginal. The likely effects reported in the Original ES are unaffected by the extension to the developable area. This is because the maximum floor area is unchanged. The Revised Combined Parameter Plan is therefore not addressed any further.

Figure 3 Revised Combined Parameter Plan



Consultation with Cherwell District Council

Traffic & Transport

21. Following receipt of comments from OCC updated versions of the Transport Assessment and Framework Travel Plan have been submitted to address the comments raised.
22. OCC requested that further detail be provided on the assignment and distribution of vehicle trips associated with the development proposals. The updated Transport Assessment included additional information on the census data and vehicle routing assumptions that were adopted to assess the assignment and distribution of vehicle trips. This provided further background on the approach adopted but no changes were proposed to the assignment or distribution presented in the original submitted Transport Assessment or Environmental Statement. The assignment and distribution of trips has subsequently been agreed with Officers at OCC. There have been no changes to either the baseline or 'with development' traffic flows considered as part of the original ES Chapter.
23. OCC raised a number of comments in relation to the layout of the highway mitigation works proposed at the junction between A41 Oxford Road and Lakeview Drive. A revised proposal for highway mitigation works at this junction has been submitted to OCC, along with a Stage 1 Road Safety Audit.
24. The changes to the layout of the highway mitigation works proposed at the A41 Oxford Road and Lakeview Drive do not change in any changes to the assessment presented in the original ES chapter and the overall conclusion of that chapter.
25. An updated Framework Travel Plan has also been re-submitted to CDC and OCC. The updated Framework Travel Plan provides further details about how a Travel Plan will be implemented at the site and measures that could be introduced at the site to encourage sustainable travel choices amongst future staff and visitors.
26. The updated Transport Assessment therefore results in no changes to the assessment methodology set out within the original ES Chapter either during the construction or operation phases of the development and there are no changes to the scope of assessment undertaken.
27. In summary, the additional information submitted as part of an updated Transport Assessment and Framework Travel Plan do not have a material effect on the assessment and conclusions presented in the submitted ES Traffic and Transport chapter.
28. The conclusion of the submitted ES Traffic and Transport chapter are therefore still valid and the Proposed Development would result in negligible residual effect on the local highway network both during the construction phase and during the operational phase.

Ecology

Biodiversity Impact Assessment

29. CDC raised concerns as to whether the proposals as indicatively shown would lead to net biodiversity gain as well as ensure protected species are safeguarded as well as preserving/enhancing priority species as required by Policy Bicester 4 and ESD10 of the Local Plan. CDC therefore requested that a biodiversity impact assessment was undertaken based on the indicative masterplan.
30. Further to CDC concerns, Prime Environment, the ecologists who produced the ecological impact assessment for the Environmental Statement have created a GIS plan of the site's habitats and recorded parameters for entry into a Biodiversity Offsetting Matrix to complete a Biodiversity Impact Assessment (BIA) (Appendix B). The matrix used was the Warwickshire, Coventry and Solihull Biodiversity Offsetting Biodiversity Impact Assessment Calculator v18.
31. The BIA metric is used to calculate the biodiversity value of a site before and after development; this then calculates if the development is likely to cause a loss or gain to biodiversity. It is used to quantify the value

of biodiversity at any site and can form an evidence base on required compensation for a development, the amount of residual biodiversity impact and if necessary the amount of required offsite compensation (Biodiversity Offsetting). Should the Biodiversity Impact Assessment calculate a residual loss to biodiversity, once the mitigation hierarchy has been followed and the development is in accordance with all other local and national planning policy and law, it may be suitable to apply principals of biodiversity offsetting.

32. Prime Environment have run various scenarios to help explain the client's options from the 'do nothing' approach, to a scheme involving a variety of on and off-site habitat creation. The BIA has demonstrated that there are several ways of achieving a net biodiversity gain by creating habitats within the site ownership boundary either confined to the current red line boundary, or by creating habitats in the flood zone. During the detailed design of the scheme and in accordance with the ES, an appropriate Landscaping and Ecology plan will be produced which details habitat creation within the site. The Applicant will commit to a net biodiversity gain and the plan will include an updated BIA providing the necessary justification.

Drainage to Offsite Ponds

33. CDC also queried the drainage to the off-site ponds and whether these would be affected. The mitigation measures proposed in Chapter 13 including the Construction and Environment Management Plan (CEMP) and Sustainable Urban Drainage Systems (Appendix C) would ensure that any effect on the Bicester Wetland Reserve would be negligible in the event that the site is hydrologically linked to the Bicester Wetland Reserve via the Langford Brook.

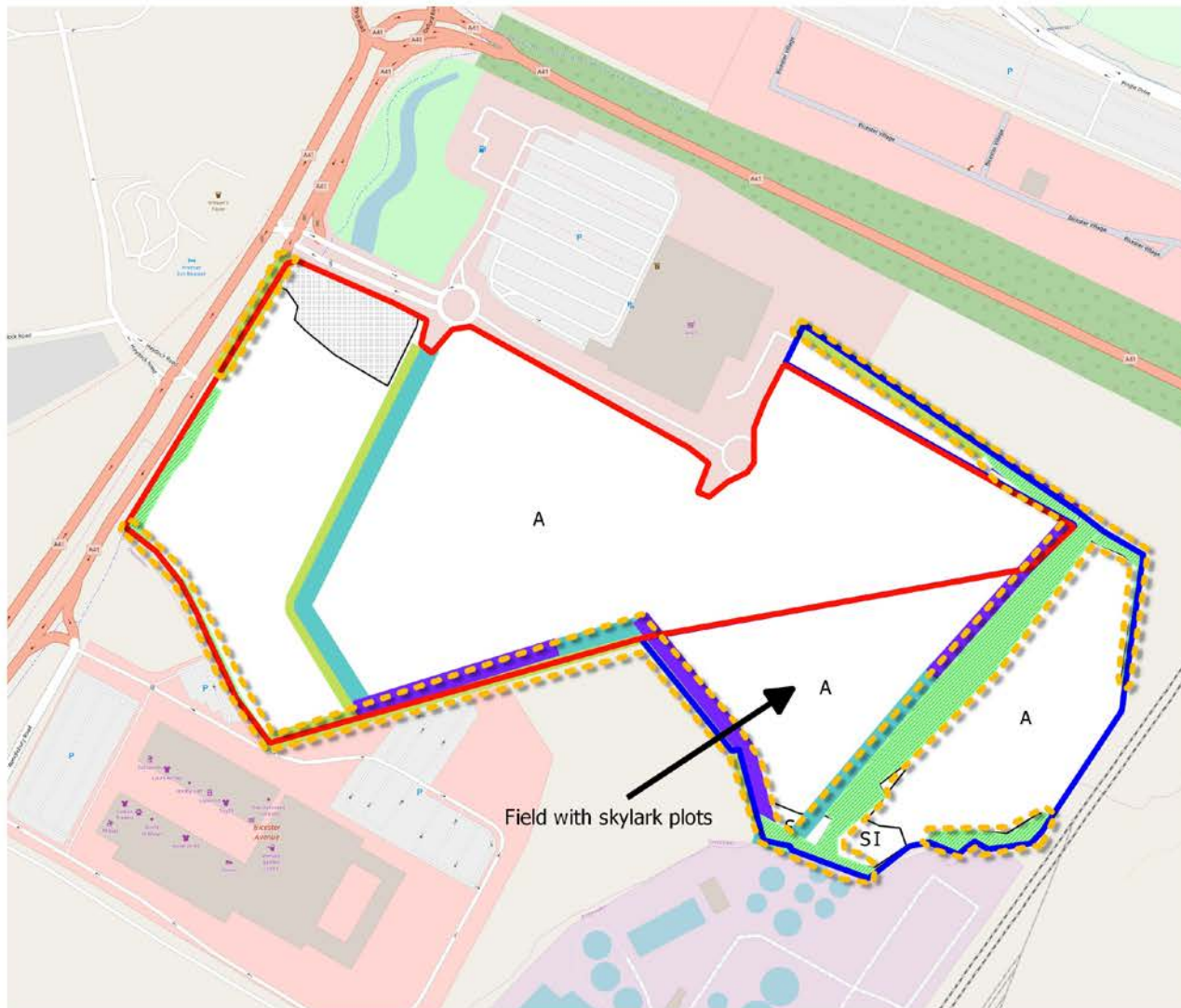
Skylark Mitigation

34. CDC has raised concerns that the proposed skylark mitigation plots would be too close to the built environment. To address this, we now propose an amended approach. The skylark plots will be included within the area to the east of the proposed development, outside of the red line plan but within the Applicants ownership (Figure 4). These will be provided within the arable field, or within areas of habitat created to balance the BIA of the scheme.
35. Plots will follow the prescription of the Higher Level Stewardship (HLS) Scheme AB4: skylark plots as far as is possible (the retained habitat size is smaller than the recommended 10 ha minimum). The plots aim to provide a bare area close to ground cover for foraging and 50 m from trees, hedges and buildings. Each plot will be at least three meters wide and with a minimum area of 16m². Four plots will be spaced across the larger of the two retained arable areas (or grassland if they are converted from crop) to provide a density of two plots per ha.

Energy

36. CDC requested a broad energy strategy for the proposed development (as per the indicative detailed scheme shown in the Design & Access Statement) that sets out the general approach to renewable energy provision and low carbon technology (Appendix D). This has been provided to CDC for information purposes and does not form part of the planning application. As such the air quality and noise assessments are not updated.
37. The air quality assessment within the original statement states that "*if a centralised energy plant is proposed, then further air quality assessment is required to determine the potential impacts and likely effects on air quality. This could be secured through planning condition*".
38. The noise chapter of the original ES states that "*plant has the potential to produce noise levels which may exceed the background sound levels by a medium magnitude, potentially leading to a moderate adverse effect if mitigation measures and plant noise limits are not inherent in the design of the installations. The design target (for a low impact) should be that cumulative noise from such plant does not exceed the background noise level at the nearby receptors*"

Figure 4: Habitat Plan



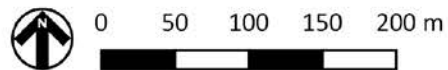
Bicester Office Park Biodiversity Impact Assessment

Figure 1 - Habitat Plan

Legend

- Red Line Area
 - Blue Line Area
 - Habitats retained in all scenarios
- Area Measurements
- A Arable
 - SI Poor semi-improved grassland
 - Mixed Wood - Plantation
 - Bare Ground
- Linear Measurements
- Hedges
 - Wet Ditch
 - Ditch - Dry

Map data copyrighted OpenStreetMap contributors and available from <https://www.openstreetmap.org>



Indicative only. Do not scale.

Review of Cumulative Impact Assessment

Combined Effects of Individual Impacts

39. The various amendments are not considered to materially alter the conclusions of the various technical topics assessed in the Original ES, the combined effects of individual impacts remain unchanged.

Combined Effects of Other Developments

40. The combined effects of other developments assessed in the Original ES remains unchanged.

Conclusions

41. It is considered amendments to the Proposed Development do not give rise to any materially different or additional likely significant environmental effects than those considered in the Original ES. The conclusions of the Original ES therefore remain valid for the Proposed Development when taking into consideration the amendments being proposed.

Appendices

Appendix A: 2018 Flood Risk Assessment

Appendix B: Prime Environment Biodiversity Impact Assessment

Appendix C: Initial Estimates of Greenfield Runoff and Onsite Storage

Appendix D: Bicester Office Park Energy Strategy

Appendix A: 2018 Flood Risk Assessment

Bicester Office Park

Flood Risk Assessment

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17 April 2018

Revision 04

Revision	Description	Issued by	Date	Checked
00	Draft for Comment	CJ	11/08/17	DKR
01	Final Draft for Comment	CJ	17/08/2017	DKR
02	For Planning	CJ	26/09/2017	DKR
03	For Planning, updated for 2017 topographic survey	CJ	14/12/2017	ADT
04	For Planning, updated for 2018 topographic survey	CJ	17/04/2018	MVS

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author **Clare Jones**

date **17/04/18**

approved **Matthew Vaughan-Shaw**

signature

date **17/04/18**

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Abbreviations

Term	Definition
AEP	Annual Exceedance Probability
EA	Environment Agency
FRA	Flood Risk Assessment
LLFA	Lead Local Flood Authority
mOD	Metres above Ordnance Datum
NPPF	National Planning Policy Framework
SFRA	Strategic Flood Risk Assessment
SuDS	Sustainable Drainage System

Glossary

Term	Definition
Annual Exceedance Probability (AEP)	The Probability that a storm event will be exceeded in any given year
Attenuation	A method to reduce a flood peak to prevent flooding, often utilising temporary storage, but increasing the duration of the flow
Design Flood Level	This is the level of flooding that flood defences or mitigation measures are designed against. This is typically the 1% (1 in 100) flood level with climate change allowance.
Discharge	The rate of flow of water measured in terms of volume per unit time
Flood Defence	A natural or man-made infrastructure used to prevent certain areas from inundation from flooding, and / or the provision of flood warning systems
Floodplain	Area of land adjacent to a water course which water flows or is stored during a flood event, or would otherwise be flooded in the absence of flood defences
Flood Risk	The level of risk to personal safety and damage to property resulting from flooding due to the frequency or likelihood of flood events
Flood Risk Assessment (FRA)	An assessment of the flood risks to the proposed development over its expected lifetime and the possible flood risks to the surrounding areas, assessing flood flows, flood storage capacity and runoff
Flood Warning Systems (FWS)	A system by which to warn the public of the potential of imminent flooding. This is typically linked to a flood forecasting system
Fluvial Flooding	Related or connected to a watercourse (river or stream)
Functional Floodplain	Greater than a 1 in 20 annual probability of flooding in any year
Groundwater	Water present within underground strata known as aquifers
Groundwater Flooding	Water occurring below ground in natural formations (typically rocks, gravels and sands)
Impermeable Surface	A surface that does not permit the infiltration of water and, therefore, generates surface water runoff during periods of rainfall
Mitigation	Actions taken to reduce either the probability of flooding or the consequences of flooding or a combination of the two
Red line boundary	Boundary drawn to indicate the site area on which the planning application is based
Residual Risk	The risk that remains after risk management and mitigation measures have been implemented
Return Period	The average frequency of a specified condition. An 'n' year event is one that occurs on average over the long term, once every 'n' years
Risk	Risk is the probability that an event will occur and the impact (or consequences) associated with that event
Runoff	Water flow over surfaces to the drainage system. Runoff occurs if the ground is impermeable or if permeable ground is saturated.
Strategic Flood Risk Assessment (SFRA)	An SFRA is the assessment and 'categorisation' of flood risk on an area-wide basis in accordance with PPS25
Surface Water Flooding	Surface water flooding occurs when the volume of water is unable to filtrate through the ground to enter drainage systems, and therefore runs quickly off land and results in localised flooding. This type of flooding is usually associated with intense rainfall.
Sustainable Drainage Systems (SuDS)	SuDS are used as a strategy to manage surface water in a sustainable manner or least damaging solution through management practices and physical structures.

1 Executive Summary

BuroHappold Engineering (BHE) has prepared this FRA on behalf of Scenic Land Developments Limited to support the Outline Planning Application for new office buildings and car parking at the Bicester Office Park site. This FRA has been undertaken in accordance with the National Planning Policy Framework (NPPF) and demonstrates that with the proposed mitigation measures, the development is considered safe up to the 1 in 100 flood event with allowance for climate change and does not increase flood risk elsewhere for the lifetime of the development. A summary of the key findings of the Flood Risk Assessment are provided in **Table 1-1**.

Table 1-1 Summary of the key findings

Subject	Element	Findings
Site Flood Risk	Fluvial	The majority of the site lies in Flood Zone 1. However, along the south eastern boundary, the site lies within 2, 3a and 3b.
	Ground Water	Low risk of flooding. Further ground investigation recommended.
	Surface Water	The majority of the site is at very low risk of surface water flooding. There are areas of low to high risk of flooding along the northern and eastern boundary, south east corner and adjacent to the drainage ditch which has now been infilled.
	Sewers and Artificial Sources	Low risk of flooding
Planning Requirements	Vulnerability Classification	Office buildings are classified as 'less vulnerable', appropriate for Flood Zone 1, 2 and 3a. Car parking located in Flood Zone 3b is considered appropriate provided no ground raising.
	Sequential Test and Exception Test	As the site is allocated within the Adopted LDP, the Sequential Test is considered to have passed. An Exception Test is not required for the site.
	Sequential Approach	The Sequential Approach has been applied within the site boundary by locating buildings outside the 1 in 100 + 35% climate change flood extent. During detailed design, apply Sequential Approach to locate office parking to areas of lower risk of flooding.
Mitigation measures	Design Flood Event	1 in 100 year +25% climate event.
	Climate change	25% to 35% allowance to be considered for the site in accordance with the latest guidance.
	Finished Floor Levels	Finished Floor Levels are proposed to be set at a minimum of the 1 in 100 year + 35% climate change plus 300mm freeboard.
	Safe access and egress	Safe access and egress to be provided from all buildings via Lakeview Drive at or above the 1in 100 year +35% climate change level.
	Floodplain compensation	No ground level raising is permitted within the Functional Floodplain. Ground raising is permitted between the 1 in 20 year flood extent and the 1 in 100 year + 25% climate change flood extent if flood compensation provided on a level for level and volume for volume basis on site.
	Construction Phase	Contractor will need to sign up to EA's flood warning service and to locate stockpiles outside the 1 in 1000 year flood extent.
	Surface water drainage strategy	Primary infrastructure constructed on the site, sized for the Proposed Development. Discharge rates limited to greenfield rates. SuDS techniques to be implemented. Exceedance routes will need to be considered to route flood water away from the threshold of buildings.
	Residual Risk	A flood evacuation and management plan should be considered during detailed design to manage the residual risk of surface water and fluvial flooding on the site posed to both people and vehicles.

2 Introduction

2.1 Background

This site specific Flood Risk Assessment (FRA) has been prepared by BuroHappold Engineering on behalf of Scenic Land Developments Limited as part of an Outline Planning Application for the Bicester Office Park development, hereafter referred to as the 'Proposed Development'. The application is in outline with all matters reserved except for access. This assessment has been carried out in accordance with the National Planning Policy Framework (NPPF).

Since the submission of the FRA for planning in December 2017, BuroHappold has undertaken hydraulic modelling to better inform the fluvial flood risk at the site. This has involved updating the EA's 2010 hydraulic model with 2017 and 2018 topographic survey for the site and adjacent floodplain. The details of the hydraulic modelling undertaken are provided in Appendix E. This FRA has been updated to reflect the revised flood extents and flood levels derived in the hydraulic modelling.

2.2 Site Description

The Proposed Development site is located to the south of Bicester in the Cherwell District of Oxfordshire, Ordnance Survey grid reference (NGR) SP 579 215. The site is bounded by the A41 Oxford Road to the west, the new Tesco foodstore to the north, to the east by open fields and to the south by Bicester Avenue shopping centre. A sewage treatment works is located to the south east of the site. The agricultural field drainage ditch that ran north/ south across the site towards the south eastern boundary has subsequently been backfilled with a perforated pipe (refer to section 4.1.1.1 for more details). The site area is approximately 13.1ha and is currently agricultural land.

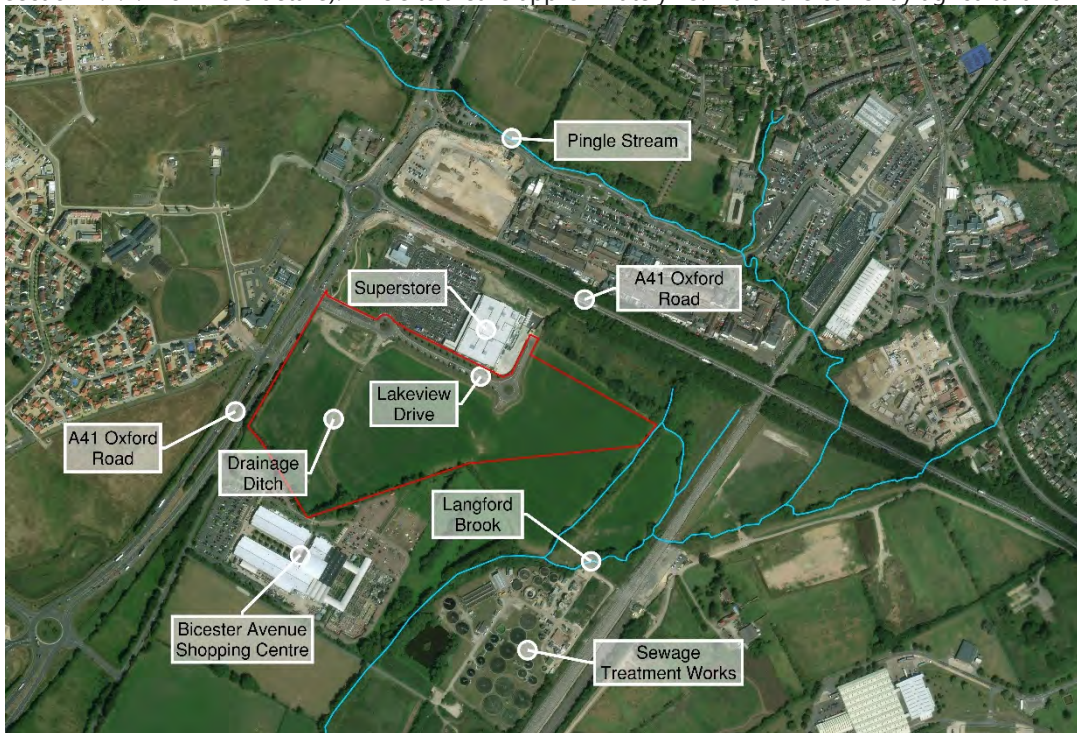


Figure 2-1 shows the location of the Proposed Development.

The Langford Brook is located approximately 180m to the south east of the Proposed Development and flows in a south westerly direction to the north of the sewage treatment works before cutting beneath the railway line. A land drain connecting into the Langford Brook is adjacent to the north east corner of the site.

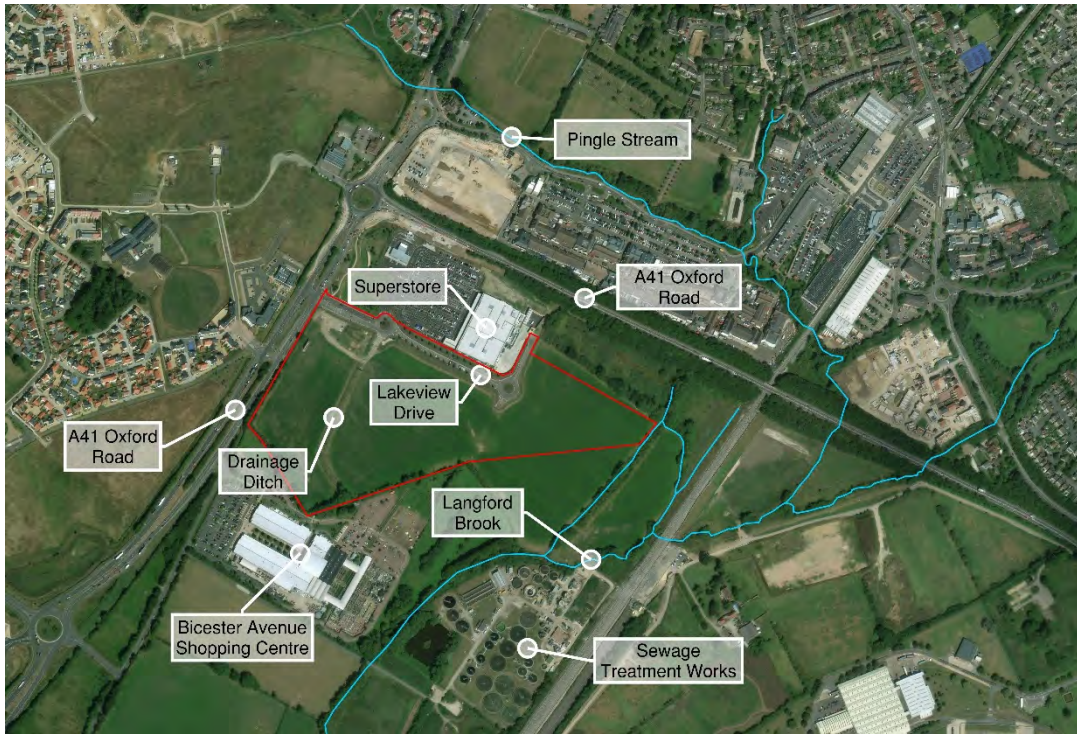


Figure 2-1: Site Location Plan with indicative red line boundary (Site Aerial received from Hyland Edgar Driver on 26/5/2017)

The site levels fall from Lakeview Drive in the north, and slopes down towards the south and south east boundary of the site towards the Langford Brook. Topographic survey data from 2017 and 2018 (Greenhatch Group) is available for the site and adjacent floodplain. These surveys indicate that land levels along Lakeview Road in the north of the site are typically between 66.5m AOD, increasing in the west to 67.7m AOD. Along the south of Lakeview Road, there is a 0.5m to 1.5m high bund and an area of material storage. Land slopes downwards from the road to the south boundary where land levels vary from 66.1m AOD to 64.8AOD and to south east where levels are typically between 64.4m AOD and 64.9m AOD. Refer to **Appendix A** for site survey information.

2.3 Proposed Development

The Proposed Development comprises between 55,000 and 60,000m² (gross external area) office use (B1(a) and B1(b)), parking for approximately 2,000 cars, associated highway, infrastructure and earthworks. The office park will be made up of differently sized buildings which will vary in height between two and four storeys and located with associated landscaping. **Figure 2-2** shows the Proposed Development parameters plan for the site and drawings are provided in Error! Reference source not found..

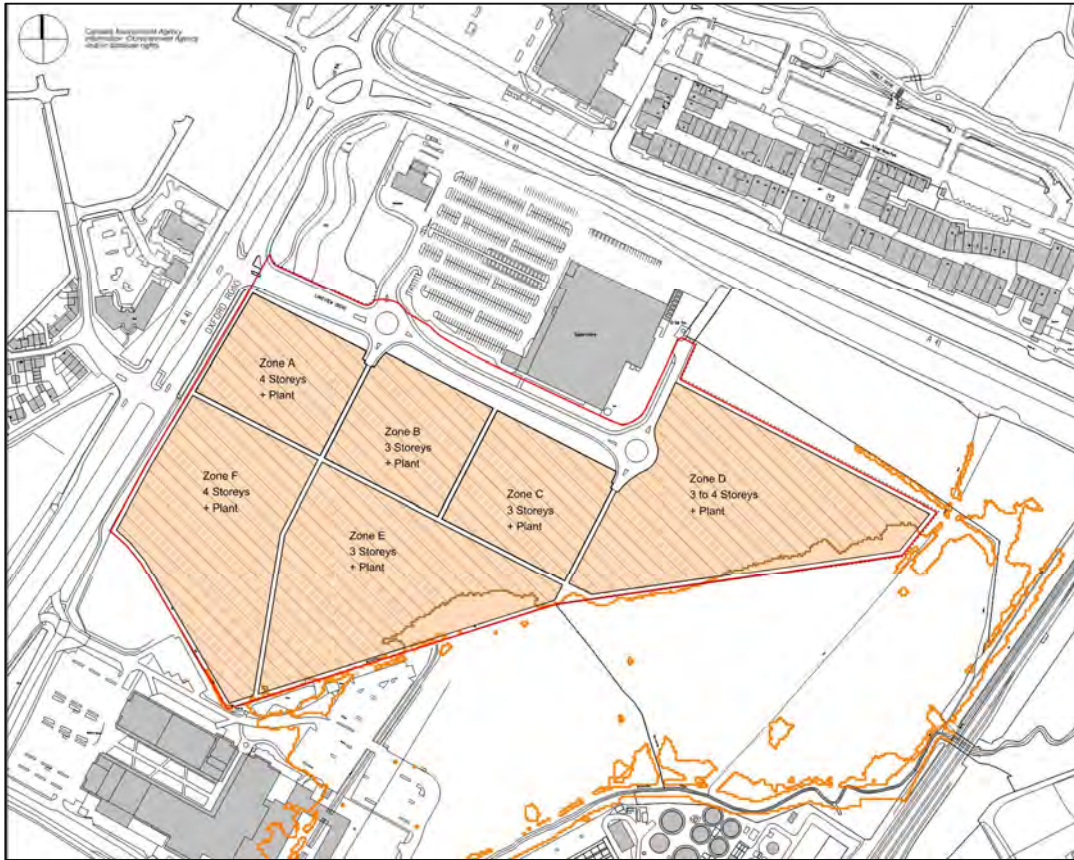


Figure 2-2: Proposed Development Parameters Plan (Drawing 1105_P_005 rev F, Bennetts Associates 13/4/18)

3 Planning Context

3.1 Overview

This FRA has been prepared in accordance with policies and guidance applicable to the Proposed Development outlined within the following publications:

- National Planning Policy Framework (March 2012)
- National Planning Policy Framework Planning Practice Guidance (March 2014)
- Flood Risk Assessments: climate change allowances (February 2016, updated February 2017)
- Thames Area Climate Change Allowances. Guidance for their use in flood risk assessments (January 2017)
- Cherwell and West Oxfordshire Level 1 Strategic Flood Risk Assessment (April 2009)
- Cherwell District Council Level 2 SFRA (March 2012)
- Oxfordshire County Council Preliminary Flood Risk Assessment Preliminary Assessment Report (June 2011)
- The Cherwell Local Plan 2011-2031. Part 1 Adopted 20 July 2015 (July 2015)

3.2 National Planning Policy Framework

3.2.1 Flood Zone Assessment

The National Planning Policy Framework¹ (NPPF) aims to avoid inappropriate development in areas at highest risk of flooding. The Planning Practice Guidance to the NPPF² contains a series of tables that help identify the risk of flooding to a development.

- Table 1 defines four Flood Zones based on the annual probability of river or sea flooding;
- Table 2 identifies specific land use types for each of the five flood risk vulnerability classifications (Essential Infrastructure, Highly Vulnerable, Less Vulnerable and Water Compatible Uses). For example, office buildings are classified as *less vulnerable*; and
- Table 3 identifies where development is appropriate for each flood risk vulnerability classification and whether the Exception Test is required.

The Flood Zones defined in the NPPF are as follows:

Flood Zone 1 Low probability

< 1 in 1,000 annual probability of river or sea flooding in any year (<0.1%).

Flood Zone 2 Medium probability

Between 1 in 100 and 1 in 1,000 annual probability of river flooding in any year (1% - 0.1%), or

between 1 in 200 and 1 in 1,000 annual probability of sea flooding in any year (0.5% - 0.1%).

Flood Zone 3a High probability

¹ Department for Communities and Local Government (2012). *National Planning Policy Framework*.

² Department for Communities and Local Government (2014). *National Planning Policy Framework Planning Practice Guidance*. [online] Available at: <https://www.gov.uk/guidance/flood-risk-and-coastal-change>. [Accessed 22 March 2017].

> 1 in 100 annual probability of river flooding in any year (>1%), or

> 1 in 200 annual probability of sea flooding in any year (>0.5%).

Flood Zone 3b Functional floodplain

> 1 in 20 annual probability of flooding in any year (5%).

The Proposed Development consists of office buildings which are classified as 'less vulnerable' in accordance with the NPPF Planning Practice Guidance and are considered appropriate for Flood Zone 1, 2 and 3a. The Environment Agency has confirmed that as the site is allocated in the Cherwell District Council Local Plan under Policy Bicester 4, car parking is considered acceptable within Flood Zone 3b. This is provided there is no ground raising within Flood Zone 3b.

3.2.2 Sequential and Exception Test

The NPPF states that *'inappropriate development in areas at risk of flooding should be avoided by directing development away from areas at highest risk, but where development is necessary, making it safe without increasing flood risk elsewhere'*. The aim of the Sequential Test is to steer new development to areas with the lowest probability of flooding. If this cannot be achieved, the Exception Test is required if indicated by the conditions specified in NPPF Table 3.

The Cherwell Local Development Plan (LDP) 2011-2031 Part 1 was adopted in July 2015 and re-adopted in December 2016. As the site is allocated under Policy Bicester 4 for Employment, the Sequential Test for the development is considered to be passed and justification is provided in Cherwell District Local Plan Sequential Test and Exception Test (Flooding) Document³. The Exception Test is not required for the Proposed Development as 'More Vulnerable' uses are not proposed on the site.

In accordance with NPPF and Policy Bicester 4 in the LDP, a Sequential Approach should be followed. The LDP policy requires *'where possible, buildings should be located away from areas at high risk of flooding but where it is necessary development should be made safe without measures increasing flood risk elsewhere'*⁴. For the Proposed Development, all the office buildings are to be located outside the 1 in 100 year + 35% climate change flood extent.

Policy Bicester 4 requires a site specific Flood Risk Assessment (FRA) to be undertaken for the Proposed Development. The Policy Bicester 4 also requires the following:

- Consideration of all sources of flooding for the site;
- 'Flood mitigation of flood risk in compliance with Policy ESD 6'⁴;
- The Proposed Development should be 'safe and remain operational (where necessary)'⁴;
- Consideration of the Strategic Flood Risk Assessment for the Proposed Development;
- Incorporation of Sustainable Drainage Systems (SUDs) for managing surface water on site which seek to 'reduce flood risk, reduce pollution and provide landscape and wildlife benefits'⁴;
- Reduction of surface water run off to greenfield discharge rates for the Proposed Development;
- Development is not within 8m of the watercourse banks.

The following site specific FRA has been prepared to meet the Policy Bicester 4 requirements.

³ Cherwell District Council. Sequential Test and Exception Test (Flooding) Strategic Sites (August 2012, updated October 2013).

⁴ Cherwell District Council. The Cherwell Local Plan 2011- 2031. Part 1 Adopted 20 July 2015. Policy Bicester 4: Bicester Business Park. (July 2015)

3.3 Consultation

3.3.1 Environment Agency

The EA has provided BuroHappold with the following information⁵ which was used to inform the assessment of flood risk to the Proposed Development:

- Flood map for planning;
- Modelled floodplain flood levels;
- Historical Flood data information;
- Flood defence information;
- Hazard Flood map;
- Bicester Flood Risk Mapping Study, Final Modelling Report (December 2009);
- Model Output data;
- Langford Brook (Bicester) & Pingle-Back-Bure 2010 ISIS-TUFLOW Model.

In addition to this, the Environment Agency has provided pre-application advice in June and July 2017 on their requirements for the Flood Risk Assessment including the approach to defining the flood extents, finished floor levels, development in Functional Floodplain and approach to floodplain compensation. In summary, the EA confirmed the following:

- The 1 in 20 year flood extent is classified as Functional Floodplain (Flood Zone 3b);
- The approach taken by BHE to define the flood extents for the 1 in 20, 1 in 100 and 1 in 1000 year using the EA's flood levels against the topographic survey and LiDAR data is acceptable*;
- Hydraulic modelling is required to define the flood levels for the 1 in 100 year + 25% and + 35% climate change scenarios required by the new 2016 climate change guidance⁶. Once defined, the same approach as above, using the topographic survey information and where unavailable, LiDAR was acceptable to define the flood extents*;
- The Design Flood Event (DFE) for the Proposed Development is the 1 in 100 year + 25% climate change allowance;
- A Sequential Approach should be taken to locating development on site. The EA advised that buildings should be located outside the 1 in 100 year + 35% climate change extent;
- Car parking within Flood Zone 3b is acceptable provided there is no ground raising;
- Minimum finished floor levels should be set at or above the DFE flood level plus 300mm freeboard, i.e. the 1 in 100 year + 25% climate change plus freeboard. For additional protection the EA has requested that the finished floor levels are set at 1 in 100 year +35% level plus 300mm freeboard.
- Ground raising outside the Functional Floodplain is not advised but would be acceptable provided floodplain compensation is provided up to the 1 in 100 year + 25% flood extent. The requirements for floodplain compensation would need to be considered through detailed design and could be dealt with through a planning condition.

A full copy of the data received and information provided by the EA is included in **Appendix C**.

⁵ Environment Agency Products 4, 5, 6 and 7

⁶ Flood Risk Assessments: climate change allowances (February 2016, updated February 2017)

* This approach has now been refined since hydraulic modelling has been undertaken with site specific topography. This is discussed further in section 4.1.1.1.

4 Appraisal and Management of Flood Risk

4.1 Fluvial Flooding

Fluvial flooding occurs when sustained or intense rainfall events increase the flow in rivers causing water level to rise above the level of the banks and into surrounding areas.

4.1.1 Baseline

4.1.1.1 Flood Zone Assessment

The Flood Zone map produced by the EA shows that the majority of the site lies within Flood Zone 1 which is considered at low risk of flooding. However, land along the south east boundary lies within Flood Zone 2 and 3a, considered at a medium and high risk of flooding respectively, due to the Langford Brook approximately 180m from the site. There are also localised areas of Flood Zone 3b, classified as functional floodplain which has more than a 1 in 20 annual probability of flooding in any one year.

The flood extents are defined as the following:

Flood Zone 1 Low probability

< 1 in 1,000 annual probability of river or sea flooding in any year (<0.1%).

Flood Zone 2 Medium probability

Between 1 in 100 and 1 in 1,000 annual probability of river flooding in any year (1% - 0.1%), or
between 1 in 200 and 1 in 1,000 annual probability of sea flooding in any year (0.5% - 0.1%).

Flood Zone 3a High probability

> 1 in 100 annual probability of river flooding in any year (>1%), or
> 1 in 200 annual probability of sea flooding in any year (>0.5%).

Flood Zone 3b Functional floodplain

> 1 in 20 annual probability of flooding in any year (5%).

BuroHappold Engineering has overlaid the 1 in 20, 1 in 100 and 1 in 1000 year flood extents provided as part of the Product 6 information with the red line boundary as shown in **Figure 4-1**. This indicates that the site partially lies within the 1 in 20 year flood extent. The EA has confirmed that the 1 in 20 year extent is Functional Flood plain i.e. Flood Zone 3b. The EA has no records of historical flooding on the site.

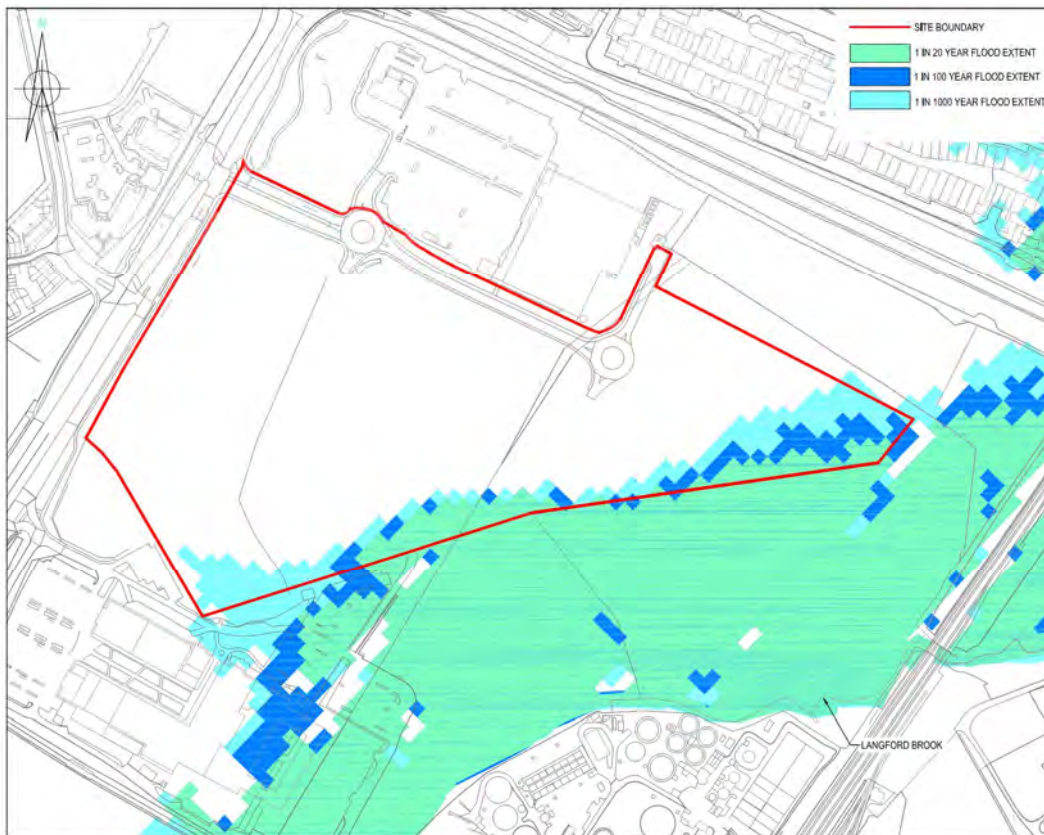


Figure 4-1 Flood Zone Extents overlaid with the red line boundary provided as part of the product 6 information from the Environment Agency on the 23rd June 2017. (Contains Environment Agency Information © Environment Agency and/or database right).

The EA produced a hydraulic model in 2010 for the Langford Brook, Pingle Stream, Bure Book and Back Brook watercourses which has derived the flood extents in Figure 4-1. Since the submission of the FRA in December 2017, BHE has undertaken hydraulic modelling to better inform the fluvial flood risk at the site. BHE has updated the EA's 2010 hydraulic model to create a hydraulic model which has been named BuroHappold BOP 2018 model. The updates include:

- Topographic Survey undertaken by GreenHatch Group issued on 28/09/17 – used to represent 2D floodplain within site boundary;
- Topographic Survey undertaken by GreenHatch Group issued on 19/02/18 – used to represent 1D in-channel cross sections and 2D floodplain between the site and Langford Brook;
- Most recent available LiDAR (combined dataset from 2003 and 2011 from data.gov.uk);
- Modified 2D boundary;
- Increase in grid resolution to capture the drainage ditches (from 10m x 10m to 2m x 2m);
- Model simulated for the 1 in 20yr, 100yr, 100yr +25% CC, 100yr + 35% CC and 1000yr flood events.

Drawings showing the topographic survey and LiDAR extents are provided in Appendix A. A copy of the hydraulic modelling report detailing the methodology and results is provided in Appendix E.

The revised flood extents are provided in **Figure 4-2** and provided in **Appendix D**. These have been used to inform the assessment of fluvial flood risk on the site and mitigation measures.

The drainage ditch that previously ran north/ south across the site towards the south eastern boundary functioned as an agricultural field drainage feature and was originally provided on the boundary of two different land ownerships. The adjoining land has been purchased by the applicant and the ownerships amalgamated into a single agricultural operation. The owners have filled this ditch for agricultural operations. Oxfordshire County Council confirmed that an Ordinary Watercourse Consent was not required. As it may have provided a limited field drainage function, a perforated drainage pipe has been installed as a precautionary measure to convey any flow to the pond. It is considered that the ditch does not provide a wider drainage function.

For the purposes of this Flood Risk Assessment, the drainage ditch has been assumed to have been filled in.

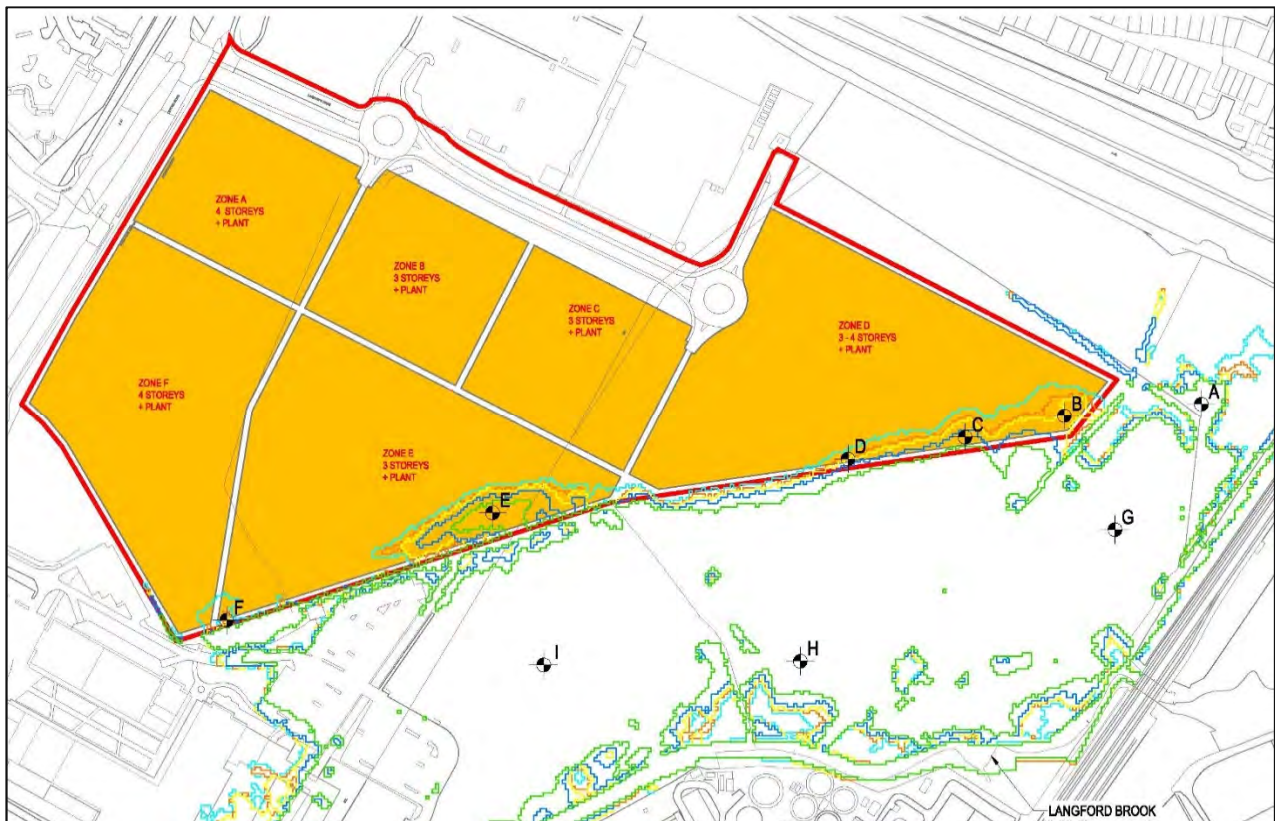
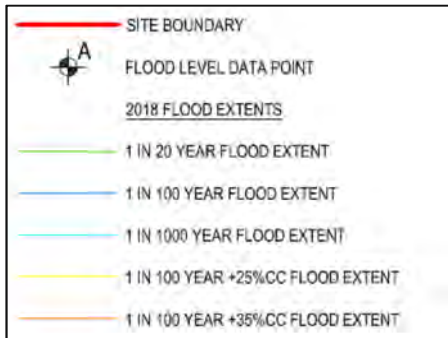


Figure 4-2– Revised Flood Zone Extents overlaid with the red line boundary (Contains Environment Agency Information © Environment Agency and/or database right). For full copyright details, refer to the drawing in Appendix D.

4.1.1.2 Flood Levels

BHE has extracted the flood level results for points along the extent of the south eastern boundary of the site from the BuroHappold BOP 2018 model results. These are provided for Points A to H in **Table 4-1**.

Table 4-1: Flood Levels extracted from the ISIS-TUFLOW within the floodplain (Contains Environment Agency Information © Environment Agency and/or database right).

Point	X Co-ordinate	Y Co-ordinate	Fluvial Flood Levels (mAOD)		
			1 in 20 year	1 in 100 year	1 in 1000 year
A	458310.6	221537.4	64.91	64.94	64.99
B	458224.2	221529.9	N/A*	N/A*	64.89
C	458161.6	221516.1	N/A*	64.77	64.87
D	458087.7	221502.4	N/A*	N/A*	64.85
E	457863.6	221468.6	64.51	64.58	64.70
F	457695.8	221400.9	N/A*	N/A*	64.67
G	458256.2	221457.9	64.90	64.93	64.99
H	458057.7	221375.2	64.64	64.72	64.82
I	457895.8	221372.8	64.46	64.56	64.69

*Flood water does not reach the point within the hydraulic model

4.1.1.3 Climate Change Allowance

Allowances for the predicted effects of climate change should be taken into account when preparing site-specific flood risk assessments. The guidance⁷ published by the EA in February 2016 to support the NPPF contains sensitivity ranges that are recommended to be applied to peak rainfall intensities, peak river flows, sea level rise, offshore wind speeds and extreme wave heights. The recommended allowances for increases in peak river flow rate in the Thames river basin district are given in **Table 4-2**.

Table 4-2: Climate change allowances for peak river flow in the Thames river basin district (Contains Environment Agency information © Environment Agency and database right)

Allowance category	Total potential change anticipated for 2015 to 2039	Total potential change anticipated for 2040 to 2069	Total potential change anticipated for 2070 to 2115
Upper end	25%	35%	70%
Higher central	15%	25%	35%
Central	10%	15%	25%

The EA guidance for the use of peak river flow allowances notes that the allowance category to be used depends on the land use vulnerability and the Flood Zone in which the site is located. Since the Proposed Development includes *less vulnerable* land uses, both the central and higher central allowances should be used. Considering a 60 year design life for the Proposed Development, the central peak river flow climate change allowance is 25% and the upper end allowance is 35%.

As the Proposed Development is classified as 'Large-Major' development, a vulnerability classification of 'Less vulnerable' and partially in Flood Zone 3, the EA has requested that hydraulic modelling is undertaken to determine the flood levels for 25% and 35% as these have not been modelled by the Environment Agency. This is in accordance with the Thames Area Climate Change guidance.

⁷ Environment Agency, (2016). *Flood risk assessments: climate change allowances*. [online] Available at: <https://www.gov.uk/guidance/flood-risk-assessments-climate-change-allowances> [Accessed 27th July 2017].

BHE has undertaken hydraulic modelling using the BuroHappold BOP 2018 model to establish the flood extents and flood levels for 1 in 100 event with 25% and 35% allowances for climate change. A summary of the flood level results are provided in **Table 4-3** and flood extents in **Figure 4-2**. For further information, refer to the hydraulic modelling report provided in **Appendix E**.

Table 4-3: Flood Levels extracted from the ISIS-TUFLOW within the floodplain (Contains Environment Agency information © Environment Agency and database right)

Point	X Co-ordinate	Y Co-ordinate	Fluvial Flood Levels (mAOD)	
			1 in 100 year + 25% climate change	1 in 100 year + 35% climate change
A	458310.6	221537.4	64.95	64.96
B	458224.2	221529.9	64.82	64.84
C	458161.6	221516.1	64.81	64.83
D	458087.7	221502.4	N/A*	64.80
E	457863.6	221468.6	64.63	64.65
F	457695.8	221400.9	64.61	64.63
G	458256.2	221457.9	64.95	64.96
H	458057.7	221375.2	64.76	64.78
I	457895.8	221372.8	64.62	64.64

*Flood water does not reach the point within the hydraulic model

4.1.1.4 Fluvial Flood Hazard

The fluvial flood hazard map for the 1 in 100 year + 35% climate change event has been provided in **Figure 4-3**. The map shows the hazard rating across the site (defined in **Table 4-4**). This is based on the following calculation which takes into consideration velocity (v) and depth of the floodwater (d) and debris factor (DF):

$$HR = d * (v+0.5) + DF$$

Figure 4-3 shows that along the south eastern boundary, there are areas that are defined at 'Very low hazard'.

Table 4-4 Flood Hazard Classifications⁸

Flood Hazard	Hazard to People Classification	
Less than 0.75	Very Low Hazard	Caution
0.75 to 1.25	Danger for some	Includes children, the elderly and the infirm
1.25 to 2.0	Danger for most	Includes the general public
More than 2.0	Danger for all	Includes the emergency services

⁸ HR Wallingford and Environment Agency (May 2008) Supplementary note of flood hazard ratings and thresholds for development planning and control purpose – Clarification of the Table 113.1 of FD2320/TR2 and Figure 3.2 of FD2321/TR1



Figure 4-3 Fluvial flooding hazard map for 1 in 100 year storm event + 35% climate change (Contains Environment Agency Information © Environment Agency and/or database right) Imagery © Google 2017, Map data © Google 2017)

4.1.2 Proposed Development

For the Proposed Development, ground levels within the Functional Floodplain (i.e. within the 1 in 20 year flood extent) are not to be raised in accordance with NPPF guidance and the EA's pre-application advice. At grade car parking within this zone is considered acceptable by the Environment Agency provided there is no raising of ground levels.

A sequential approach should be taken to locating development within areas of lower risk of flooding. The office buildings are to be located outside of the 1 in 100 + 35% climate change flood extents, with minimum floor levels applied. Car parking should be located, where possible, towards areas of lower risk of flooding (i.e. away from the south eastern boundary).

Finished floor levels for the office buildings are to be set at the 1 in 100 year + 35% climate change flood level with an additional 300mm freeboard.

During detailed design of the site, if ground raising is required between the 1 in 20 year flood extent and the 1 in 100 year + 25% climate change flood extent, then flood compensation will be required to be provided. This will need to be provided on a level for level and volume for volume basis on site in accordance with the Level 2 SFRA Table 5-3 guidance for the site.

4.1.2.1 Construction Phase

During the construction phase, the Contractor will need to sign up to the EA's flood warning service which covers the site and produces a construction flood and evacuation plan for managing flood risk on site during the construction phase.

During construction, stockpiles of material should not be stored within the Functional Floodplain as land raising is not permitted. It is recommended that stockpiles are located outside the 1 in 1000 year flood extent.

4.2 Flooding from Surface Water

Surface water flooding occurs when intense rainfall is unable to naturally soak into the ground due to impermeable ground covering such as concrete or tarmac, or low permeability ground conditions preventing infiltration. This excess surface water can flow through built-up areas and open space and pond in lower-lying areas causing localised flooding.

4.2.1 Baseline

The Environment Agency surface water map shows that the majority of the site is at very low risk of surface water flooding (i.e. less than 1 in 1,000 annual probability of surface water flooding in any year). **Figure 4-4** has been reproduced using the EA flood extent data. The map shows that there is an area at high risk of flooding (less than a 1 in 30 annual probability of surface water flooding) from the north to the south of the site. This corresponds to the location of the drainage ditch which has been infilled.

There are areas of low to medium risk of surface water flooding (between a 1 in 30 and 1 in 100 and between a 1 in 100 and 1 in 1000 annual probability respectively) adjacent to the drainage ditch, along the eastern boundary and south eastern corner of the site. The predicted depths from the EA's modelling are less than 300mm for the 1 in 100 annual probability event as shown in **Figure 4-5**.

The area along the northern boundary of the site shows areas of low, medium and high surface flood risk. This area has been re-configured as part of the 2015 superstore works which may not be reflected in the modelling. Depths for the 1 in 100 annual probability event are predicted as below 300mm.



Figure 4-4 Environment Agency's surface water flood extents map with indicative red line boundary. Accessed 16/8/17 (© Environment Agency copyright and/or database right 2015. All rights reserved. Some features of this map are based on digital spatial data from the Centre for Ecology & Hydrology, © NERC (CEH). Soils Data © Cranfield University (NSRI) and for the Controller of HMSO 2013. Imagery © Google 2017, Map data © Google 2017)



Figure 4-5 EA's surface water flood depth map for 1 in 100 annual probability event with indicative red line boundary. Accessed 16/8/17 (© Environment Agency copyright and/or database right 2015. All rights reserved. Some features of this map are based on digital spatial data from the Centre for Ecology & Hydrology, © NERC (CEH) and © Lead Local Flood Authorities. Soils Data © Cranfield University (NSRI) and for the Controller of HMSO 2013. Imagery © Google 2017, Map data © Google 2017)

Figure 4-6 shows that for the 1 in 100 annual probability event, the flooding in the locality of the drainage ditch has areas which pose a 'Danger for most', 'Danger for some' and areas 'Very Low Hazard – Caution'. However, the ditch has now been infilled and therefore the flood hazard will be reduced in this part of the site.

There are 'Very Low Hazard – Caution' areas along the south eastern northern boundary and to the west of the drainage ditch with localised spots of 'Danger for some' on Lakeview Drive and to the west of the drainage ditch.



Figure 4-6 Environment Agency’s surface water flood hazard map for the 1 in 100 annual probability event with indicative red line boundary. Accessed 16/8/17 (© Environment Agency copyright and/or database right 2015. All rights reserved. Some features of this map are based on digital spatial data from the Centre for Ecology & Hydrology, © NERC (CEH). Soils Data © Cranfield University (NSRI) and for the Controller of HMSO 2013. Imagery © Google 2017, Map data © Google 2017)

The 2011 Preliminary Flood Risk Assessment (PFRA) Map 1 and Map 2 show no recorded surface water flood events during July 2007 and other past events. The Level 2 SFRA also reports that the EA and Cherwell District Council have no records of surface water flooding on site.

In January 2014, following a period of major winter storms which brought widespread heavy and extended rainfall to the UK, BHE undertook a site visit to Bicester. BHE observed localised surface water ponding at the then recently excavated superstore construction site to the north of the development site where the underlying soil was identified as clay with poor permeability, as well as localised ponding at lower ground level areas in the vicinity of the manhole structures and overhead power line posts near the eastern boundary. BHE estimated that the rainfall over the 16 day period from 23rd December 2013 to 7th January 2014 was equivalent to a 1 in 17 year event.

4.2.2 Proposed Development

The primary surface water drainage infrastructure to serve the Proposed Development has already been constructed as part of the primary infrastructure contract for the site. The drainage was designed to provide capacity to serve the development proposals covered by the 2007 outline planning application.

The surface water infrastructure was installed along Lakeview Drive with spurs left to facilitate drainage connections from the masterplan. A 600mm diameter surface water pipe crosses the Proposed Development site and outfalls into the drainage ditch upstream of the confluence with the Langford Brook as shown in **Appendix F**.

The primary surface water sewer was designed with a capacity to serve the proposed 60,000m² B1 development. In accordance with the previously agreed drainage strategy, surface water runoff from the developed site will be limited to current 'greenfield' runoff rates and onsite storage will be required. The greenfield runoff rate will be estimated using the HR Wallingford *uksuds* tool. The sewer capacity of the constructed surface water drainage has been designed on this basis.

Attenuation measures for the developed site will be designed to accommodate the increased rainfall intensities in accordance with the climate change recommendations issued by the Environment Agency in February 2016.

The drainage system to serve the development site will incorporate the recommendations of Sustainable Drainage Systems (SuDS) good practice. The current Good Practice Guidance is contained in CIRIA Report C753 issued in 2015. This will be used to design the onsite drainage network unless superseded in the future.

In accordance with Policy Bicester 4, the site is not permitted to flood from surface water up to and including the 1 in 30 year event. Surface water flooding above this event up to a 1 in 100 year event with allowance for climate change is permitted provided it is safely contained within the site. During detailed design, exceedance routes will need to be considered to route flood water away from the threshold of buildings.

Refer to **Appendix F** for the surface water drainage strategy.

4.3 Flooding from Sewers

Flooding from sewers is typically associated with blockage, failure or overloading of the sewer network.

4.3.1 Baseline Flood Risk

The Level 2 SFRA Thames Water DG5 database map showed no recorded sewer flooding incidents within or in the vicinity of the site for the period during 2000-2010 from public foul, combined or surface water sewers. The SFRA also reported that Cherwell District were not aware of any historical incidents on the site but '*are aware of the limited sewer capacity in Bicester*'.

There are two existing combined public sewers which are to the south east of the proposed development site. These sewers run parallel to the existing ditch (tributary of the Langford Brook) from Bicester village to the sewage treatment plant as shown in **Figure 4-7** which is taken from the 2011 BuroHappold Drainage Strategy for the Tesco Development⁹. The BHE site report from 2014 showed evidence of localised sewer flooding however, these were related to manholes outside of the site boundary as shown in **Figure 4-7**.

⁹ Buro Happold 028858 Bicester Business Park Drainage Strategy (Pre Development Application for Tesco) Revision 02 (September 2011)

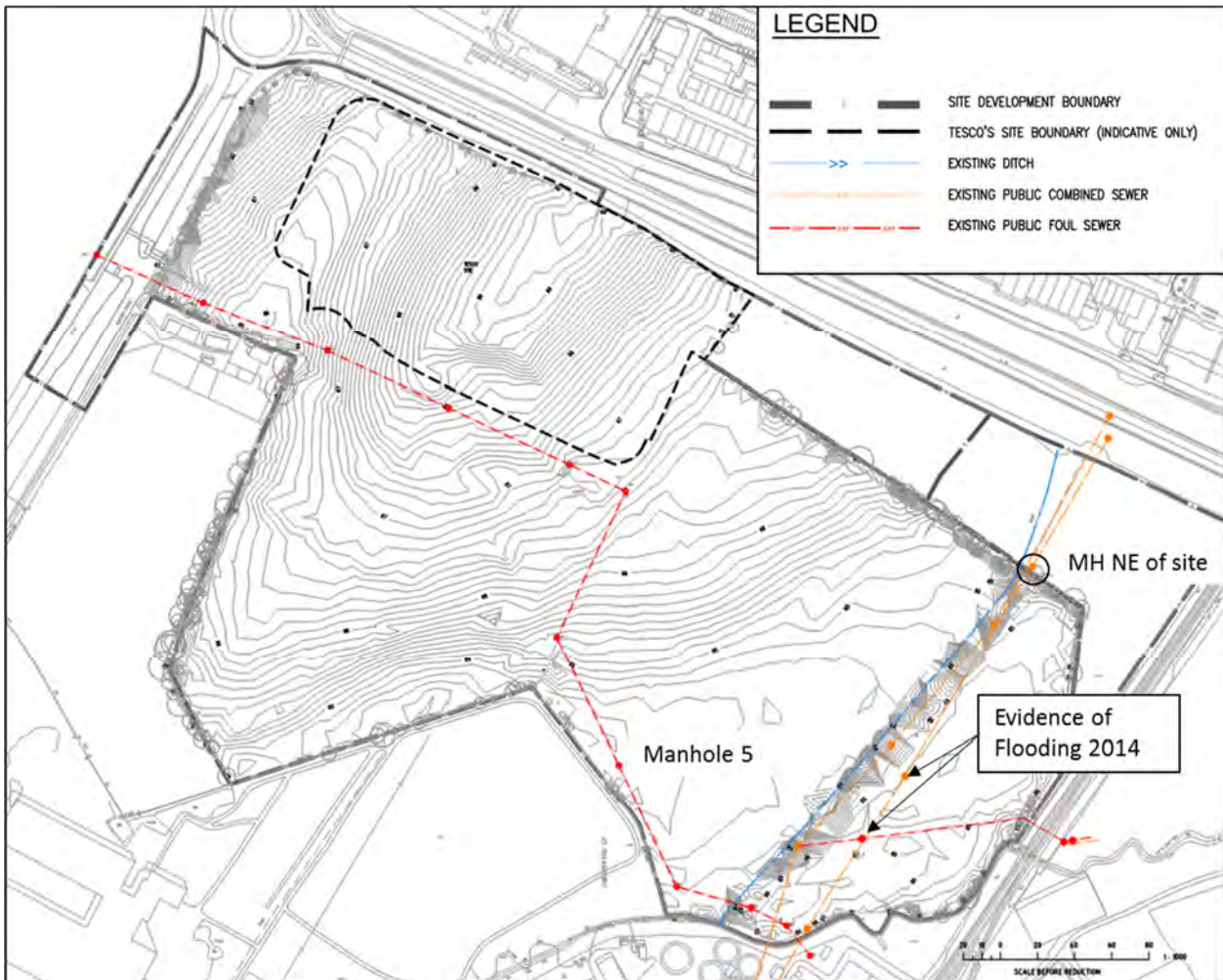


Figure 4-7 Existing Services Information from 2011 Tesco Drainage Strategy¹⁰.

There is also an existing 600mm diameter foul sewer which crosses the site from the A41 Oxford Road east along Lakeview Drive before turning south and then south east towards the sewage treatment works. This was installed as part of the primary infrastructure works to support the Tesco foodstore and masterplan works.

In December 2014/ January 2015, it was reported that there was localised foul flooding at Manhole 5 and the two combined sewers to the south east of the site. It is understood that this was associated with an issue downstream at the sewage treatment works rather than a capacity issue.

¹⁰ Buro Happold 028858 Bicester Business Park Drainage Strategy (Pre Development Application for Tesco) Revision 02 (September 2011)

There are no known sewer flood incidents on site however, there have been incidents of sewer flooding in the vicinity of the site due to downstream issues. During a site visit in November 2017, there was evidence of sewer flooding from the two combined sewer manholes and the manhole north east of the site (circled on **Figure 4-7**) by the presence of detritus. From a review of the topographic survey and LiDAR data in combination with a review on site, flood water from the north east manhole would likely flow along the drainage ditch to the east away from the site. BuroHappold are led to believe that the offsite foul sewer flooding at MH5 was as a result of a combination of unusual events which led to surcharging rather than a pipe capacity issue.

Overall the risk of sewer flooding to the site is therefore considered low. However, further consultation will be needed with Thames Water during detailed design.

4.3.2 Proposed Development

The primary foul water drainage infrastructure to serve the proposed development has already been constructed as part of the primary infrastructure contract for the site in 2011. The drainage was installed with connection points to facilitate the future connection of the masterplan site. The flow rates from the proposed development have been estimated based on the benchmarks for B1 uses. The total flow rate from the completed development will be very low in comparison with the capacity of the public sewer. It is not anticipated that there will be any flow restrictions placed on the connections by Thames Water. For further information refer to **Appendix F**.

4.4 Groundwater Flooding

Flooding from groundwater occurs when the water table in permeable rocks or soils such as chalk and limestone rises to enter underground spaces such as basements and cellars or reaches a sufficient level to emanate from the ground surface itself. Groundwater flooding is not necessarily directly linked to a specific rainfall event and is generally of longer duration than other causes of flooding (possibly lasting for weeks or months).

4.4.1 Baseline

The Cherwell District Council Level 2 SFRA provides the Environment Agency’s Area Susceptibility to Groundwater Flooding map. The map shows that the eastern half of the site lies within a 1km square which has up to 25% of its area susceptible to groundwater flooding and the western site between or equal to 25% and less than 50%.

The anticipated site geology is summarised in **Table 4-5** - Summary of Anticipated Geology. This has been determined with reference to the relevant BGS map (1:50,000 series, sheet 219, Buckingham. BGS 2002); BGS borehole logs; the Groundsure report and historic site investigation data.

Table 4-5 - Summary of Anticipated Geology

Strata	Description	Depth to top [Thickness] (m)	Aquifer status
Alluvium	Normally soft to firm consolidated, compressible silty clay, but can contain layers of silt, sand, peat and basal gravel. A stronger, desiccated surface zone may be present.	GL [$<3m$]	Secondary
River Terrace Deposits	Sand and gravel, locally with lenses of silt, clay or peat.	GL [<3]	Secondary
Kellaways Formation	Siltstone and mudstone.	GL – 3 [2-3]	Unproductive

Cornbrash Formation	Limestone, medium- to fine-grained, generally and characteristically intensely bioturbated and consequently poorly bedded. Generally bluish grey when fresh, but weathers to olive or yellowish brown. (Regionally between 1 to 4m thick)	<5 [2]	Secondary
Forest Marble Formation	Silicate-mudstone, greenish grey, variably calcareous. A variety of limestone types occur, of which grey, weathering brown and flaggy, variably sandy medium to coarsely bioclastic grainstone or less commonly, packstone predominates, especially at the base. (Regionally between 2 to 7m thick).	2.5 - >5 [7]	Unproductive
White Limestone Formation	A pale grey to off-white or yellowish limestone, peloidal wackestone and packstone with subordinate ooidal and shell fragmental grainstones. (Regionally between 7 and 18m thick)	9 [base not proven]	Principle

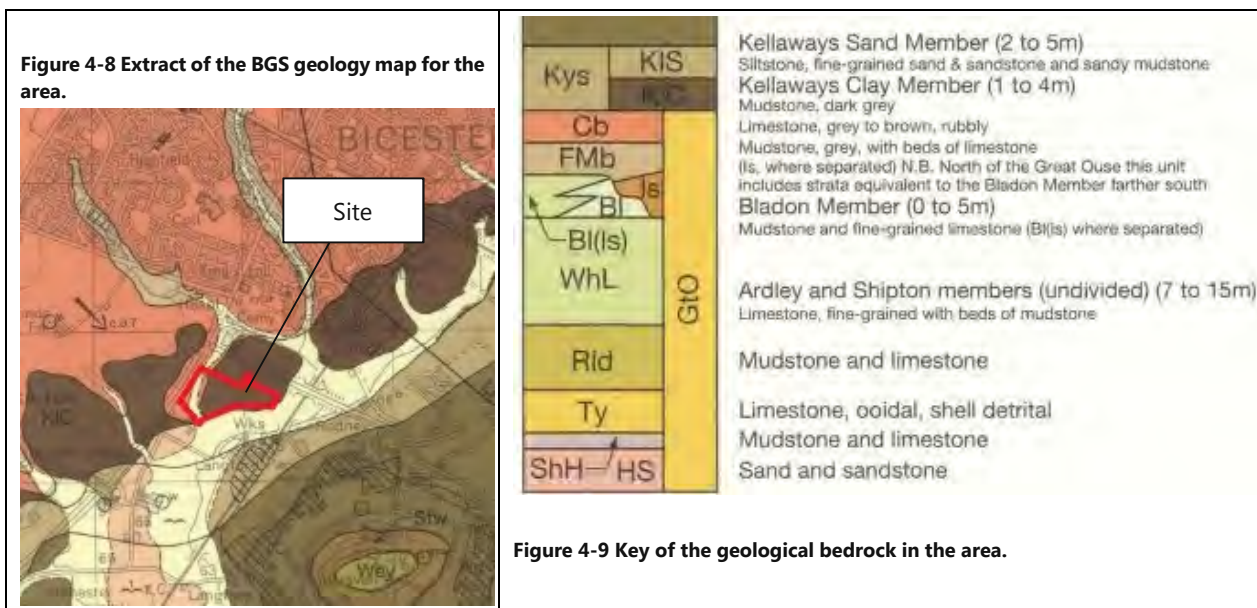


Figure 4-8 shows that a band of alluvium (cream) and the Cornbrash Formation (pink) underlies the western part of the site at the surface. Both of these are permeable formations and are classified as Secondary Aquifers which could potentially pose a risk of groundwater flooding. However, given the permeability of the alluvium, it is likely that an increase in groundwater level in the Cornbrash formation, is likely to be dissipated by the alluvium towards the river.

The alluvium band extends from the Langford Brook to the site as shown in Figure 4-8 and is likely to be in hydraulic connectivity with the river. The ground levels within the alluvium band in the south corner of the site are typically above 64.7m AOD with the lowest level at 64.54m AOD. The lowest ground levels on site are similar to the estimated 1 in 100 year + 35% climate change flood level within the river at 64.53m AOD (ISIS Flood Model Node LA 0762). As there is a lag for groundwater to respond to rising river levels, the primary flooding mechanism for the site is likely to be from water overtopping the banks of the Langford Brook upstream of the site rather than from rising groundwater levels.

Overall there is a low risk of groundwater flooding to the western part of the site. However, there may be a risk if groundwater rises and is unable to drain through the alluvium layer towards the river. This will need to be considered during the detailed ground investigation.

Ground investigation was undertaken on site in 2008 and 2014 for the proposed trunk sewer, access road and ornamental lake. Boreholes (BHs) BH2, BH 3 and Trial Pit (TP) TP1 shown in the site plan on the western part of the site in **Appendix G** show that groundwater was either not encountered or was an artesian groundwater level at depth between 8.9, and 11.7m within the Forest Marble Formation. This formation is considered a confined aquifer with low permeability.

The Eastern part of the site is underlain immediately by the Kellaways Formation which is classified as an Unproductive Aquifer with the Forest Marble Formation at depth. Boreholes and Trial Pits (BH 4 and 5, TPs 2, 3, 6 and 7) showed groundwater levels were within the superficial deposits between 0.6m and 1.4m. Given the low permeability of the Kellaways Formation geology, it is considered that there is a low risk of groundwater flooding for the Eastern part of the site. Overall the site is considered to be at low risk of groundwater flooding.

4.4.2 Proposed Development

The Proposed Development does not include development below ground level that could be affected by high ground water levels such as basement car parking. Although the risk of groundwater flooding to the Proposed Development is considered low, further ground investigation during detailed design will be undertaken and consideration through the design of foundations to minimise the impact of groundwater.

To minimise any risk from groundwater flooding during excavation of the new development, cut levels will be limited to at least 0.5m above groundwater level. Where this is not possible, dewatering and other groundwater control measures will be required. Any such groundwater control measures will also require pollution control measures in accordance with EA guidance.

4.5 Flooding from Artificial Sources

The Environment Agency map shows that there are no reservoirs located within the vicinity of the site and that the site does not lie within a breach flood flow path of a reservoir. The Preliminary Flood Risk Assessment Map 4 shows that there are no canals within the vicinity of the site and therefore the site is not at risk of canal flooding.

There is a pond to the north of the site as part of the Tesco foodstore development. This is an ornamental pond which forms part of the landscaping works and has an overflow into the drainage network. The pond is lower than the surrounding ground levels so the risk to the site resulting from breach of the pond is considered to be very low.

There is also a small pond along the south east boundary of the site which forms part of the surface water drainage strategy for the garden centre. The Level 2 SFRA advises that *'LiDAR has shown that it lies at a lower elevation to the site and therefore is not considered to pose a risk of flooding from breach¹¹*.

The site is therefore at low risk of flooding from artificial sources.

4.6 Other considerations

4.6.1 Safe access and egress

A safe access and egress route for the site for vehicles and pedestrians will be via Lakeview Drive which is within Flood Zone 1 to the A41 Oxford Road to the west of the site. A safe access and egress route will need to be provided at a minimum of 1 in 100 year + 35% climate change flood level from each of the office buildings.

¹¹ Cherwell District Council. Cherwell District Council Level 2 SFRA (March 2012)

4.6.2 Residual Risk

There is a residual flood risk to the site as there are areas which flood in a 1 in 20 year event. A sequential approach should be taken to locating development within areas of lower risk of flooding. Office buildings are to be located outside the 1 in 100 year +35% climate change flood extent. The finished floor levels for the buildings will be set at or above the 1 in 100 year + 35% climate change plus 300mm, which is above the flood level in the 1 in 1000 year event. However, there is a residual risk of flooding for 1 in 1000 year to the external areas of the site, potentially impacting the access to the buildings.

During detailed design, office car parking will need to be located on the site and this may need to be located in areas of the site at greater annual probability of flooding.

A flood evacuation and management plan will be required during detailed design to manage the residual risk of flooding on the site posed to both people and vehicles. The plan will consider:

- Signing up to the EA's flood warning service to provide early warning of a flood event on site;
- Closing of parts of the site predicted to be affected by flooding to prevent people entering the floodwater;
- Moving cars within car parking areas predicted to be affected by flooding to other areas on site or offsite;
- Methodology to establish how the flood levels are monitored and what/ when actions are taken on site.

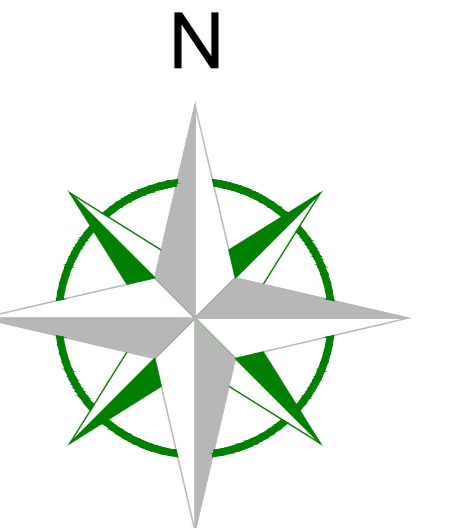
5 Summary and Conclusions

BHE has prepared this FRA on behalf of Scenic Land Developments Limited to support the Outline Planning Application for the Bicester Office Park site. This FRA has been undertaken in accordance with the National Planning Policy Framework (NPPF) and demonstrates that with the proposed mitigation measures, the Proposed Development is considered safe up to the 1 in 100 flood event with allowance for climate change and does not increase flood risk elsewhere for the lifetime of the Proposed Development. A summary of the key findings of the Flood Risk Assessment are provided in **Table 5-1**.

Table 5-1 Summary of the key findings

Subject	Element	Findings
Site Flood Risk	Fluvial	The majority of the site lies in Flood Zone 1. However, along the south eastern boundary, the site lies within 2, 3a and 3b.
	Ground Water	Low risk of flooding. Further ground investigation recommended.
	Surface Water	The majority of the site is at very low risk of surface water flooding. There are areas of low to high risk of flooding along the northern and eastern boundary, south east corner and adjacent to the drainage ditch which has now been infilled.
	Sewers and Artificial Sources	Low risk of flooding
Planning Requirements	Vulnerability Classification	Office buildings are classified as 'less vulnerable', appropriate for Flood Zone 1, 2 and 3a. Car parking located in Flood Zone 3b is considered appropriate provided no ground raising.
	Sequential Test and Exception Test	As the site is allocated within the Adopted LDP, the Sequential Test is considered to have passed. An Exception Test is not required for the site.
	Sequential Approach	The Sequential Approach has been applied within the site boundary by locating buildings outside the 1 in 100 + 35% climate change flood extent. During detailed design, apply Sequential Approach to locate office parking to areas of lower risk of flooding.
Mitigation measures	Design Flood Event	1 in 100 year +25% climate event.
	Climate change	25% to 35% allowance to be considered for the site in accordance with the latest guidance.
	Finished Floor Levels	Finished Floor Levels are proposed to be set at a minimum of the 1 in 100 year + 35% climate change plus 300mm freeboard.
	Safe access and egress	Safe access and egress to be provided from all buildings via Lakeview Drive at or above the 1in 100 year +35% climate change level.
	Floodplain compensation	No ground level raising is permitted within the Functional Floodplain. Ground raising is permitted between the 1 in 20 year flood extent and the 1 in 100 year + 25% climate change flood extent if flood compensation provided on a level for level and volume for volume basis on site.
	Construction Phase	Contractor will need to sign up to EA's flood warning service and to locate stockpiles outside the 1 in 1000 year flood extent.
	Surface water drainage strategy	Primary infrastructure constructed on the site, sized for the Proposed Development. Discharge rates limited to greenfield rates. SuDS techniques to be implemented. Exceedance routes will need to be considered to route flood water away from the threshold of buildings.
	Residual Risk	A flood evacuation and management plan should be considered during detailed design to manage the residual risk of surface water and fluvial flooding on the site posed to both people and vehicles.

Appendix A Topographic and LiDAR survey



Station Information:

Station	Easting (m)	Northing (m)	Level (m)
GH1	458049.908	221684.512	67.095
GH2	458016.141	221588.821	67.325
GH3	457918.706	221648.867	67.729
GH4	457813.990	221701.959	67.358
GH5	457769.668	221703.759	66.947
GH6	457715.541	221753.903	67.049
GH7	457723.838	221818.099	68.544
GH8	457783.068	221886.926	69.454
GH9	457618.945	221662.947	67.808
TA16	457841.170	221311.839	64.373
TA17	457751.130	221341.429	64.375
TA18	457665.165	21373.9880	65.364
TA19	457597.196	221422.121	66.110
TA20	457508.503	221468.539	67.041
TA21	457624.739	221652.797	67.471

OS Note:
Some services may have been omitted due to parked vehicles.
The Ordnance Survey file is to be used as a guide only.

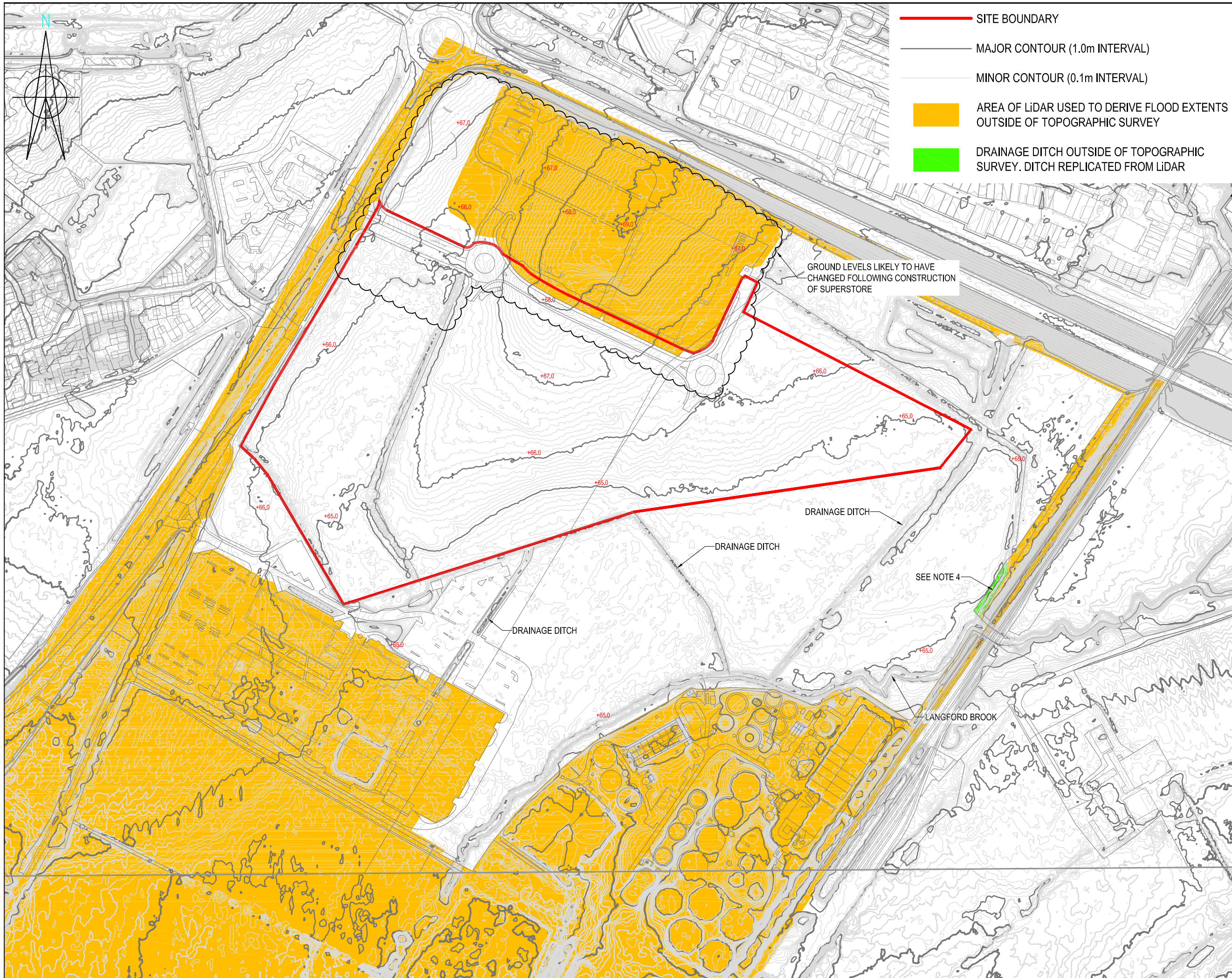
OS Buildings Surveyed Buildings

This survey has been orientated to the Ordnance Survey (O.S.) National Grid OSGB36(15) via Global Navigational Satellite Systems (GNSS) and the O.S. Active Network (OS Net).
A true OSGB36 coordinate has been established near to the site centre via a transformation using the OSTN15GB & OSGB15GB transformation models.
The survey has been correlated to this point and a further one or more OSGB36 (15) points established to create a true O.S. bearing for angle orientation.

No scale factor has been applied to the survey therefore the coordinates shown are arbitrary & not true O.S. Coordinates which have a scale factor applied.
Please refer to Survey Station Table to enable establishment of the on-site grid and datum.

Legend:

	Buildings		Overhead Cables		IC		Proprietary		Street
	Surveyed Buildings		Concrete walls		Proprietary		Proprietary		Boundary
	Water		Concrete walls		Proprietary		Proprietary		Boundary
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- SITE BOUNDARY
- MAJOR CONTOUR (1.0m INTERVAL)
- MINOR CONTOUR (0.1m INTERVAL)
- AREA OF LIDAR USED TO DERIVE FLOOD EXTENTS OUTSIDE OF TOPOGRAPHIC SURVEY
- DRAINAGE DITCH OUTSIDE OF TOPOGRAPHIC SURVEY. DITCH REPLICATED FROM LIDAR

- Notes
1. BACKGROUND OS TILE Ref. 100019980
 2. COMBINED LIDAR (2M RESOLUTION) DIGITAL TERRAIN MODEL DOWNLOADED FROM <http://environment.data.gov.uk>. CONTAINS PUBLIC SECTOR INFORMATION LICENSED UNDER THE OPEN GOVERNMENT LICENCE v3.0. LIDAR COMBINES DATA FROM 2003 AND 2011
 3. CONTOURS DERIVED FROM LIDAR SURVEY POINT LEVEL DATA
 4. SECTION OF DRAINAGE DITCH UNABLE TO BE SURVEYED DURING THE 2018 TOPOGRAPHIC SURVEY DUE TO BEING INACCESSIBLE. PLAN LOCATION BETWEEN LANGFORD BROOK AND DRAINAGE DITCH TAKEN FROM LIDAR WITH BANK LEVELS BEING INTERPOLATED USING LEVELS RECORDED IN THE 2018 TOPOGRAPHIC SURVEY.

03 NEW TOPO SURVEY EXTENT INFORMATION / 29.03.18	SM/CJ
02 NEW TOPO SURVEY EXTENT INFORMATION / 17.10.17	SM/CJ
01 AMENDED PROJECT NAME INFORMATION / 12.09.17	SM/CJ
00 INFORMATION / 16.08.17	SM/CJ
Rev Description/Date	Drm/Chk

INFORMATION

Status of drawing

BUROHAPPOLD
ENGINEERING

Camden Mill Lower Bristol Road Bath BA2 3DQ UK
Tel: +44 (0)1225 320600 Fax: +44 (0)870 787 4148 Email: 0040031@burohappold.com Web: www.burohappold.com

Architect *Bennett Associates*

Project **Bicester Office Park**

Drg Title **LiDAR SURVEY WITH 100mm INTERVAL CONTOURS**

Scales@A3 NTS

Drawn by SM

Checked by CJ

Date AUGUST 2017

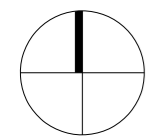
Job No. **0040031**

Drg No. **WSK007**

Rev **03**

Simon Mudd \lsrv-jondon05\CAD\0040031 Bicester Business Park- Planning Support\F34 Water\Sheet\Sketches\WSK007.dwg

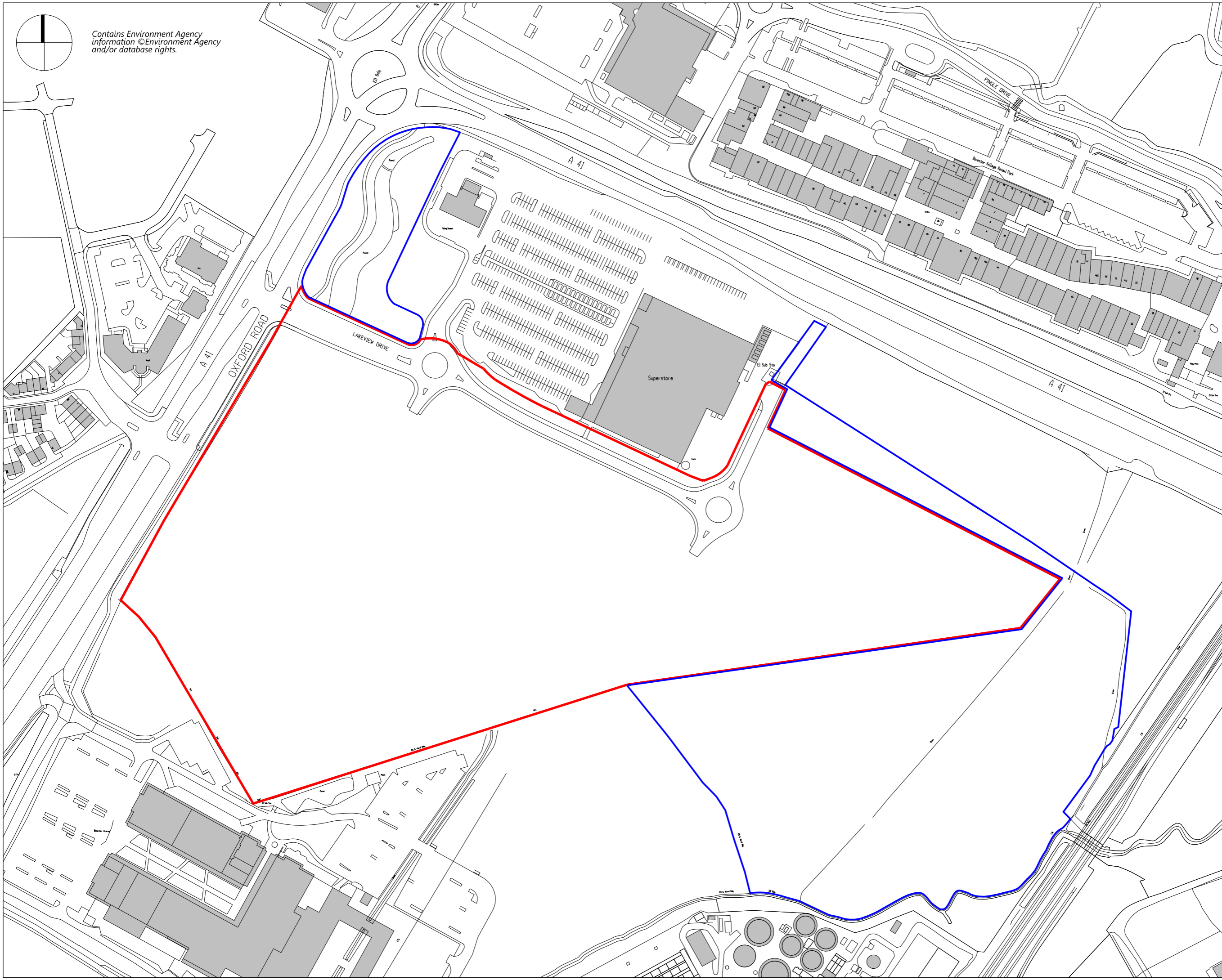
Appendix B Proposed Development



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Revisions
170815 First Issue
A 170925 Title block amended

By Chk
DW LS
DW LS



Notes

- Land within planning application boundary
- Land within applicant's ownership

Site application area: 13.1 Hectares

BENNETTS ASSOCIATES

1 Rawstorne Place, London EC1V 7NL
T +44(0)207 5203300 • www.bennettsassociates.com
E mail@bennettsassociates.com

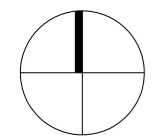
For Bennetts Associates' electronic information issue of claimer - www.bennettsassociates.com/info/visualisation/

Project **Bicester Office Park** Project No. 1105
Scenic Land Developments Limited

Drawing Title
Planning Application Boundary

Drawing Number **1105_P_004** Revision/Suitability **A**

Scale @ A3 **1 : 2500** Scale @A1 **1 : 1250** Revision Date **170925**
YY MM DD



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Revisions	By	Chk
170809 Issued for Comment	TP	DW
A 170814 Development Zones Amended	DW	LS
B 170815 Hitch Amended	DW	TP
C 170925 Title Block amended	DW	TP
D 171129 Flood line amended	DW	TP
E 171130 Flood lines amended	DW	TP
F 180413 Flood lines amended	DW	LS

Key

	Development Zone
	Zone in which B1(a) / B1(b) class buildings can be built
	Site Application Area
	1 in 100 year flood + 35%

Notes

Area Summary (GEA)

Zone A		
Development Area		10,875
Building Area		10,875
Maximum floor area		5,460
Zone B		
Development Area		10,745
Building Area		10,745
Maximum floor area		7,740
Zone C		
Development Area		11,650
Building Area		11,650
Maximum floor area		7,740
Zone D		
Development Area		27,730
Building Area		26,000
Maximum floor area		14,390
Zone E		
Development Area		25,000
Building Area		22,540
Maximum floor area		11,610
Zone F		
Development Area		22,470
Building Area		22,470
Maximum floor area		13,060
Total		
Site Area		13.1 Ha
Development Area		108,470 sq.m
Building Area		104,280 sq.m
Maximum Floor Area		60,000 sq.m

Please Note:
The areas included here are for guidance only and should be re-measured by a Chartered Surveyor prior to any valuation or feasibility exercise.

BENNETTS ASSOCIATES

1 Rawstorne Place, London EC1V 7NL
T +44(0)207 5203300 • www.bennettsassociates.com
E mail@bennettsassociates.com

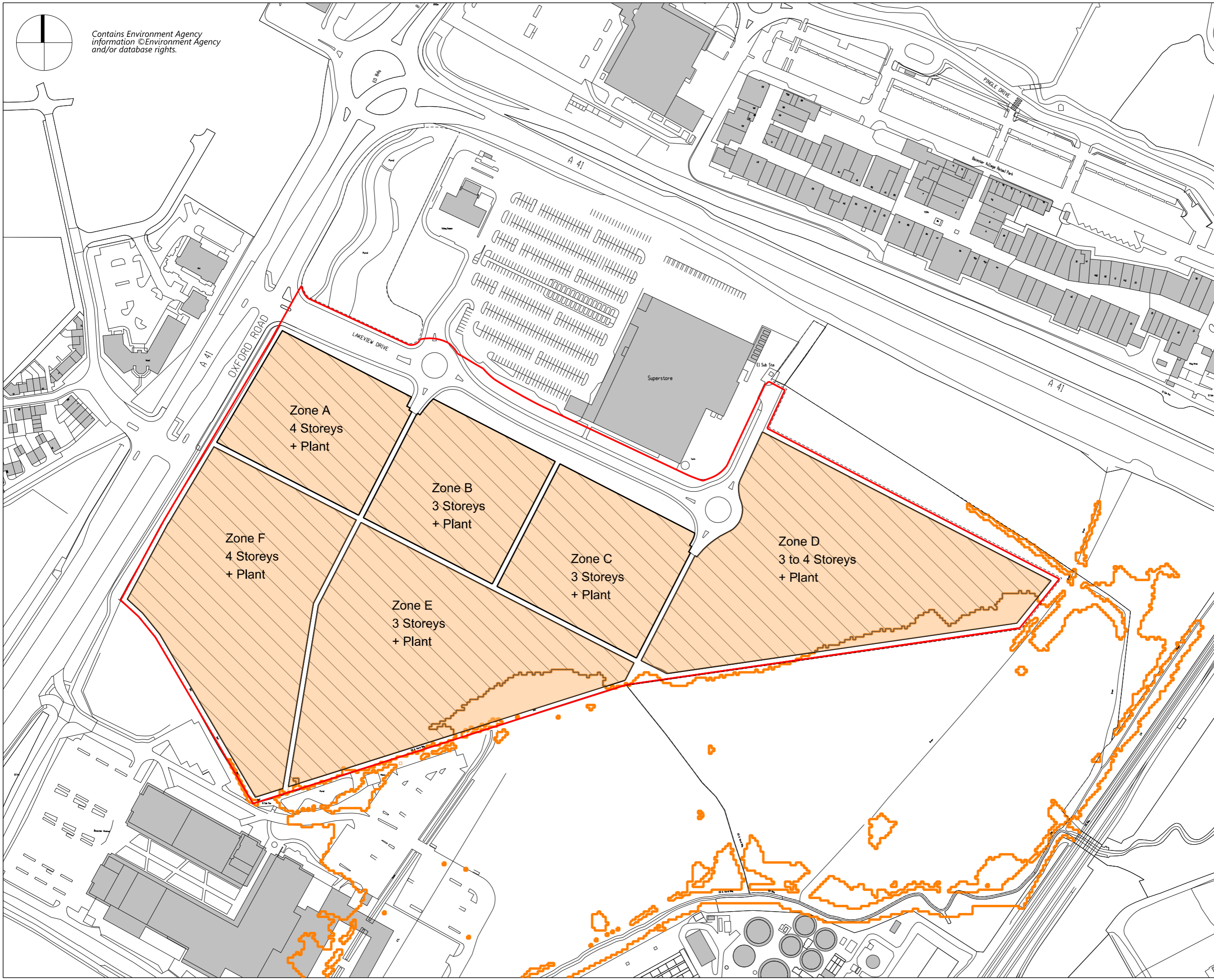
For Bennetts Associates' electronic information issue of a plan -
www.bennettsassociates.com/info/visualisation/

Project **Bicester Office Park** Project No. 1105
Scenic Land Developments Limited

Drawing Title
Parameters Plan

Drawing Number
1105_P_005

Scale @ A3 1 : 2500
Scale @ A1 1 : 1250
Revision Date
180413
YY MM DD



Appendix C Environment Agency Consultation

Product 4 (Detailed Flood Risk) for Bicester Office Park, Oxfordshire, OX26 1DE

Our Ref: **THM48041**

Product 4 is designed for developers where Flood Risk Standing Advice FRA (Flood Risk Assessment) Guidance Note 3 Applies. This is:

- i) "all applications in Flood Zone 3, other than non-domestic extensions less than 250 sq metres; and all domestic extensions", and
- ii) "all applications with a site area greater than 1 ha" in Flood Zone 2.

Product 4 includes the following information:

Ordnance Survey 1:25k colour raster base mapping;
Flood Zone 2 and Flood Zone 3;
Relevant model node locations and unique identifiers (for cross referencing to the water levels, depths and flows table);
Model extents showing *defended* scenarios;
FRA site boundary (where a suitable GIS layer is supplied);
Flood defence locations (where available/relevant) and unique identifiers; (supplied separately)
Flood Map areas benefiting from defences (where available/relevant);
Flood Map flood storage areas (where available/relevant);
Historic flood events outlines (where available/relevant, not the Historic Flood Map) and unique identifiers;
Statutory (Sealed) Main River (where available within map extents);

A table showing:

- i) Model node X/Y coordinate locations, unique identifiers, and levels and flows for *defended* scenarios.
- ii) Flood defence locations unique identifiers and attributes; (supplied separately)
- iii) Historic flood events outlines unique identifiers and attributes; and
- iv) Local flood history data (where available/relevant).

Please note:

If you will be carrying out computer modelling as part of your Flood Risk Assessment, please request our guidance which sets out the requirements and best practice for computer river modelling.

This information is based on that currently available as of the date of this letter. You may feel it is appropriate to contact our office at regular intervals, to check whether any amendments/ improvements have been made. Should you re-contact us after a period of time, please quote the above reference in order to help us deal with your query.

This information is provided subject to the enclosed notice which you should read.

This letter is not a Flood Risk Assessment. The information supplied can be used to form part of your Flood Risk Assessment. Further advice and guidance regarding Flood Risk Assessments can be found on our website at:

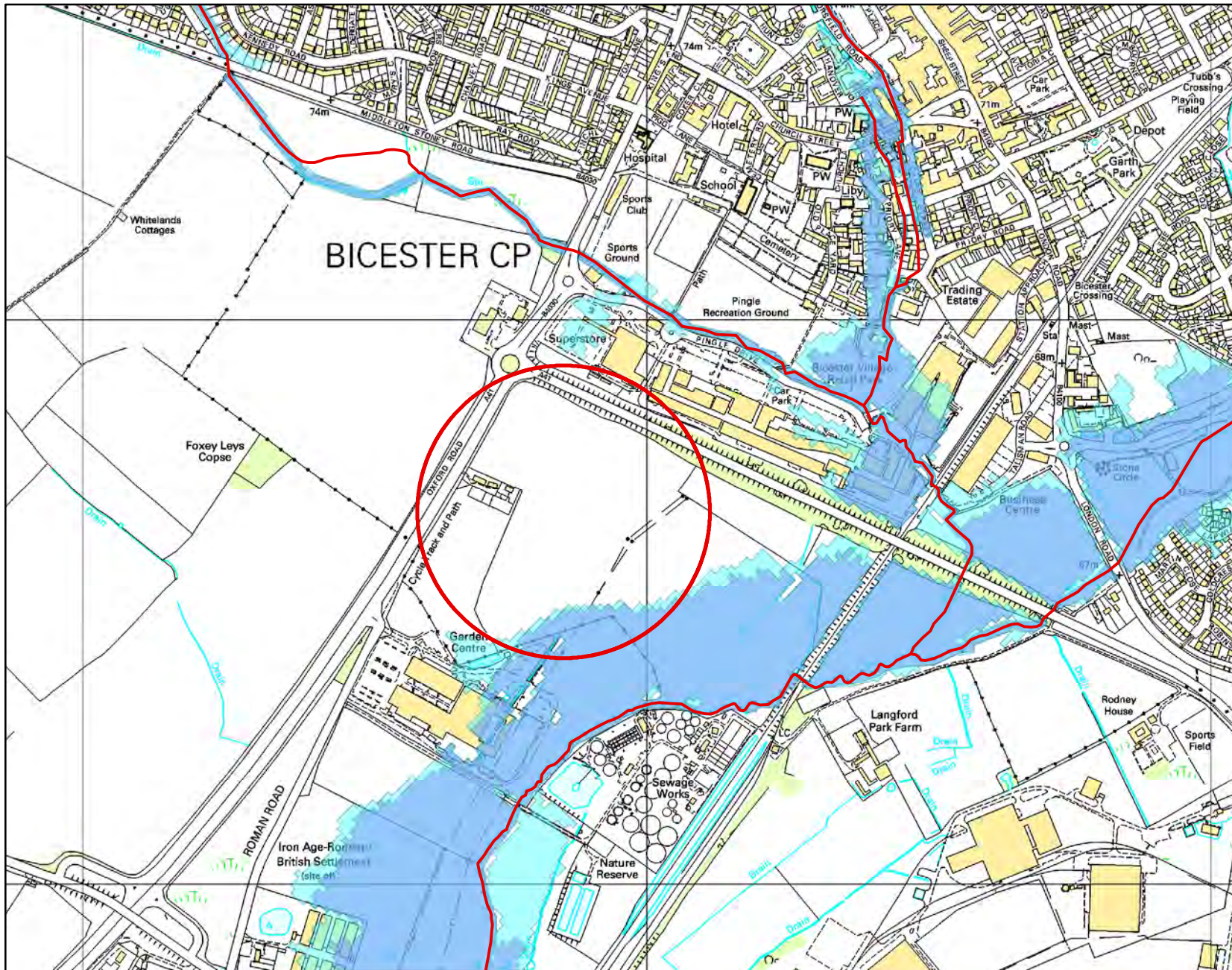
<https://www.gov.uk/guidance/flood-risk-assessment-local-planning-authorities>

If you would like advice from us regarding your development proposals you can complete our pre application enquiry form which can be found at:

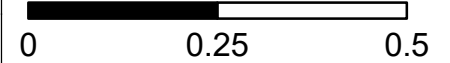
<https://www.gov.uk/government/publications/pre-planning-application-enquiry-form-preliminary-opinion>

Flood Map for Planning centred on Lakeview Drive Bicester OX26 1DE

Created on 23/05/17 REF: THM48041



Kilometres



Legend

- Main River
- Flood defences
- Areas benefiting from flood defences
- Flooding from rivers or sea (FZ3)
- Extent of extreme flood (FZ2)
- Flood Map - flood storage areas

Flooding from rivers or sea without defences (Flood Zone 3) shows the area that could be affected by flooding:

- from the sea with a 1 in 200 or greater chance of happening each year
- or from a river with a 1 in 100 or greater chance of happening each year.

The Extent of an extreme flood (Flood Zone 2) shows the extent of an extreme flood from rivers or the sea with up to a 1 in 1000 chance of occurring each year.

Defence information

Defence Location: No defences on Main River

Description: This location is not currently protected by any formal defences and we do not currently have any flood alleviation works planned for the area. However we continue to maintain certain watercourses and the schedule of these can be found on our internet pages.

Model information

THM48041

Model: Langford Brook (Bicester) & Pingle-Back-Bure 2010

Description: The information provided is from the Langford Brook (Bicester) & Pingle-Back-Bure 2010 detailed mapping project. The study was carried out using 2D modelling software (ISIS-Tuflow).

Model design runs:

1 in 5 / 20% Annual Exceedance Probability (AEP); 1 in 20 / 5% AEP; 1 in 50 / 2% AEP; 1 in 100 / 1% AEP; 1 in 100+20% / 1% AEP plus 20% increase in flows and 1 in 1000 / 0.1% AEP

Mapped Outputs:

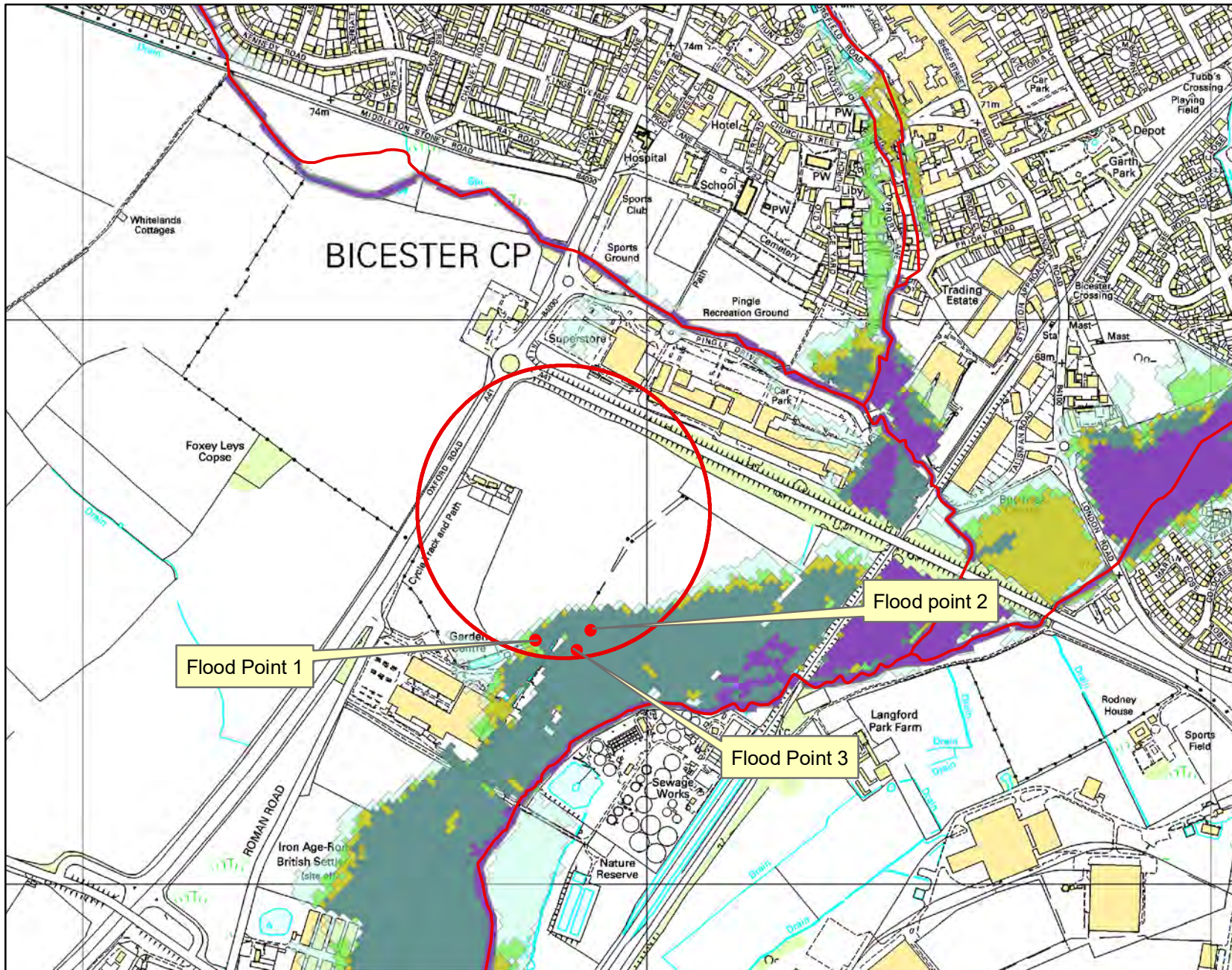
1 in 5 / 20% AEP; 1 in 20 / 5% AEP; 1 in 50 / 2% AEP; 1 in 100 / 1% AEP and 1 in 1000 / 0.1% AEP

Model accuracy:

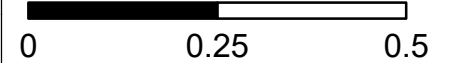
Levels \pm 250mm

Detailed FRA centred on Lakeview Drive Bicester OX26 1DE

Created on 23/05/17 REF: THM48041



Kilometres



Legend

- Main River
- 20% AEP
- 5% AEP
- 2% AEP
- 1% AEP
- 0.1% AEP

AEP = Annual Exceedance Probability
The probability of a flood of a particular magnitude, or greater, occurring in any given year

Modelled floodplain flood levels

THM48041

The modelled flood levels for the closest most appropriate model grid cells for your site are provided below:

2D grid cell reference	Model	Easting	Northing	flood levels (mAOD)								
				20% AEP	5% AEP	2% AEP	1% AEP	1% AEP (+20% increase in flows)	1% AEP (+25% increase in flows)	1% AEP (+35% increase in flows)	1% AEP (+70% increase in flows)	0.1% AEP
Flood Point 1	Langford Brook (Bicester) & Pingle-Back-Bure 2010	457,806	221,434		64.66	64.70	64.74	64.78				64.85
Flood Point 2	Langford Brook (Bicester) & Pingle-Back-Bure 2010	457,904	457,904		64.67	64.72	64.76	64.80				64.90
Flood Point 3	Langford Brook (Bicester) & Pingle-Back-Bure 2010	457,876	221,413		64.64	64.70	64.73	64.78				64.86

This flood model has represented the floodplain as a grid.
The flood water levels have been calculated for each grid cell.

Note:
Due to changes in guidance on the allowances for climate change, the 20% increase in river flows should no longer be used for development design purposes. The data included in this Product can be used for interpolation of levels as part of an intermediate level assessment.

For further advice on the new allowances please visit
<https://www.gov.uk/guidance/flood-risk-assessments-climate-change-allowances>

Historic flood data

THM48041

Our records show that the area of your site has been affected by flooding.
Information on the floods that have affected your site is provided in the table below:

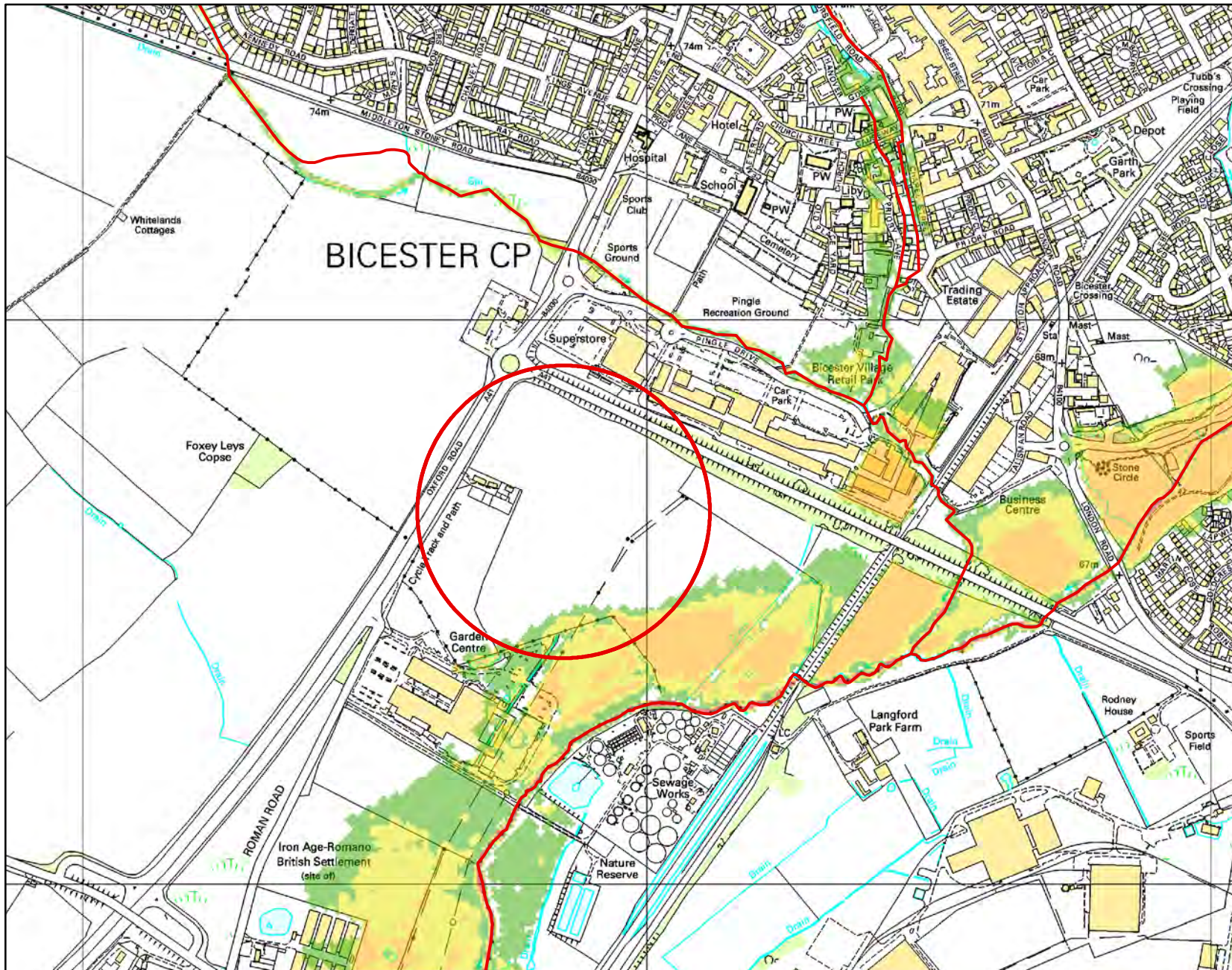
Flood Event Code	Flood Event Name	Start Date	End Date	Source of Flooding	Cause of Flooding
We hold no records of historic flooding for this location					

Please note the Environment Agency maps flooding to land not individual properties. Floodplain extents are an indication of the geographical extent of a historic flood. They do not provide information regarding levels of individual properties, nor do they imply that a property has flooded internally.

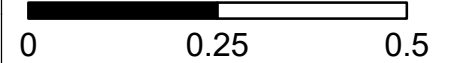
Start and End Dates shown above may represent a wider range where the exact dates are not available.

Hazard Map centred on Lakeview Drive Bicester OX26 1DE

Created on 23/05/17 REF: THM48041



Kilometres



Legend

- Main River
- Very low hazard
- Danger for some
- Danger for most
- Danger for all

For hazard and debris factor we used HR Wallingford and Environment Agency (May 2008) supplementary note on flood hazard ratings and thresholds for development planning and control purpose. The following calculation is used:
 $HR = d \times (v+0.5) + DF$
HR = flood hazard rating
d = depth of flooding (m)
v = velocity of floodwaters (m/sec)
DF = debris factor calculated (0, 0.5, 1 depending on probability that debris will lead to a hazard)

Hazard Mapping

Hazard Mapping methodology:

To calculate flood hazard with the debris factor we have used the supplementary note to Flood Risk to People Methodology (see below).

The following calculation is used:

$$HR = d \times (v+0.5) + DF$$

Where HR = flood hazard rating

d = depth of flooding (m)

v = velocity of floodwaters (m/sec)

DF = debris factor calculated (0, 0.5, 1 depending on probability that debris will lead to a hazard)

The resultant hazard rating is then classified according to:

Flood Hazard	Colour	Hazard to People Classification
Less than 0.75	Green	Very low hazard - Caution
0.75 to 1.25	Yellow	Danger for some - includes children, the elderly and the infirm
1.25 to 2.0	Orange	Danger for most - includes the general public
More than 2.0	Red	Danger for all - includes the emergency services

REF: HR Wallingford and Environment Agency (May 2008) Supplementary note of flood hazard ratings and thresholds for development planning and control purpose – Clarification of the Table 113.1 of FD2320/TR2 and Figure 3.2 of FD2321/TR1

Thames Area Climate Change Allowances

Guidance for their use in flood risk assessments

Jan 2017

We recently updated our national guidance on climate change allowances for Flood Risk Assessments. The following information provides additional local guidance which applies to developments within our Thames area boundary.

Climate change allowances - overview

The National Planning Practice Guidance refers planners, developers and advisors to the Environment Agency to our guidance on considering climate change in Flood Risk Assessments. We updated this guidance in February 2016 and it should be read in conjunction with this document to inform planning applications, local plans, neighbourhood plans and other projects. It provides:

- Climate change allowances for peak river flow, peak rainfall, sea level rise, wind speed and wave height
- A range of allowances to assess fluvial flooding, rather than a single national allowance
- Advice on which allowances to use for assessments based on vulnerability classification, flood zone and development lifetime

Updated climate change allowances guidance:

<https://www.gov.uk/guidance/flood-risk-assessments-climate-change-allowances>

National Planning Practice Guidance:

<http://planningguidance.communities.gov.uk/>

Assessing climate change impacts on fluvial flooding

Table A below indicates the level of technical assessment of climate change impacts on fluvial flooding appropriate for new developments depending on their scale and location (flood zone). Please note that this should be used as a guide only. Ultimately, the agreed approach should be based on expert local knowledge of flood risk conditions, local sensitivities and other influences.

Applicants and consultants may contact the Environment Agency at the pre-planning application stage to confirm the assessment approach on a case-by-case basis. We provide standard guidance free of charge or bespoke advice for a fee for developments for which we are a statutory consultee. If your development is instead covered by Flood Risk Standing Advice, we recommend you contact the relevant Local Planning Authority for their guidance and confirmation of the assessment approach. Flood Risk Standing Advice can be found here:

<https://www.gov.uk/flood-risk-assessment-local-planning-authorities>

Table A defines three possible approaches to account for flood risk impacts due to climate change in new development proposals:

1. **Basic** - Developer can add an allowance to the 'design flood' (i.e. 1% annual probability) peak levels to account for potential climate change impacts. The allowance should be derived and agreed locally by Environment Agency teams.
2. **Intermediate** - Developer can use existing modelled flood and flow data to construct a stage-discharge rating curve, which can be used to interpolate a flood level based on the required peak flow allowance to apply to the 'design flood' flow.
3. **Detailed** - Perform detailed hydraulic modelling, through either re-running Environment Agency hydraulic models (if available) or construction of a new model by the developer.

Table A – Indicative guide to assessment approach

Vulnerability classification	Flood zone	Assessment by development type		
		Minor	Small-Major	Large-Major
Essential infrastructure	Zone 2	Detailed		
	Zone 3a	Detailed		
	Zone 3b	Detailed		
Highly vulnerable	Zone 2	Intermediate/Basic	Intermediate/Basic	Detailed
	Zone 3a	Not appropriate development		
	Zone 3b	Not appropriate development		
More vulnerable	Zone 2	Basic	Basic	Intermediate/Basic
	Zone 3a	Basic	Detailed	Detailed
	Zone 3b	Not appropriate development		
Less vulnerable	Zone 2	Basic	Basic	Intermediate/Basic
	Zone 3a	Basic	Basic	Detailed
	Zone 3b	Not appropriate development		
Water compatible	Zone 2	None		
	Zone 3a	Intermediate/Basic		
	Zone 3b	Detailed		

Definitions of terms in Table A

Minor

1-9 dwellings/less than 0.5 ha; office/light industrial under 1ha; general industrial under 1 ha; retail under 1 ha; travelling community site between 0 and 9 pitches.

Small-Major

10 to 30 dwellings; office/light industrial 1ha to 5ha; general industrial 1ha to 5ha; retail over 1ha to 5ha; travelling community site over 10 to 30 pitches.

Large-Major

30+ dwellings; office; light industrial 5ha+; general industrial 5ha+; retail 5ha+; gypsy/traveller site over 30+ pitches; any other development that creates a non-residential building or development over 1000 sqm.

Further info on vulnerability classifications:

<http://planningguidance.communities.gov.uk/blog/guidance/flood-risk-and-coastal-change/flood-zone-and-flood-risk-tables/table-2-flood-risk-vulnerability-classification/>

Further info on flood zones:

<http://planningguidance.communities.gov.uk/blog/guidance/flood-risk-and-coastal-change/flood-zone-and-flood-risk-tables/table-2-flood-risk-vulnerability-classification/>

Specific local considerations

Where the Environment Agency and the applicant or their consultant has agreed that a basic level of assessment is appropriate, the figures in Table B below can be used as an allowance for potential climate change impacts on peak design (i.e. 1% annual probability) fluvial flood level rather than undertaking detailed modelling.

Table B – Local allowances for potential climate change impacts

Watercourse	Central	Higher central	Upper
Thames	500mm	700mm	1000mm

Use of these allowances will only be accepted after discussion with the Environment Agency.

Fluvial food risk mitigation

Please use the [national guidance](#) to find out which allowances to use to assess the impact of climate change on flood risk.

For planning consultations where we are a statutory consultee and our [Flood Risk Standing Advice](#) does not apply, we use the following benchmarks to inform flood risk mitigation for different vulnerability classifications.

These benchmarks are a guide only. We strongly recommend you contact us at the pre-planning application stage to confirm this on a case-by-case basis. Please note you may be charged for pre-planning advice.

For planning consultations where we are not a statutory consultee or where our Flood Risk Standing Advice does apply, we recommend local planning authorities and developers use these benchmarks but we do not expect to be consulted.

Essential Infrastructure

For these developments, our benchmark for flood risk mitigation is for it to be designed to the **upper end** climate change allowance for the epoch that most closely represents the lifetime of the development, including decommissioning.

Highly Vulnerable

For these developments in flood zone 2, the **higher central** climate change allowance is our minimum benchmark for flood risk mitigation. In sensitive locations it may be necessary to use the **upper end** allowance.

More Vulnerable

For these developments in flood zone 2, the **central** climate change allowance is our minimum benchmark for flood risk mitigation. In flood zone 3 the **higher central** climate change allowance is our minimum benchmark for flood risk mitigation. In sensitive locations it may be necessary to use the **higher central** (in flood zone 2) and the **upper end** allowance (in flood zone 3).

Water Compatible or Less Vulnerable

For these developments, the **central** climate change allowance for the epoch that most closely represents the lifetime of the development is our minimum benchmark for flood risk mitigation. In sensitive locations it may be necessary to use the **higher central** to inform built in resilience, particularly in flood zone 3.

Further info on our Flood Risk Standing Advice:

<https://www.gov.uk/guidance/flood-risk-assessment-local-planning-authorities>

There may be circumstances where local evidence supports the use of other data or allowances. Where you think this is the case we may want to check this data and how you propose to use it.

For more information

Please contact our Thames area Customers and Engagement team:

Enquiries_THM@environment-agency.gov.uk

Ms Clare Jones
Buro Happold Ltd.
Infrastructure Water
17 Newman Street
London
W1T 1PD

Our ref: ENVPAC/WTHAMS/00432
(WA/2017/124029/01-L01)

Date: 27 June 2017

Dear Ms Jones

The proposed development, includes the construction of a business park comprising between 55,000 and 60,000m² office use (B1), parking for approximately 2,000 cars, associated highway, infrastructure and earthworks.

Bicester Office Park, Oxfordshire, OX26 1DE

Thank you for consulting us. We received confirmation to proceed with the work on 22 June and we are now in a position to respond.

We have reviewed the following documents:

- Emails from Clare Jones (Buro Happold), dated 02, 22, 27 June 2017
- Pre-application Enquiry Form
- Draft EIA Scoping Report produced by TRIUM Environmental, dated 15 May 2017
- Drawing 1105(SK)058 Rev A – Site Plan
- Drawing 1105(SK)065 Rev B – Parking Provision
- Drawing WSKL001 Rev 01 – Flood Extents 2017
- Drawing WSKL002 Rev 01 – 2007 and 2017 Flood Extents
- Drawing WSKL003 Rev 00 – Flood Extents Derived From Topographic Levels
- Drawing WSKL004 Rev 00 – Flood Extents Derived From 2011 LiDAR Data

We have reviewed the draft EIA Scoping report in relation to Flood Risk only as confirmed under our charging agreement. We disagree that the Flood Risk topic area should be scoped out of the EIA. Flood risk to this site is surely one of the most significant environmental impacts affecting this site and therefore should warrant assessment within the EIA. The reasoning given within the Scoping Report for scoping out this topic is frankly misinterpreting the level of risk on site. It fails to acknowledge that there are areas of this site at the highest level of flood risk (Flood Zones 3a and 3b). We would therefore be likely to object if an EIA was submitted for this site that did not include a chapter on flood risk.

We can confirm that the site is affected by the 1 in 20 year modelled flood extent and we consider this to be the functional floodplain (Flood Zone 3b). In normal circumstances we would not accept development of this type in areas at this high risk.

Cont/d..

However, this site has been allocated (Bicester 4) within the Cherwell District Council Local Plan and has been sequentially tested. We therefore have no in principle objection on flood risk grounds to this site coming forward for development.

To ensure that Policy Bicester 4 clearly states that a sequential approach should be followed and that where possible buildings should be located away from the highest risk of flooding. We are pleased to see from the drawings you have provided that no buildings are proposed within the 1 in 20 (functional floodplain) extents. We would accept car parking within this area of highest risk providing that there was no raising of ground levels.

However, we would expect that a sequential approach is taken to ensure that no built development is located in areas up to the 1 in 100 year plus climate change (plus 35%) flood level. We note that you have carried out an intermediate assessment to establish a new climate change level and then mapped it on a topographic survey. This shows buildings located in the 1 in 100 year plus climate change (plus 35%) flood extent which we feel is not in line with the principles of Bicester Policy 4.

We strongly advise that any master plan is re-orientated so that there is no built development or ground raising in areas within the 1 in 100 year plus climate change (plus 35%) flood extent. There appears to be plenty of car parking in areas at much lower risk and so we see no need to place any buildings within this area of risk.

We also have concerns that the 1 in 100 year plus climate change (plus 35%) flood level has been established by using the intermediate approach. Please find attached the Thames Climate Change Guidance which clearly states that a detailed assessment is required for 'Large-Major' development in Flood Zone 3a or 3b.

In summary, the scoping report is inadequate as it fails to represent the true level of flood risk affecting this site and makes recommendations that are flawed. The 1 in 100 year plus climate change flood level needs to be established by carrying out a detailed assessment as outlined in our guidance. The site must be developed in accordance with the principles as set out in Bicester Policy 4. This clearly stipulates that built development should be located in areas of the site at least flood risk.

Yours sincerely,

Mr Jack Moeran
Planning Specialist

Direct dial 02030259655

Direct e-mail planning-wallingford@environment-agency.gov.uk

Clare Jones

From: Moeran, Jack ·
Sent: 24 July 2017 14:20
To: Clare Jones
Subject: RE: THM48041 Product 4 Bicester Office Park, Oxfordshire,OX26 1DE

** External E-Mail **

Hi Clare,

Yes I'm happy that this is an accurate reflection of our conversation.

One point I would just like to clarify is the following:

- The EA confirmed it was acceptable to have car parking with Functional Floodplain (1 in 20 year extent), providing it wasn't increasing the level of 'use vulnerability' from what is existing and that there was no ground raising.

Thanks,

Jack Moeran
Planning Specialist

[FCRM Planning Specialist - PSO - Thames Area](#)

From: Clare Jones
Sent: 27 June 2017 17:13
To: 'Moeran, Jack' ·

Subject: RE: THM48041 Product 4 Bicester Office Park, Oxfordshire,OX26 1DE

Jack,

Thank you for your quick response to the pre-application enquiry. As a record of our earlier conversation today, please find below a summary of the items discussed:

EIA Water Chapter

- The EA has confirmed that they will require an EIA Water Chapter to be written for the site to accompany the Flood Risk Assessment for the Outline Planning Application. They explained that a site lying in Flood Zone 3(a and b) would be considered to be a significant environmental effect which would need to be assessed under an EIA. The EA advised that whilst an FRA was proposed, they would also expect to see the EIA Water Chapter.

Flood Extents

- The EA confirmed that the approach taken to define the flood extents for the 1 in 20, 1 in 100 and 1 in 1000 year using the flood levels against the topographic survey information was acceptable. Whilst the topographic survey information was available for most of the site, BHE explained that there was an area to the west where topo survey information was not available. The EA confirmed that it was acceptable to use LiDAR to define the flood extents in this area and to combine this with the flood extent derived from the topographic survey, provided this was explained on the drawings.
- The EA confirmed that they require hydraulic modelling to be undertaken to define the flood levels for the 1 in 100 + 25% and 1 in 100 + 35% climate change events. The same approach of deriving the flood extents based on

the topographic survey should be adopted. For the hydraulic modelling, the EA would expect to see an appendix to the FRA detailing the method adopted for the modelling and the results with a short summary in the FRA.

- The EA recommended that the flood extent plans submitted are overlaid with the parameter plan rather than the illustrative masterplan which could change in the future.

Development in the Flood Zones

- The EA confirmed it was acceptable to have car parking with Functional Floodplain (1 in 20 year extent), provided that there was no ground raising.
- The EA would seek that all buildings were located outside the 1 in 100 year + 35% extent.
- If ground raising is required between the 1 in 20 year and 1 in 100 year + climate change level (25%) then floodplain compensation would be required. BHE advised that parameter plans would be submitted for Outline planning and the need for flood compensation would not be known until detailed design. The EA agreed that this could be dealt with at a later date through a planning condition.

Finished Floor Levels

- The EA confirmed that the design flood event for the site is the 1 in 100 year +25% climate change event. The EA anticipate that the levels will be very close to the 1 in 100 year + 35% climate change event. The EA would seek that we adopt the 1 in 100 year +35% level with 300mm freeboard to define finished floor levels. The EA would review this if there was a significant difference in levels between the 1 in 100 year +25% climate change and 1 in 100 year +35% levels.

We would appreciate if you can review the above and confirm if this is an accurate record of the conversation.

Kind Regards
Clare

Clare Jones CEng MICE
Senior Engineer
BuroHappold Engineering | Water
T: +44 (0)1225 320600
www.burohappold.com | [@burohappold](https://twitter.com/burohappold)

From: Moeran, Jack
Sent: 27 June 2017 12:05
To: Clare Jones
Subject: RE: THM48041 Product 4 Bicester Office Park, Oxfordshire,OX26 1DE

** External E-Mail **

Hi Clare,

Please find attached our response.

If you wish to chat any of the content through with me then please don't hesitate to give me a call.

Kind regards,

Jack Moeran
Planning Specialist

[FCRM Planning Specialist - PSO - Thames Area](#)

From: Clare Jones [<mailto:Clare.Jones@BuroHappold.com>]
Sent: 27 June 2017 10:04
To: Planning_THM <Planning_THM@environment-agency.gov.uk>

Subject: RE: THM48041 Product 4 Bicester Office Park, Oxfordshire,OX26 1DE

Jack,

Further to my email below, please find attached drawings showing the flood extents derived from survey data from both topographic survey information (2007) and LiDAR Data (2011, 1m resolution) with the illustrative masterplan. We would propose to refine the modelled flood extents to those defined from the topographic survey information. Unfortunately as the topo survey does not cover a section west of the site so for this section, we would propose to defer back to the LiDAR contour. The methodology for defining the flood extents is summarized below. We are intending to write this up in more detail for the FRA but before we do, we would appreciate the EA's view on this methodology. Also attached, are drawings showing the current EA extents overlaid with the illustrative masterplan and showing the flood extents from the 2007 OPA FRA for the site for information.

The flood extents have been derived by the following means:

- Flood model level information has been extracted from the Langford Brook (Bicester) & Pingle-Back- Bure 2010 ISIS-TUFLOW Model for Points A to G in the floodplain. It has been assumed that the levels within the floodplain are the same as within the corresponding point in the river channel. Using 3D modelling software, a flood level surface for each return period event has been created by interpolating between the flood level points defined in the floodplain and the channel.
- The survey information used (topographic survey or LiDAR) has been used to create a ground level surface by interpolating between the LiDAR contours/ topographic survey points.
- 3D modelling software has then been used to determine where the flood level meets the ground level surface. The model has defined a contour for each of the flood level extents which is provided on the attached drawings.

In addition to the 1 in 20, 1 in 100 and 1 in 1000 flood extents, the climate change allowance has been calculated. In accordance with February 2016 climate change guidance, for office developments (defined as Less Vulnerable) in Flood Zone 3a, the central and higher central allowances are required to be assessed. For the Thames region, this would require the 25% and 35% climate change allowances to be considered. We have defined these using the Intermediate approach as follows:

1. 1D flood levels and flows have been extracted out of the Langford Brook (Bicester) & Pingle-Back- Bure 2010 ISIS-TUFLOW Model for Points LA.0865, LA.0957 and LA.1350 for the 1 in 5, 1 in 20, 1 in 100, 1in 100+20% and 1 in 1000
2. The above flow (Q) data was plotted against flood level (H) data and a line of best fit derived.
3. For the 1 in 100 year + 25% and 1 in 100 year + 35%, the flood flows were calculated using the below relationship, with the 35% value used as an example:

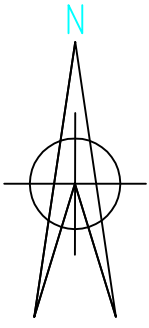
$$\left(\frac{100\text{yr}+20\%CC - 100\text{yr}}{20} \times (35 - 20) \right) + 100\text{yr}+20\%CC$$

4. Using the line of best fit HQ relationship, the flood levels for the 1D flood levels at A, B, C and G for the 1 in 100 year + 25% and 1 in 100 year + 35% have been calculated.
5. The 1 in 100 year + 25% and 1 in 100 year + 35% in the 2d domain have then been calculated based on scaling the level differences between the 1d and 2d domains from the other return periods.
6. For the remaining points (i.e. D,E and F, the levels have been interpolated)


Hopefully the above illustrates that we have taken an appropriate approach to defining the flood extents in this location. Please give me a call if you have any queries.

Kind Regards,

Appendix D Flood Extents Drawing



POINT ID	X	Y	1 in 20 (mAOD)	1 in 100 (mAOD)	1 in 100 + 25%CC (mAOD)	1 in 100 + 35%CC (mAOD)	1 in 1000 (mAOD)
A	458310.6	221537.4	64.91	64.94	64.95	64.96	64.99
B	458224.2	221529.9	N/A	N/A	64.82	64.84	64.89
C	458161.6	221516.1	N/A	64.77	64.81	64.83	64.87
D	458087.7	221502.4	N/A	N/A	N/A	64.80	64.85
E	457863.6	221468.6	64.51	64.58	64.63	64.65	64.70
F	457695.8	221400.9	N/A	N/A	64.61	64.63	64.67
G	458256.2	221457.9	64.90	64.93	64.95	64.96	64.99
H	458057.7	221375.2	64.64	64.72	64.76	64.78	64.82
I	457895.8	221372.8	64.46	64.56	64.62	64.64	64.69

- SITE BOUNDARY
-  FLOOD LEVEL DATA POINT
- 2018 FLOOD EXTENTS**
- 1 IN 20 YEAR FLOOD EXTENT
- 1 IN 100 YEAR FLOOD EXTENT
- 1 IN 1000 YEAR FLOOD EXTENT
- 1 IN 100 YEAR +25%CC FLOOD EXTENT
- 1 IN 100 YEAR +35%CC FLOOD EXTENT

Notes
 BACKGROUND OS TILE Ref. 10019980
 COMBINED LIDAR (2M RESOLUTION) DIGITAL TERRAIN MODEL DOWNLOADED FROM <http://environment.data.gov.uk>. CONTAINS PUBLIC SECTOR INFORMATION LICENSED UNDER THE OPEN GOVERNMENT LICENCE v3.0.
 TOPOGRAPHIC SURVEY BY GREENHATCH (FEBRUARY 2018)
 ARCHITECT PARAMETER PLAN Ref. 1105_P_005
 FLOOD LEVEL INFORMATION DERIVED FROM THE BICESTER OFFICE PARK 2018 HYDRAULIC MODEL CREATED BY BUROHAPPOLD BASED ON THE ENVIRONMENT AGENCY'S PRODUCT 6 DATA JUNE 2017 (LANGFORD_BROOK_BICESTER_ & PRINGLE BACK BURE_2010 MODEL). CONTAINS ENVIRONMENT AGENCY INFORMATION © ENVIRONMENT AGENCY AND/OR DATABASE RIGHT.

THE FLOOD EXTENTS HAVE BEEN DERIVED BASED ON THE 2018 TOPOGRAPHIC SURVEY INFORMATION WHERE THIS EXISTS AND THE LIDAR DIGITAL TERRAIN MODEL WHERE THE TOPOGRAPHIC SURVEY DOES NOT EXIST.

06 UPDATED FLOOD EXTENTS FROM MODEL / 29.03.18	SM/CJ
05 MINOR CHANGES TO FLOOD EXTENTS / 29.11.17	SM/CJ
04 UPDATED FLOOD EXTENTS FOR SM/CJ NEW TOPO SURVEY / 17.10.17	
03 AMENDED PROJECT NAME INFORMATION / 12.09.17	SM/CJ
Rev Description/Date	Drn/Chk

INFORMATION

Status of drawing

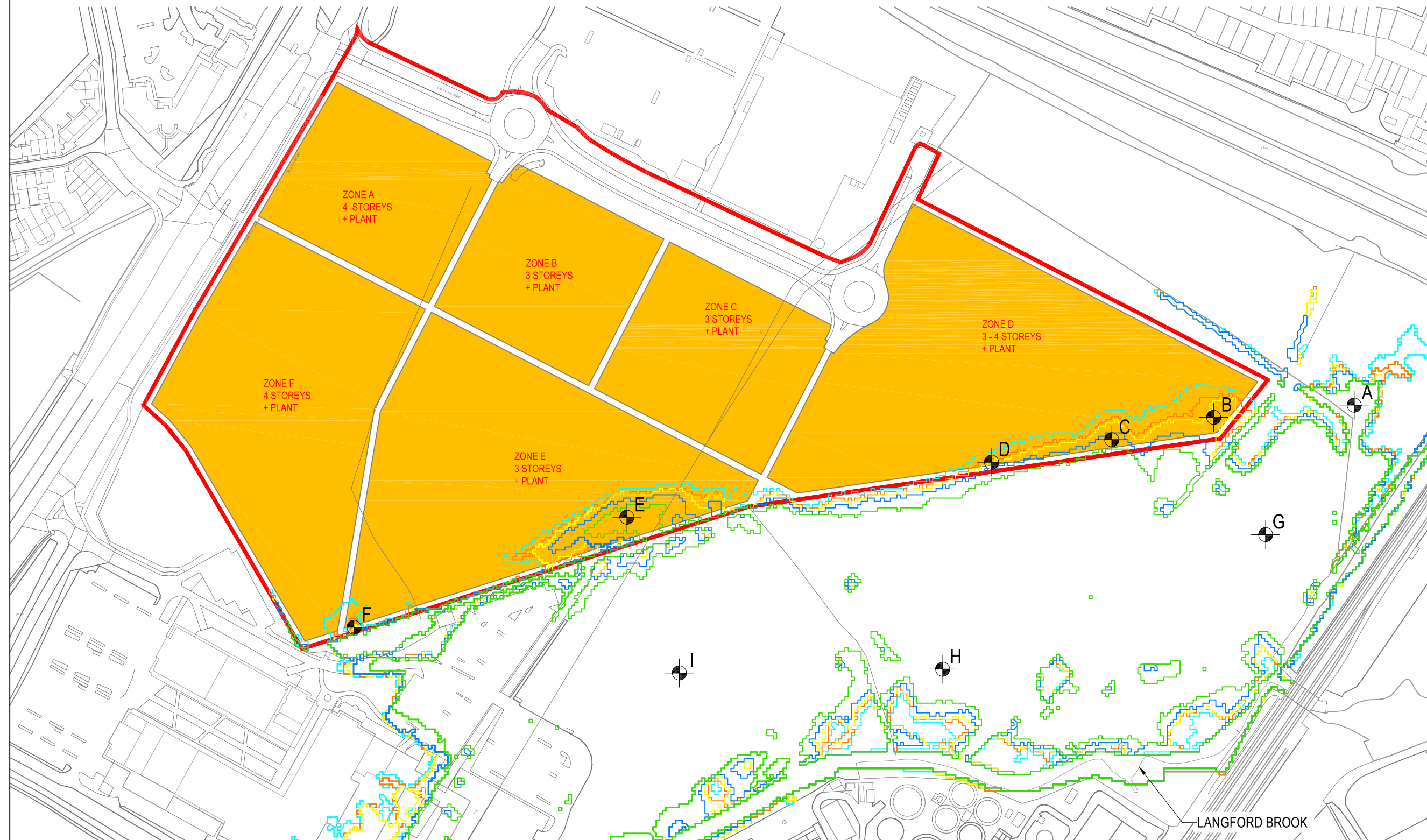
**BUROHAPPOLD
ENGINEERING**

Camden Mill Lower Bristol Road Bath BA2 3DQ UK
 Tel: +44 (0)1225 320600 Fax: +44 (0)870 787 4148 Email: 0040031@burohappold.com Web: www.burohappold.com

Architect **Bennett Associates**
 Project **Bicester Office Park**
 Drg Title **FLOOD EXTENTS DERIVED FROM COMBINED LIDAR AND 2018 TOPO SURVEY**

Scales@A3 1:2500
 Drawn by SM
 Checked by CJ
 Date JULY 2017

Job No. **0040031**
 Drg No. **WSK005**
 Rev **06**




Simon Mudd \lsrv-london05\CAD\0040031 Bicester Business Park- Planning Support\F34 WaterSheet\Sketches\WSK005.dwg

Appendix E Hydraulic Modelling Note

Design Note

Project Bicester Office Park
 Subject Hydraulic Modelling Note for updated topographic survey
 Project no 0040031
 Date 9 April 2018

Revision	Description	Issued by	Date	Approved (signature)
00	For Planning	NV	09/04/18	

1 Introduction

In December 2017 Scenic Land Developments Ltd. submitted an Outline Planning Application (Planning Ref 17/02534/OUT) for new office buildings and car parking at Bicester Office Park, south of Bicester (NGR SP579 215). To support the planning application, BuroHappold Engineering (BuroHappold) completed a Flood Risk Assessment (FRA) for the proposed site.

Since submission of the planning application, BuroHappold has undertaken hydraulic flood modelling for the site. This has involved updating the EA's 2010 hydraulic model with site specific topographic survey data to better inform the fluvial flood risk at the site.

BuroHappold has produced this Technical Note to provide details of the hydraulic modelling and the flood levels estimated at the site. The site specific flood level information will be used to update the FRA issued for Planning in December 2017.

This TN has been issued to the Environment Agency (EA) for the purposes of gaining their formal approval to adopt the flood levels and flood extents produced from the site specific hydraulic model within the Flood Risk Assessment. Once formal EA approval has been provided this information will be used to inform the Bicester Office Park design proposals.

1.1 Hydraulic Modelling Summary

The table below provides a summary of the hydraulic flood models used in the assessment and the main differences between them.

Model Description	Model Reference Name in Report	Comments
EA Langford_Brook_Bicester_& _Pingle-Back-Bure_2010	EA 2010 model	- ISIS-TUFLOW Model issued to BH 31/05/17 - Final Design Model Ref Name: 'Bicester_012_17hr' - Model simulated for the 1 in 5yr, 20yr, 50yr, 100yr, 100yr+20%CC and 1000yr flood events.

		<ul style="list-style-type: none"> - Based on survey data undertaken February to April 2007 and 2003 LiDAR data. - Simulated using a 10m x 10m grid resolution - ISIS-TUFLOW 1D-2D model (ISIS version 3.1 and TUFLOW version 2008-08-AH-iSP).
EA 2010 model with climate change	BH 2017 model	<ul style="list-style-type: none"> - Model inflows increased to simulate the 1 in 100yr+25%CC and 1 in 100yr+35%CC events (see BH Technical Note in Appendix A). - Simulated using a 10m x 10m grid resolution - ISIS-TUFLOW 1D-2D model (ISIS version 3.6 and TUFLOW version 2016-03-AE-iSP-w64).
Bicester Office Park (BOP) model	BH BOP 2018 model	<ul style="list-style-type: none"> - Model updated to include the following: <ul style="list-style-type: none"> - Modified 2D boundary - Topographic Survey undertaken by GreenHatch Group issued on 28/09/17 – used to represent 2D floodplain within site boundary. - Topographic Survey undertaken by GreenHatch Group issued on 19/02/18 – used to represent 1D in-channel cross sections and 2D floodplain between the site and Langford Brook. - Most recent available LiDAR (combined dataset from 2003 and 2011 from data.gov.uk) - Simulated using a 2m x 2m grid resolution rather than 10m x 10m. - Model simulated for the 1 in 20yr, 100yr, 100yr+25%CC, 100yr+35%CC and 1000yr flood events. - ISIS-TUFLOW 1D-2D model (ISIS version 3.6 and TUFLOW version 2016-03-AB-iSP-w64).

Table 1.1 Summary of hydraulic models undertaken at the proposed site

1.2 Purpose of Hydraulic Modelling Study

Since the submission of the planning application in December 2017, further topographic survey has been undertaken for the site and the adjacent floodplain. To more accurately assess the flood risk at the proposed Bicester Office Park, the EA 2010 model has been updated to create a hydraulic model (BuroHappold BOP 2018 model) with the latest topographic survey information.

The predominant flood risk identified at the site is due to overtopping of the right bank of the Langford Brook after it passes through the railway embankment. The ground levels are generally flat between the site and the river (approximately 1 in 1000 gradient) and therefore, relatively sensitive to changes in flood level. A series of ditch networks and ponds surround the site, which may have an impact on the flood mechanism not currently represented in the EA 2010 model. As such, a higher resolution of the floodplain has been defined in the BuroHappold BOP 2018 model to better represent the ditches within the model and therefore the flood mechanism at the site. This is discussed further in section 2.1.

2 Hydraulic Modelling Methodology

2.1 Model Set-Up

As discussed in section 1.2, the purpose of the modelling was to more accurately depict the ditches and variations in topography in and around the site. The ditches surveyed varied in width between 3m and 6m. It was considered that a 2m x 2m grid resolution in the 2D domain would be more representative than the EA 2010 model's 10m x 10m resolution. In order to simulate the hydraulic model over an appropriate run time based on a higher resolution, a reduced 2D domain was required compared to the EA 2010 model which was based on a 10m x 10m grid resolution.

The A41, north of the site, and the railway line to the east of the site, form an embankment preventing overland flow reaching the site from upstream areas. Therefore, it was considered that the location at which the Langford Brook flows through the railway embankment, immediately upstream of the site, was an appropriate location to truncate the EA 2010 model and use as the start location for the site specific model.

Figure 2.1 below indicates the 2D domain extent in the EA 2010 model and the BuroHappold Bicester Office Park (BOP) 2018 model.

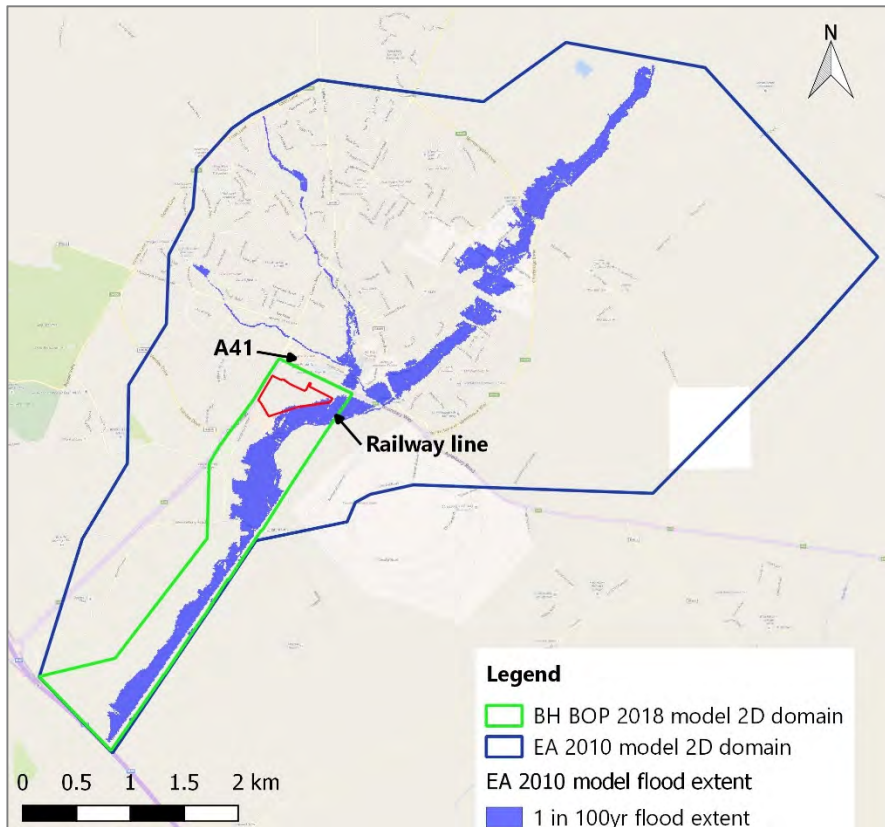


Figure 2.1 BuroHappold BOP 2018 model 2D domain extent compared to EA 2010 model 2D domain and 1 in 100 year flood extent. Site boundary shown in red.

2.2 Model Testing

Prior to creating the BuroHappold BOP 2018 model, BuroHappold undertook modelling test runs of the EA 2010 model and the truncated model to ensure suitability of use in the study. The table below provides a summary of the two models.

Model Description	Model Reference Name in Report	Comments
EA 2010 model re-run (test model)	BH re-run 2018 model	<ul style="list-style-type: none"> - EA model re-run on more recent software versions before site specific model created. - Model run to assess impact of software versions on results. - EA 2010 model run for 1 in 20yr event, and 1 in 100 year event. - Simulated using a 10m x 10m grid resolution - ISIS-TUFLOW 1D-2D model (ISIS version 3.6 and TUFLOW version 2016-03-AB-ISP-w64).
EA 2010 model truncated (test model)	BH truncated 2018 model	<ul style="list-style-type: none"> - EA model truncated at the railway embankment (NGR 458277, 221357) to reduce size of model extent. - A test simulation for the 1 in 20yr event and 1 in 100 year event created to compare to EA 2010 results. - Simulated using a 10m x 10m grid resolution - ISIS-TUFLOW 1D-2D model (ISIS version 3.6 and TUFLOW version 2016-03-AB-ISP-w64).

Table 2.1 Summary of elements included in test model runs

The BH truncated 2018 model (model C in Table 2.2) was set up based on the EA 2010 model set-up (model A in Table 2.2), except for the 2D domain which was reduced (as shown in Figure 2.1) and the corresponding 1D layers edited to enable the model to be simulated from the railway embankment. The BH truncated 2018 model was created to test

whether truncating the model would be appropriate to assessing any topographic changes at the site as part of the site specific model.

The model was also simulated using more up-to-date software versions, therefore a re-run of the EA 2010 model (model B in Table 2.2) was also undertaken to determine any effects on flood levels that may be caused by the software alone. Test runs were undertaken for both the 1 in 20 year event and 1 in 100 year event.

The results of the flood extents are shown below in Figure 2.2 and Table 2.2 below.

1 in 20yr event

ID Ref	EA 2010 model (mAOD) (A)	BH re-run 2018 model (mAOD) (B)	Diff of B from A (mm)	BH truncated 2018 model (C)	Diff of C from B (mm)
1D Node					
LA.1350D	65.268	65.273	5	65.275	2
LA.0957	64.548	64.540	-8	64.533	-7
LA.0865	64.438	64.430	-8	64.424	-6
LA.0767	64.302	64.294	-8	64.290	-4
LA.0737	64.221	64.217	-4	64.214	-2
LA.0726	64.217	64.213	-4	64.211	-2
2D Point					
1	64.832	64.821	-11	64.819	-2
2	64.666	64.649	-18	64.647	-1
3	65.026	65.014	-13	65.013	-1
4	64.817	64.810	-7	64.808	-2
5	64.368	64.372	4	64.372	1
6	64.636	64.624	-12	64.623	-1

1 in 100yr event

ID Ref	EA 2010 model (mAOD) (A)	BH re-run 2018 model (mAOD) (B)	Diff of B from A (mm)	BH truncated 2018 model (C)	Diff of C from B (mm)
1D Node					
LA.1350D	65.367	65.378	11	65.373	-5
LA.0957	64.632	64.622	-10	64.607	-15
LA.0865	64.526	64.525	0	64.512	-13
LA.0767	64.388	64.382	-6	64.369	-13
LA.0737	64.266	64.265	-1	64.258	-8
LA.0726	64.259	64.258	-1	64.251	-7
2D Point					
1	64.946	64.935	-12	64.925	-10
2	64.740	64.735	-5	64.725	-10
3	65.120	65.113	-7	65.104	-9
4	64.911	64.904	-6	64.893	-11
5	64.433	64.440	6	64.430	-10
6	64.720	64.721	1	64.710	-11

Table 2.2 Flood level data extracted from the EA 2010 model and compared to the re-run and truncated models (BH re-run 2018 model and BH truncated 2018 model respectively) for the 1 in 20 year event and 1 in 100 year event

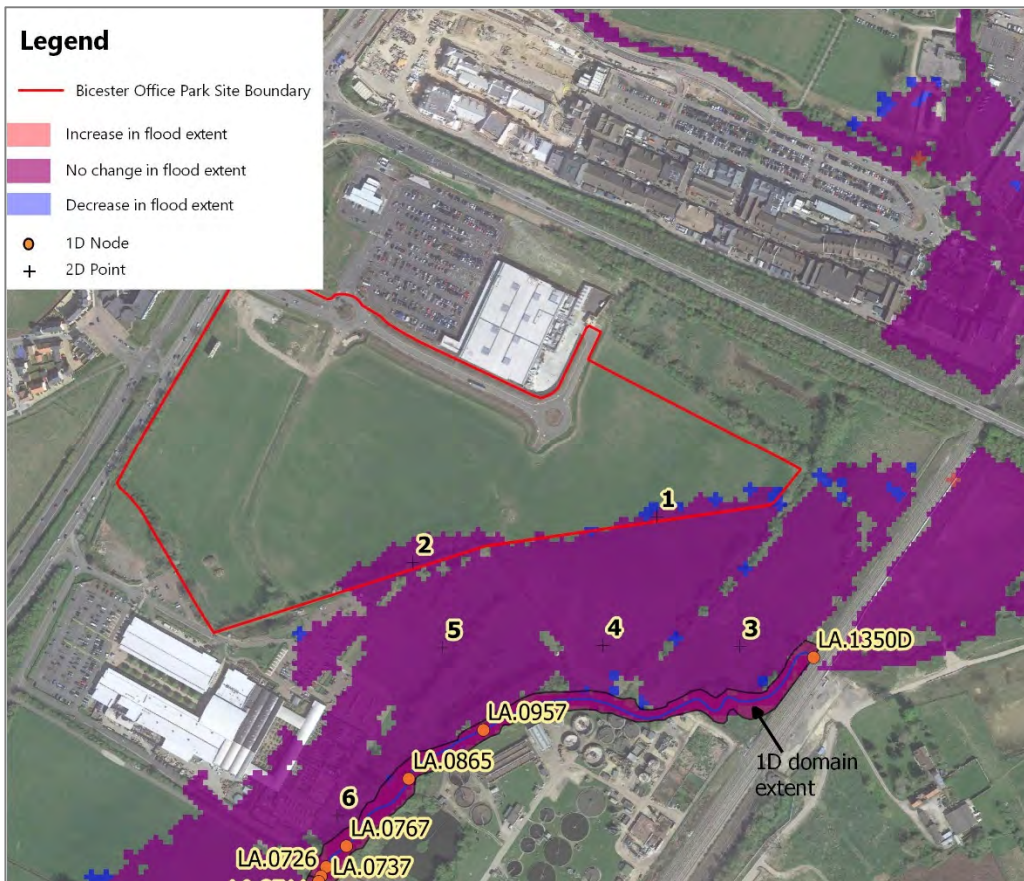


Figure 2.2 Result location map showing change in 1 in 20 year event flood extent between EA 2010 model and re-run of the model (BH re-run 2018 model). Result location points indicated.

The results indicate a minor change in flood level, less than 20mm when the EA 2010 model was re-run using more recent software (BH re-run 2018 model). Truncating the model at the railway embankment also showed minor changes in flood level, with changes in the 2D domain observed at less than 2mm. These variations in flood levels are considered to be within modelling tolerances.

Therefore, truncating the model is considered appropriate for the site specific hydraulic model.

2.3 Model Topography

The updated 2017 and 2018 topographic survey for the site was based on the following parameters:

- Elevation data recorded every 15m across the site
- Cross sections along the ditch network and pond recorded every 15m along the length, picking up top of bank and bed levels.
- 10 Nr cross sections surveyed within the river channel recording bed level and top of bank level.

Both the 1D and 2D components of the BuroHappold BOP 2018 model have been updated to better represent the ground conditions in the river and surrounding site. 3D point data was received by Greenhatch Group of the ground elevation levels. This data was read into 12d software and interpolated to derive a topographical surface. The data was stitched to the most recent 2m resolution filtered LiDAR data for the surrounding area to form a Digital Elevation Model (DEM) file that could be read into the TUFLOW software. The Composite Digital Terrain Model LiDAR data was obtained from the open source government website (<https://environment.data.gov.uk>). This is based on data collected from the area in the years 2003 and 2011. The LiDAR data was also used for the remaining area within the model (outside the site). Appendix B illustrates the surveyed topography and the extent of the LiDAR data used in proximity of the site.

The figure below provides an illustration of the differences between the survey data and the topographic data used in the EA 2010 model (based on a 2m resolution output). Parts of the floodplain between the site and the Langford Brook are lower by up to 200mm in the updated topographic survey compared to the LiDAR data.

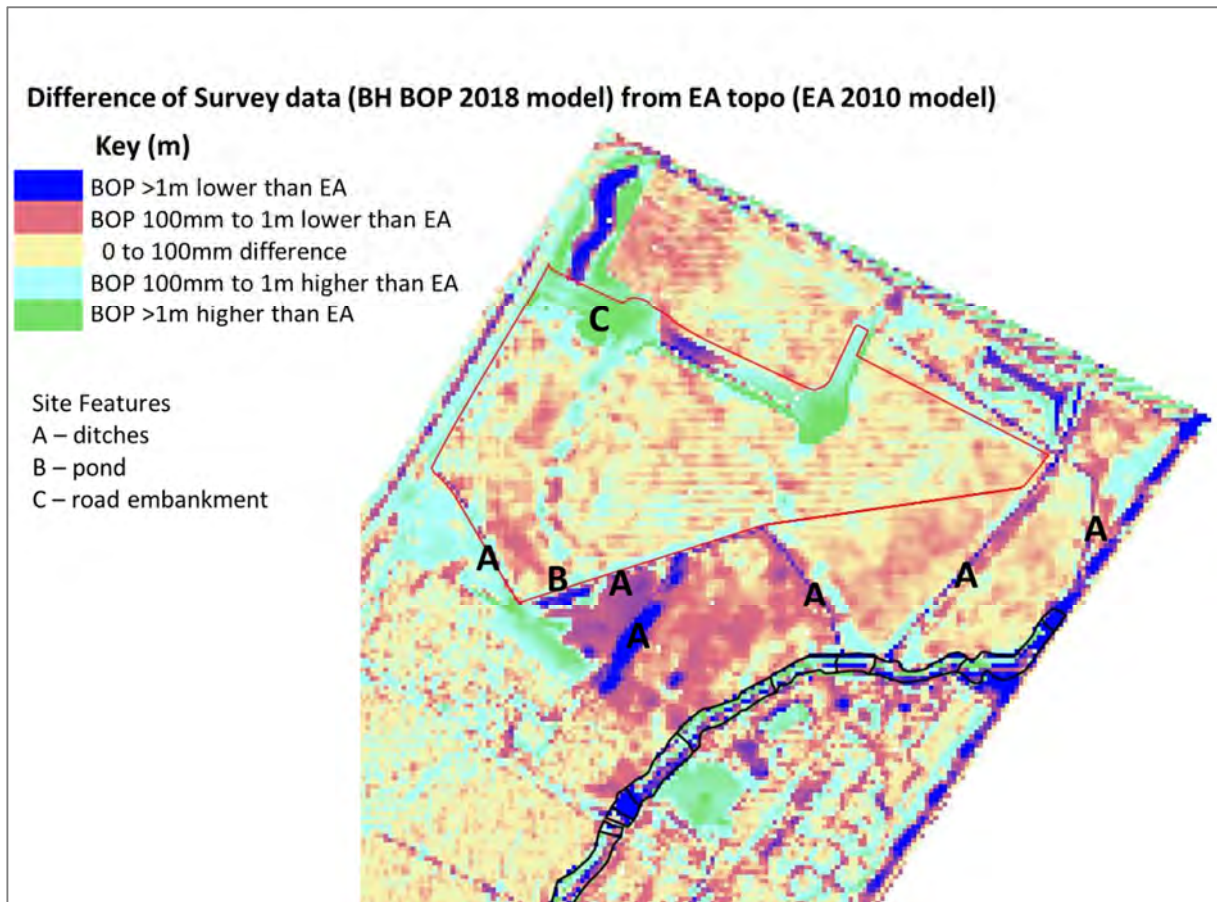


Figure 2.3 Topographic data in BuroHappold BOP 2018 model compared to topographic data used in EA 2010 model

Updated cross section survey data have also been incorporated into the BuroHappold BOP 2018 model. The distance from the upstream 1D Node LA.1350D in the EA 2010 model to the next downstream cross section was almost 400m. Additional cross sections were included to reduce the distance between sections, to approximately 100m or where changes in the channel section were observed. Where possible, updated levels were taken at the locations of the EA 2010 cross sections.

The figure below indicates the locations of the new 1D Nodes (blue) within the BuroHappold BOP 2018 model compared to the EA 2010 model (orange). Cross section survey data was recorded between the railway embankment at the upstream end (LA. 1350D) and the road crossing (LA.0726). The remaining 1D cross sections downstream of the road crossing in the BuroHappold BOP 2018 model were based on the EA 2010 model.

To ensure that the connection between 1D and 2D appropriately modelled out of bank flooding, the elevation was extracted every 2m along the bank edge either from the survey data or the LiDAR and incorporated in the 2d_bc_HX layer. This enabled the low points where the ditches outfall into the Langford Brook to be picked up more accurately within the model.

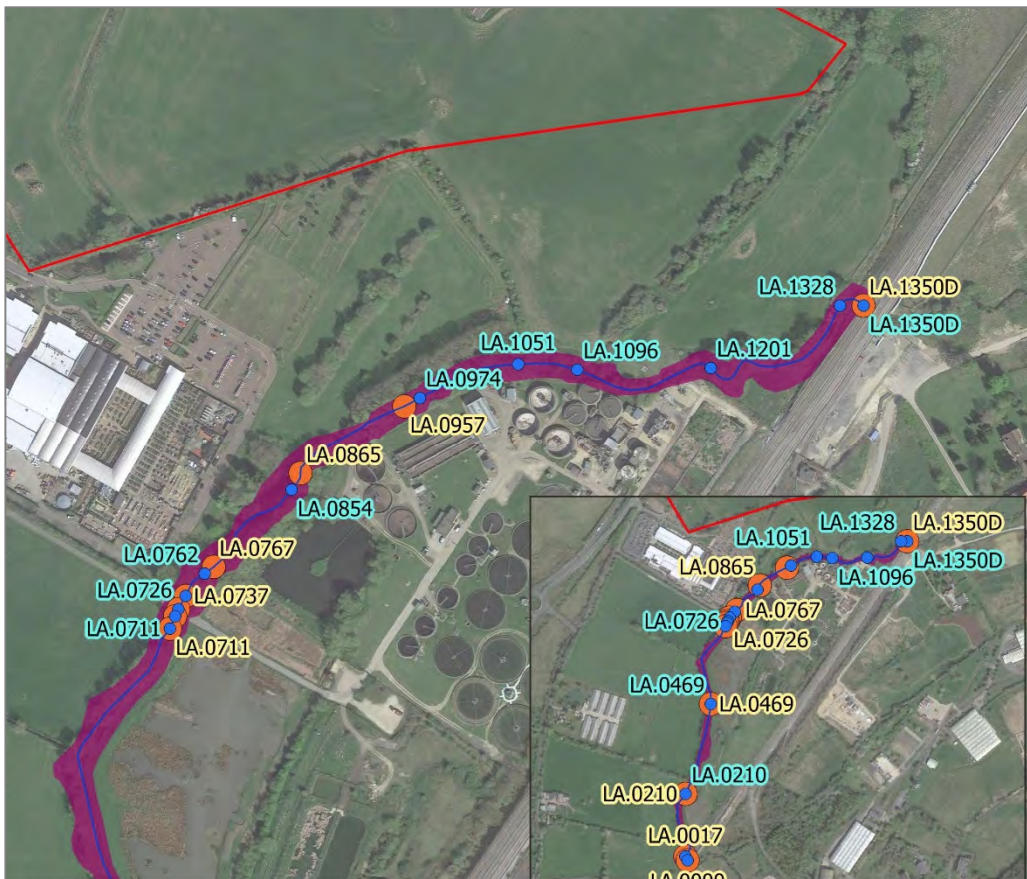


Figure 2.4 Location of 1D Nodes in the EA 2010 model (orange dot) and the BuroHappold BOP 2018 model (blue dot). Inset shows full extent of 1D model.

2.4 Ground Condition

The varying ground conditions were represented in the model using Manning’s roughness value. The EA 2010 model included different values for the channel bed and bank edge roughness to differentiate the dense vegetation along the bank edge. The same values were used in the updated 1D cross sections and in the 2D domain along the river edge. This ensured that instability issues were reduced during out-of-bank flooding. The EA 2010 model used the default value of 0.05 for the floodplain. This was amended in the BuroHappold BOP 2018 model to differentiate ground conditions between the location of the ditches and vegetation within the site. The remainder of the site observed as grass cover was represented using a lower value where the roughness is less. The table and figure below summarises the values used in the BuroHappold BOP 2018 model.

Feature	Manning’s roughness value (n)
Channel bed	0.05
Channel bank	0.06
Ditches and vegetation	0.06
Grass cover	0.035

Table 2.3 Manning’s roughness values used in the 1D and 2D component of the BuroHappold BOP 2018 model.



Figure 2.5 Illustration of the extent that different Manning’s roughness values were applied in the 2D component of the BuroHappold BOP 2018 model.

2.5 Hydraulic Structures

Within the ditch network surrounding the site, circular culverts connect flow through the ditches where paths and roads cross the ditch. This was represented in the 2D domain using a 1d_nwk layer. Figure 2.6 indicates the location of the pipes included in the model. Invert levels and pipe dimensions were incorporated based on those surveyed in the 2018 topographic survey. Ground elevations were locally modified where required to incorporate the pipe details.



Figure 2.6 Location of pipes within ditch network shown by red squares

2.6 Model Boundaries

Hydrological analysis was not undertaken for this study. The EA 2010 model underwent a detailed study of estimating rainfall runoff in the wider catchment area and used five storm events to calibrate the hydraulic model. In 2017 BuroHappold undertook an additional analysis to estimate flood flows during different climate change events based on

EA guidance that was published subsequent to the construction of the EA 2010 model. Details of the BuroHappold 2017 study are provided in Appendix A.

The estimated flow from both the EA 2010 model and the BuroHappold 2017 model was extracted and included as hydrographs in a QT (flow-time) unit boundary in the 1D ISIS model at Node LA.1350D. Table 2.4 and Figure 2.7 illustrate the hydrographs used in the assessment and the hydraulic models from which the data was extracted.

Storm Event (Return Period)	Hydraulic Model Hydrograph Extracted
1 in 20 year	EA 2010 model
1 in 100 year	EA 2010 model
1 in 100 year + 25% Climate Change	BH 2017 model
1 in 100 year + 35% Climate Change	BH 2017 model
1 in 1000 year	EA 2010 model

Table 2.4 Return periods assessed in study and the hydraulic model from which the data was extracted

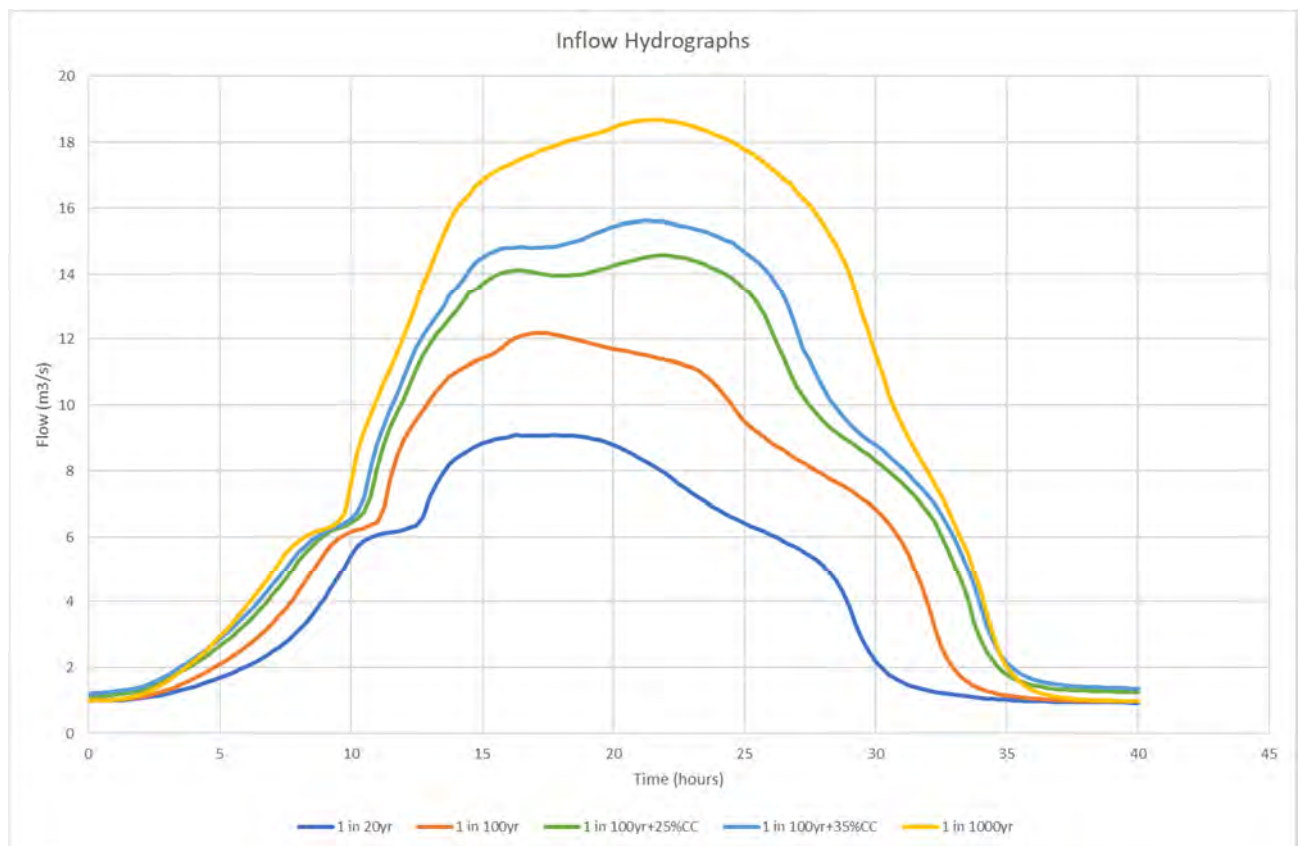


Figure 2.7 Inflow hydrographs used in the BuroHappold BOP 2018 model

The downstream boundary remained as that in the EA 2010 model; an HQ (stage-discharge) boundary in the 2D domain. This is located on the downstream side (east side) of the railway embankment to capture flow discharging through the four circular culverts in the railway embankment.

Two lateral inflows were linked to the final two cross sections in the 1D model (Nodes LA.0210 and LA.0017) in the EA 2010 model. These inflows were also included in the BuroHappold BOP 2018 model as lateral inflow units.

Surface water discharging from the ditches surrounding the site into the Langford Brook are not considered to be significant and therefore were not included in the model. However, standing water is understood to be present year-round within the pond (labelled B in Figure 2.3). This was included in the BuroHappold BOP 2018 model using an initial water level set at 64.26mAOD. A site visit confirmed that the upstream section of the pond has an informal weir and would therefore restrict the flow of water further downstream along the ditch and reduce the amount of flood storage available. The weir was represented in the model at the pond inlet location by locally modifying the bed elevation to match the pond water level.

3 Design Storm Event

Consultation with the EA undertaken for the Flood Risk Assessment identified the storm events presented in Table 3.1 as being required for assessment when setting design levels at the proposed Bicester Office Park site.

Storm Event (Return Period)	Design Implication
1 in 20 year	Classified as Functional Floodplain (Flood Zone 3b). Car parking considered acceptable provided no ground raising.
1 in 100 year + 25% Climate Change	Design Flood Event for proposed development. Any ground raising within the flood extent (outside 1 in 20 year extent) would require flood compensation.
1 in 100 year + 35% Climate Change	Buildings to be located outside of flood extent.
1 in 100 year + 35% Climate Change + 300mm freeboard	Minimum elevation finished floor levels should be set at.
1 in 1000 year	Area outside extent considered at a low risk from fluvial flooding

Table 3.1 Storm events and their implications on design at the proposed site.

The BuroHappold BOP 2018 model provides a refinement to the flood extents observed at the proposed site based on the latest and most up-to-date topographic data for the site and surrounding area. The next section indicates the estimated flood levels at the site at key locations within the site boundary.

4 Modelling Results

The flood mechanism observed from the model outputs indicate that the ditch network is being depicted more accurately in the BuroHappold BOP 2018 model than in the EA 2010 model, which has a lower resolution. Out-of-bank flooding along the ditch network occurs earlier in the storm event and extends north before flowing westwards towards the site. The image below illustrates the movement of water in the vicinity of the site between the EA 2010 and BuroHappold BOP 2018 models. The flow mechanism in both models is similar, with main differences likely being attributed to the differences in ground topography and model grid resolution. Flood flows that pass the site, re-enter the Langford Brook upstream of the road crossing, or overtop the road in both models.

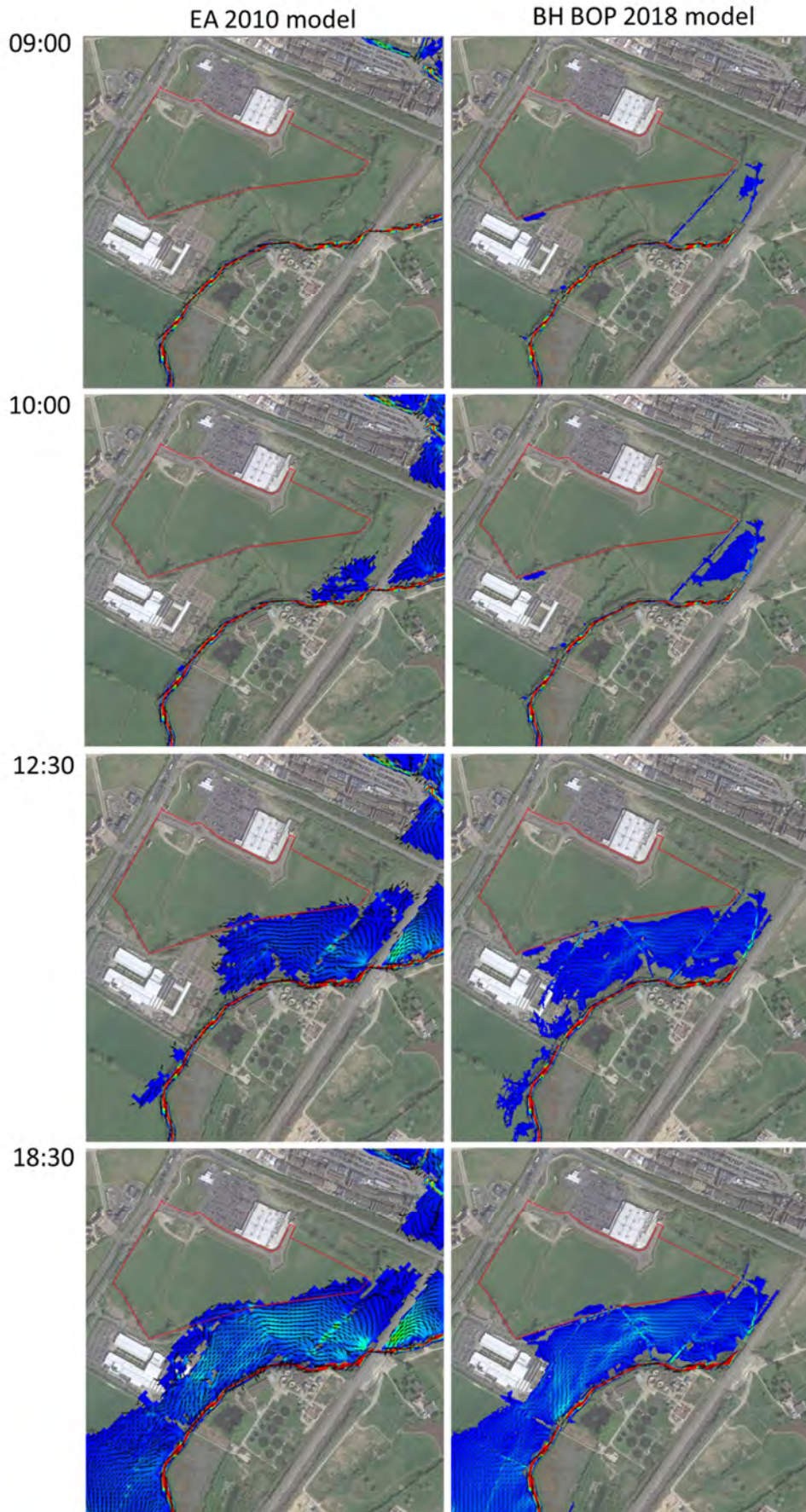


Figure 4.1 Images illustrating the flood mechanism in the EA 2010 model compared with the BuroHappold BOP 2018 model at different time outputs (shown on the left) within the 1 in 100year + 35% allowance for climate change model run. Images show flow output with arrows indicating direction of flow. The last image (time 18.30) shows the maximum extent observed at the site.

A sensitivity test was undertaken for the 1 in 100 year plus 35% allowance for climate change event varying the Manning’s ‘n’ value in both the 1D and 2D components by +/- 20%. The results show the following:

- 20% increase in Manning’s value = ~40mm increase in flood level
- 20% decrease in Manning’s value = ~20mm decrease in flood level

ID Ref	BH BOP 2018 model (mAOD)	BH BOP 2018 model +20% Manning's (mAOD)	Diff of BH +20% Manning's from BH BOP 2018 (mm)	BH BOP 2018 model -20% Manning's (mAOD)	Diff of BH -20% Manning's from BH BOP 2018 (mm)
1D Node					
LA.1350D	65.260	65.326	66	65.240	-20
LA.1328	65.291	65.333	42	65.275	-15
LA.1201	64.982	65.016	34	64.965	-17
LA.1096	64.844	64.866	23	64.825	-19
LA.1051	64.759	64.793	34	64.739	-20
LA.0974	64.662	64.702	40	64.639	-23
LA.0854	64.574	64.613	39	64.557	-17
LA.0762	64.527	64.562	35	64.512	-14
LA.0743	64.488	64.528	40	64.469	-19
LA.0726	64.444	64.484	40	64.425	-20
2D Node					
1	64.810	64.857	47	64.788	-22
2	64.651	64.695	44	64.625	-25
3	64.968	64.996	28	64.960	-8
4	64.775	64.820	45	64.753	-23
5	64.540	64.575	36	64.525	-15
6	64.636	64.683	47	64.611	-25

Table 4.1 Comparison of flood levels derived for the sensitivity tests +/- 20% Manning’s value

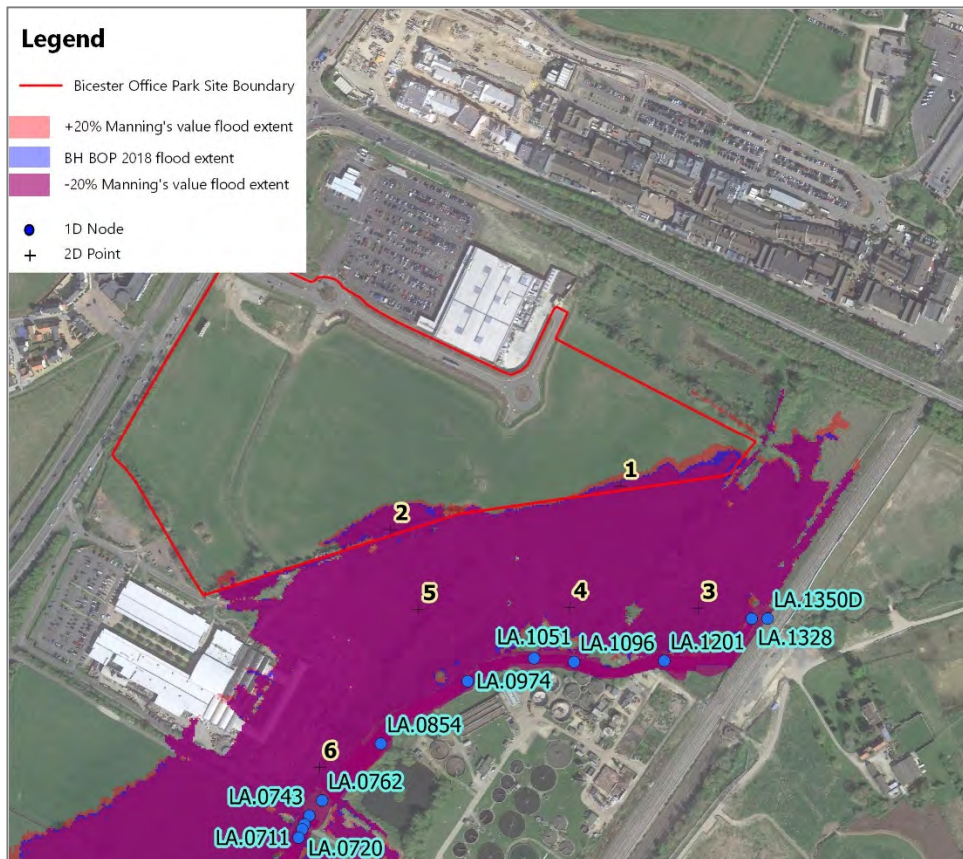


Figure 4.2 Change in flood extent from BuroHappold BOP 2018 model extent for the 1 in 100 year plus 35% allowance for climate change due to +/- 20% in Manning’s value.

Table 4.1 and Figure 4.2 indicate that by adjusting the Manning’s value slight changes are observed in flood level and extent. These changes are generally consistent across the area producing very similar flood extents within the site boundary. The results indicate that the model sensitivity in the BuroHappold BOP 2018 model is relatively consistent and therefore, appropriate for its use in this study.

The flood extents observed from the site specific BuroHappold BOP 2018 model are presented in Figure 4.3 and Table 4.2 below. The results indicate that parts of the south eastern boundary of the site is within the functional floodplain (1 in 20 year event) and at risk from events including the 1 in 1000 year event. Further flood level data is provided in Appendix C.

It is recommended that the flood levels in Table 4.2 are used for future development proposals within the site boundary, in line with the recommendations in Table 3.1.

An assessment of the flood hazard, based on EA Defra guidance¹, indicates that the hazard is considered very low within the site boundary during a 1 in 100 year event with a 35% allowance for climate change (see Figure 4.4).

Due to the smaller 2D grid size compared to the EA 2010 model, the timestep in 2D was decreased from 4s to 2s and from 2s to 1s in 1D. This improved the stability of the model run. The performance of the model is considered appropriate and a healthy model, with the maximum cumulative mass error being less than 0.12% for all design simulations.

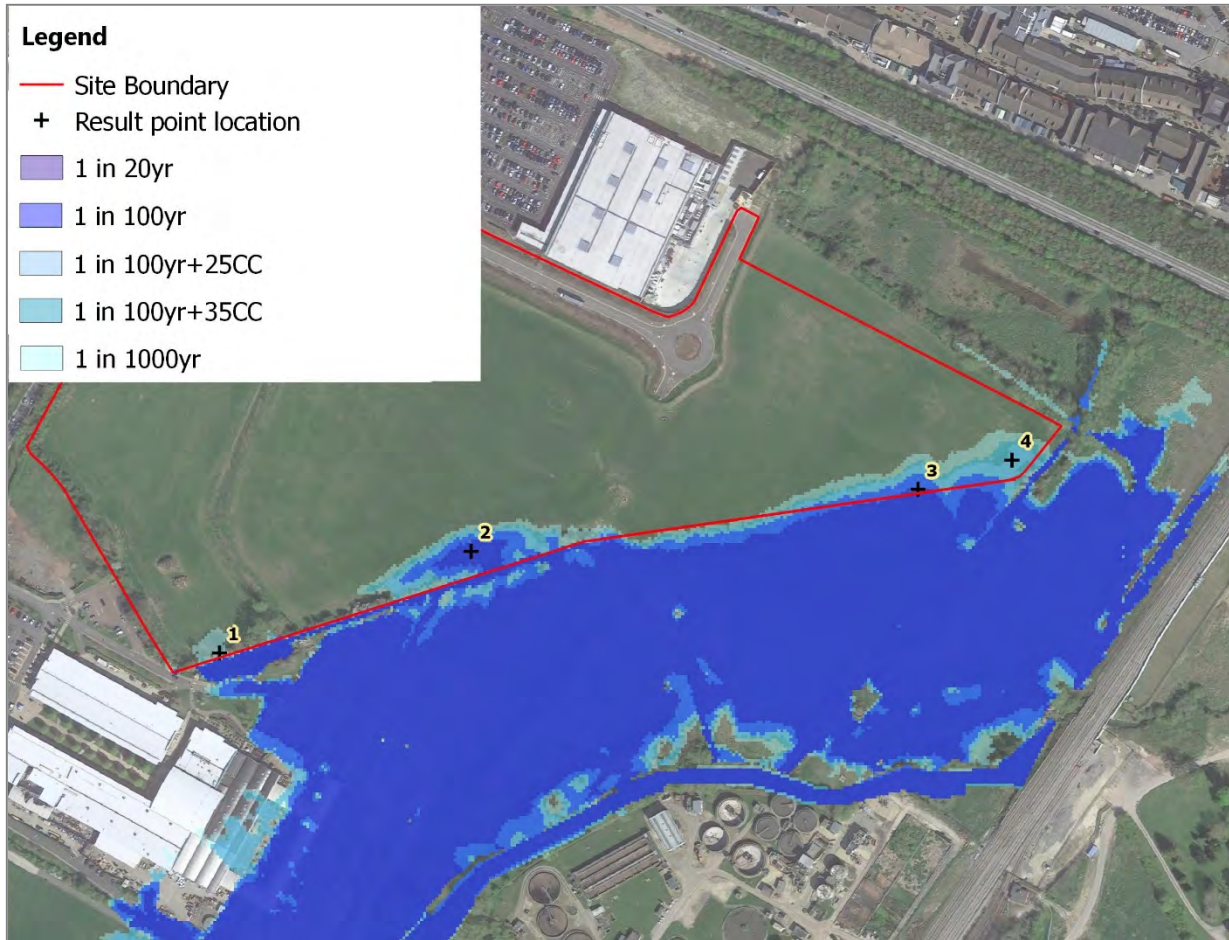


Figure 4.3 Flood map extent for different return periods as simulated in the BH Bicester Office Park 2018 model. See Table 4.2 for flood levels at the location points identified.

ID	Easting	Northing	Flood Level (mAOD)				
			1 in 20yr	1 in 100yr	1 in 100yr+25%CC	1 in 100yr+35%CC	1 in 1000yr
1	457695.8	221400.9	No flooding	No flooding	64.61	64.63	64.67
2	457863.6	221468.6	64.51	64.58	64.63	64.65	64.70
3	458159.0	221509.0	64.71	64.77	64.81	64.83	64.87
4	458224.2	221529.9	No flooding	No flooding	64.82	64.84	64.89

Table 4.2 Flood levels at four locations along the site boundary

¹ HR Wallingford and Environment Agency, May 2008, Supplementary note on flood hazard ratings and thresholds for development planning and control purpose – Clarification of the Table 113.1 of FD2320/TR2 and Figure 3.2 of FD2321/TR1



Figure 4.4 Flood hazard map for the 1 in 100 year including 35% allowance for climate change

5 Conclusion and Recommendations

Hydraulic modelling has been undertaken to assess the fluvial flood risk at the proposed Bicester Office Park site. The EA produced a hydraulic model in 2010 for the Langford Brook, Pingle Stream, Bure Brook and Back Brook watercourses in Bicester to inform strategic flood management in the area by the EA and Local Authority.

The EA hydraulic model has been modified and updated to include topographic survey information in and around the vicinity of the site, obtained in 2017 and 2018. The updated hydraulic model is referred to as the BuroHappold Bicester Office Park 2018 model (BuroHappold BOP 2018 model). The main changes made in the BuroHappold BOP 2018 model compared to the EA 2010 model include the following:

- Topographic Survey undertaken by GreenHatch Group issued on 28/09/17 – used to represent 2D floodplain within site boundary.
- Topographic Survey undertaken by GreenHatch Group issued on 19/02/18 – used to represent 1D in-channel cross sections and 2D floodplain between the site and Langford Brook.
- Most recent available LiDAR (combined dataset from 2003 and 2011 from data.gov.uk).
- Modified 2D boundary.
- Model simulated at a higher grid resolution using a 2m x 2m grid rather than 10m x 10m.
- Model simulated for the 1 in 20yr, 100yr, 100yr+25%CC, 100yr+35%CC and 1000yr flood events.

The Flood Risk Assessment issued for Planning in December 2017 has been updated based on the findings from this study and it is recommended that the flood levels derived from the BuroHappold BOP 2018 model are used for future development proposals, including informing flood risk design mitigations within the site boundary.

Appendix A Hydraulic Modelling Summary Technical Note (December 2017)


Design Note

Project Bicester Office Park

Subject Hydraulic Modelling Summary

Project no 0040031

Date 17 July 2017

Revision	Description	Issued by	Date	Approved
00	Summary of hydraulic modelling to define flood extents	DKR	24/07/17	CEJ
01	Appendix B added	DKR	11/08/17	CEJ
02	Appendix A updated	CEJ	17/08/17	DKR
03	Final for Planning	CEJ	11/09/17	DKR
04	For Planning (updated survey)	CEJ	14/12/17	

1 Introduction

BuroHappold Engineering has produced the following note to summarise the work carried out to define the flood extents for the 1 in 100 year event including the effects of climate change using the latest Environment Agency (EA) guidance for climate change allowances.

Through the pre-planning application enquiry process, the EA confirmed that hydraulic modelling was required to define the flood levels for the 1 in 100 year with the new climate change allowances using the existing ISIS- TUFLOW model. This note provides a summary of the hydraulic modelling undertaken, the model output results and the derived flood extents.

The note is intended to support the flood risk assessment being carried out for the Bicester Office Park development, located to the south of Bicester.

2 Modelling Methodology

2.1 Hydraulic Model

The EA provided a hydraulic model built by Peter Brett Associates in 2009 which covers the Langford Brook, Pingle Stream, Bure Brook and Back Brook watercourses in Bicester, Oxfordshire. The model has been built using detailed topographic survey and LiDAR topographic data and the model calibrated based on five recent (at time of model construction) flood events.

The model simulations were carried out using the following software versions:

- ISIS – version 3.1
- TUFLOW – version 2008-08-AH-iSP

This version of the hydraulic model will be referred to as the EA model to distinguish it from the models re-run by BuroHappold.

2.2 Re-baselining the Model 1 – Model Version

On receipt of the EA model BuroHappold re-ran the model to attempt to replicate the results from the EA model which were provided separately by the EA.

Since the model was originally run, both ISIS and TUFLOW have updated their software. Since the versions listed above have been superseded it was not possible to re-run the models using the same software versions as the original models. To determine what the effect of changing the software versions would have on the model results a number of test simulations were carried out.

Following this investigation, it was decided to run the models using the following software versions:

- ISIS – version 3.7 using the backwards compatibility options to match the version 3.1 defaults.
- TUFLOW – version 2016-03-AE-iSP-w64

The results of these investigations showed that the flood levels predicted by the re-baselined model were lower than the flood levels provided by the Environment Agency. In the vicinity of the site the reductions in the modelled flood levels were of the order of 5mm and were therefore considered to be within modelling tolerances.

2.3 Modelling the Effects of Climate Change

The results provided by the Environment Agency included one climate change scenario, the 1 in 100 year event plus an allowance for climate change through increasing the inflow hydrographs by 20%. Following completion of the modelling process in 2009 the EA has updated its recommended allowances for how climate change should be represented.

The latest guidance for the Thames catchment recommends that climate change be considered through an uplift to the inflow hydrographs of 25%, 35% or 70%. The choice of climate change allowance depends on the land uses, and for the development site the EA has confirmed that the two scenarios to be tested are the 25% and 35% climate change scenarios.

The inflow hydrographs for the 1 in 100 year +25% and 1 in 100 year + 35% scenarios were developed by increasing the 1 in 100 year flow multiplier in ISIS by 25% or 35% in a similar manner to the way that the 1 in 100 year + 20% climate change scenario has been represented.

2.4 Other changes to the Model

No other changes to the model were made apart from those described in order to carry out the simulations using the latest software versions and to increase the flows for two new climate change scenarios.

3 Model Results

3.1 Comparison against Previous Results

The model results in the vicinity of the site were evaluated for eight locations in the 2d domain, referred to as locations A-H. These locations can be seen on the drawing provided in Appendix A along with the peak flood levels observed at these locations. A summary of the results are provided in Table 3-1 below and the hydraulic model outputs.

Table 3-1 ISIS-TUFLOW Model levels at points on the site

Point ID	1 in 100 year + 20% climate change (mAOD)	1 in 100 year + 25% climate change (mAOD)	1 in 100 year + 35% climate change (mAOD)
A	64.73*	64.74*	64.75*
B	64.73	64.74	64.75
C	64.79	64.79	64.81
D	64.94	64.94	64.96
E	65.00	65.00	65.03
F	65.02	65.02	65.05

G	65.04	65.04	65.07
H	65.16	65.16	65.18

* Flood Levels based on Point B due to flood water not reaching the point within the hydraulic model.

The results showed that the peak water levels for the 1 in 100 year + 25% allowance for climate change event were overall similar to those reported for the 1 in 100 + 20% allowance for climate change event with differences of between 0-5mm.

Results from the 1 in 100 year + 35% run show increases in peak flood levels of approximately 15-35mm from the 1 in 100 year + 20% run in the floodplain to the south of the site.

3.2 Generation of Flood Extents

The peak flood levels from all of the models provided by the EA and simulated by BH were used to create 3d flood level surfaces for the section adjacent to the site.

A 3d topographic model was constructed using 12d software from the topographical survey for the site from 2017 and LiDAR information for the section of the site not covered by the topographic survey. The intersection of the flood level surface and the topographic surface was used to define the flood extent within the site for each of the flood events modelled.

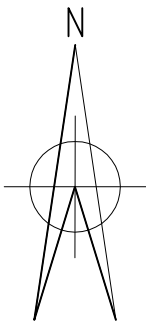
Since there are differences between the levels measured during the topographic survey and the LiDAR survey, due to the respective tolerances, there were some discontinuities between the flood extent lines at the boundary between the topographic survey and LiDAR surfaces. At these locations the flood extent line has been interpolated between the flood extents on either side of the discontinuity at the point where there is the least difference between the two surveys. A drawing showing the flood extent lines shown in Appendix A, with the locations where the flood extent line defined by the LiDAR and topographic survey clearly marked.

4 Conclusions

A hydraulic model constructed by PBA in 2009 has been rerun by BuroHappold using updated software versions to determine the peak flood levels in the 1 in 100 year event, including a 25% and 35% uplift in the hydrographs to allow for the effects of climate change based on the most recent guidance from the EA.

The peak flood levels from these simulations are shown in Appendix A and the model outputs provided in Appendix B.

Appendix A – Flood Extent Map



LANGFORD BROOK (BICESTER) & PINGLE-BACK-BURE 2010 ISIS-TUFLOW MODEL LEVELS

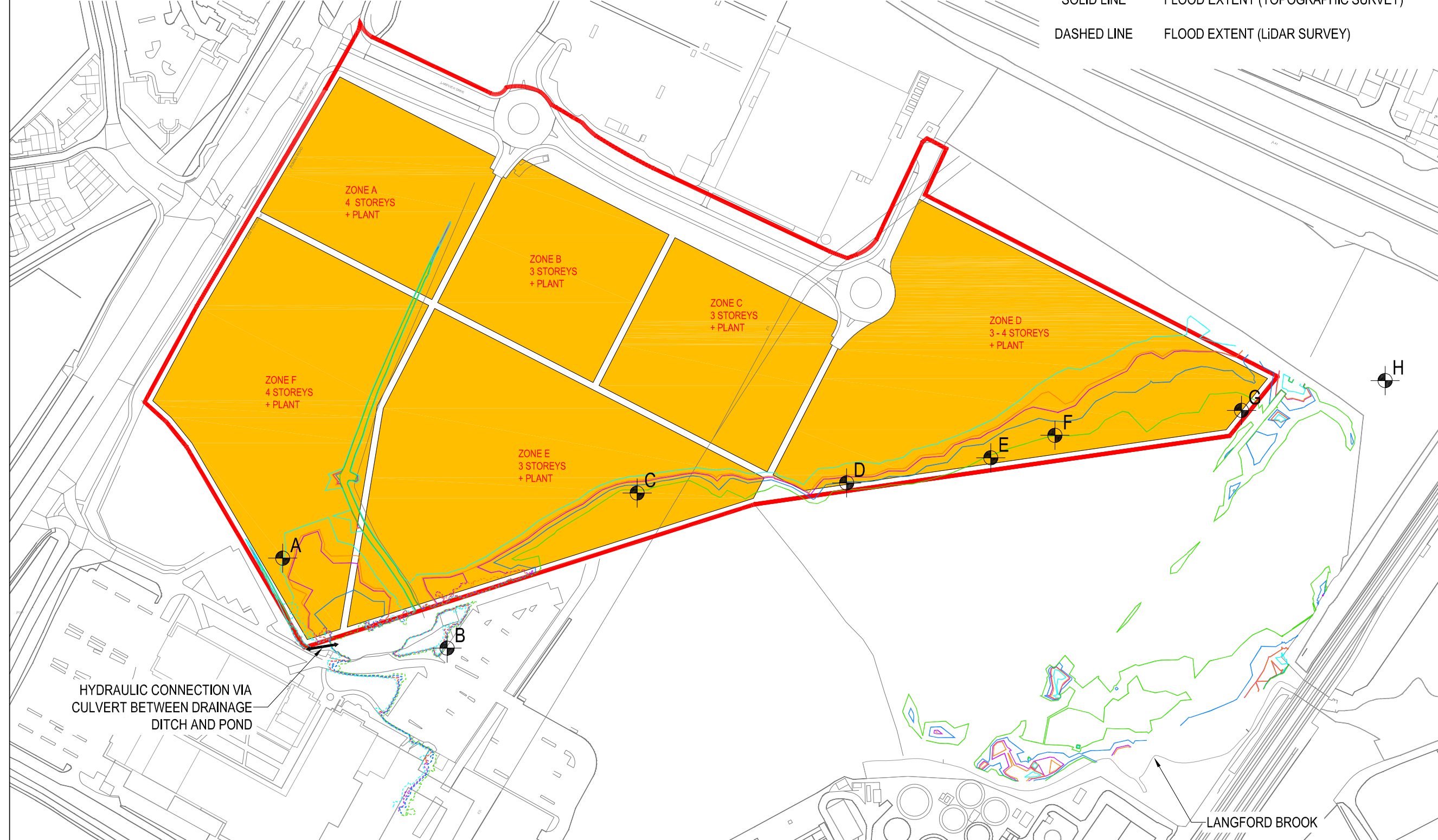
POINT ID	X	Y	1 in 20 (mAOD)	1 in 100 (mAOD)	1 in 100 + 25%CC (mAOD)	1 in 100 + 35%CC (mAOD)	1 in 1000 (mAOD)
A	457650.7	221442.2	64.63*	64.70*	64.74*	64.75*	64.81
B	457751.0	221387.3	64.63	64.70	64.74	64.75	64.81
C	457866.9	221481.9	64.67	64.74	64.79	64.81	64.88
D	457994.7	221488.1	64.81	64.89	64.94	64.96	65.04
E	458082.6	221503.4	64.83	64.93	65.00	65.03	65.13
F	458121.7	221517.1	64.84	64.95	65.02	65.05	65.16
G	458235.5	221532.3	64.84	64.96	65.04	65.07	65.19
H	458323.1	221550.5	65.02	65.11	65.16	65.18	65.27

* FLOOD LEVELS BASED ON POINT B DUE TO FLOOD WATER NOT REACHING THE POINT WITHIN THE HYDRAULIC MODEL

- SITE BOUNDARY
- FLOOD LEVEL DATA POINT
- 2017 FLOOD EXTENTS
- 1 IN 20 YEAR FLOOD EXTENT
- 1 IN 100 YEAR FLOOD EXTENT
- 1 IN 1000 YEAR FLOOD EXTENT
- 1 IN 100 YEAR +25%CC FLOOD EXTENT
- 1 IN 100 YEAR +35%CC FLOOD EXTENT
- SOLID LINE FLOOD EXTENT (TOPOGRAPHIC SURVEY)
- DASHED LINE FLOOD EXTENT (LIDAR SURVEY)

Notes
 BACKGROUND OS TILE Ref. 10019980
 2011 LIDAR (1M RESOLUTION) DIGITAL TERRAIN MODEL DOWNLOADED FROM <http://environment.data.gov.uk>. CONTAINS PUBLIC SECTOR INFORMATION LICENSED UNDER THE OPEN GOVERNMENT LICENCE v3.0.
 TOPOGRAPHIC SURVEY BY GREENHATCH (SEPTEMBER 2017)
 ARCHITECT PARAMETER PLAN Ref. 1105_P_005
 FLOOD LEVEL INFORMATION PROVIDED AS PART OF ENVIRONMENT AGENCY'S PRODUCT 6 DATA JUNE 2017. CONTAINS ENVIRONMENT AGENCY AND/OR DATABASE RIGHT.
 BUROHAPPOLD HAS RE-RUN THE ISIS-TUFLOW HYDRAULIC MODEL TO DERIVE THE 1 IN 100 YEAR +25%CC AND 1 IN 100 YEAR +35%CC EVENT CLIMATE CHANGE SCENARIOS.
 THE FLOOD EXTENTS HAVE BEEN DERIVED BASED ON THE 2017 TOPOGRAPHIC SURVEY INFORMATION WHERE THIS EXISTS, THE 2011 LIDAR DIGITAL TERRAIN MODEL WHERE THE TOPOGRAPHIC SURVEY DOES NOT EXIST AND THE 2010 ISIS-TUFLOW MODEL FLOOD LEVELS USING 3D MODELLING SOFTWARE. INTERPOLATION HAS BEEN REQUIRED BETWEEN THE LIDAR AND TOPOGRAPHIC SURVEY EXTENTS.

05 MINOR CHANGES TO FLOOD EXTENTS / 29.11.17	SM/CJ
04 UPDATED FLOOD EXTENTS FOR SM/CJ NEW TOPO SURVEY / 17.10.17	
03 AMENDED PROJECT NAME INFORMATION / 12.09.17	SM/CJ
02 UPDATED REDLINE BOUNDARY & MASTERPLAN / 16.08.17	SM/CJ
Rev Description/Date	Drn/Chk



INFORMATION
 Status of drawing

BUROHAPPOLD ENGINEERING

Camden Mill
 Lower Bristol Road
 Bath BA2 3DQ
 UK
 Tel: +44 (0)1225 320600
 Fax: +44 (0)870 787 4148
 Email: 0040031@burohappold.com
 Web: www.burohappold.com

Architect **Bennett Associates**
 Project **Bicester Office Park**
 Drg Title **FLOOD EXTENTS DERIVED FROM COMBINED 2011 LIDAR AND TOPO SURVEY**

Scales@A3 1:2500
 Drawn by SM
 Checked by CJ
 Date JULY 2017

Job No. **0040031**
 Drg No. **WSK005**
 Rev **05**

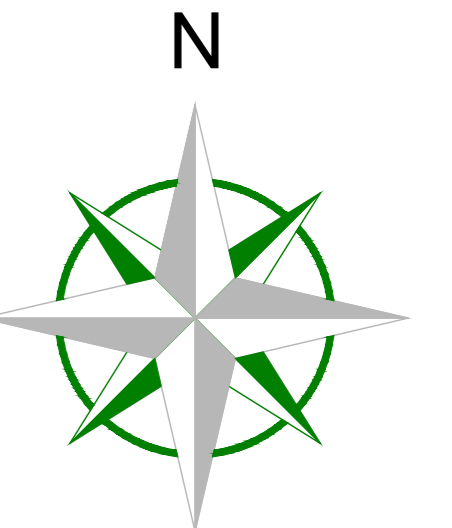
Simon Mudd \lsrv-london03\CAD\0040031 Bicester Business Park- Planning Support\F54 Water\Sheet\Sketches\WSK005.dwg

Appendix B – Flood Model Outputs

The table below provides the maximum flood levels from the 1d model at each of the nodes in the vicinity of the site. The levels are to ordnance datum.

Node	1 in 20	1 in 100	1 in 100 + 20%	1 in 100 + 25%	1 in 100 + 35%	1 in 1000
LA.1408	65.454	65.640	65.728	65.759	65.809	65.954
LA.1362	65.299	65.429	65.491	65.515	65.557	65.684
LA.1350	65.268	65.378	65.424	65.442	65.473	65.572
LA.1350BU	65.268	65.378	65.424	65.442	65.473	65.572
LA.1350BD	65.268	65.378	65.424	65.442	65.473	65.571
LA.1873CU	65.832	65.996	66.128	66.138	66.165	66.257
LA.1350D	65.268	65.378	65.424	65.442	65.473	65.571
LA.0957	64.548	64.623	64.660	64.666	64.680	64.714
LA.0865	64.438	64.525	64.571	64.580	64.599	64.640
LA.0767	64.302	64.382	64.466	64.479	64.501	64.550
LA.0737	64.221	64.266	64.374	64.387	64.409	64.461
LA.0726	64.217	64.258	64.364	64.376	64.397	64.445
LA.0726BU	64.217	64.258	64.364	64.376	64.397	64.445
LA.0720BD	64.215	64.255	64.272	64.280	64.295	64.332
LA.0720	64.215	64.255	64.272	64.280	64.295	64.332
LA.0711	64.208	64.251	64.269	64.278	64.296	64.337

Appendix B Topographic and LiDAR Information



Station Information:

Station	Easting (m)	Northing (m)	Level (m)
GH1	458049.908	221684.512	67.095
GH2	458016.141	221588.821	67.325
GH3	457918.706	221648.867	67.729
GH4	457813.990	221701.959	67.358
GH5	457769.668	221703.759	66.947
GH6	457715.541	221753.903	67.049
GH7	457723.838	221818.099	68.544
GH8	457783.068	221886.926	69.454
GH9	457618.945	221662.947	67.808
TA16	457841.170	221311.839	64.373
TA17	457751.130	221341.429	64.375
TA18	457665.165	21373.9880	65.364
TA19	457597.196	221422.121	66.110
TA20	457508.503	221468.539	67.041
TA21	457624.739	221652.797	67.471

OS Note:
Some services may have been omitted due to parked vehicles.
The Ordnance Survey file is to be used as a guide only.

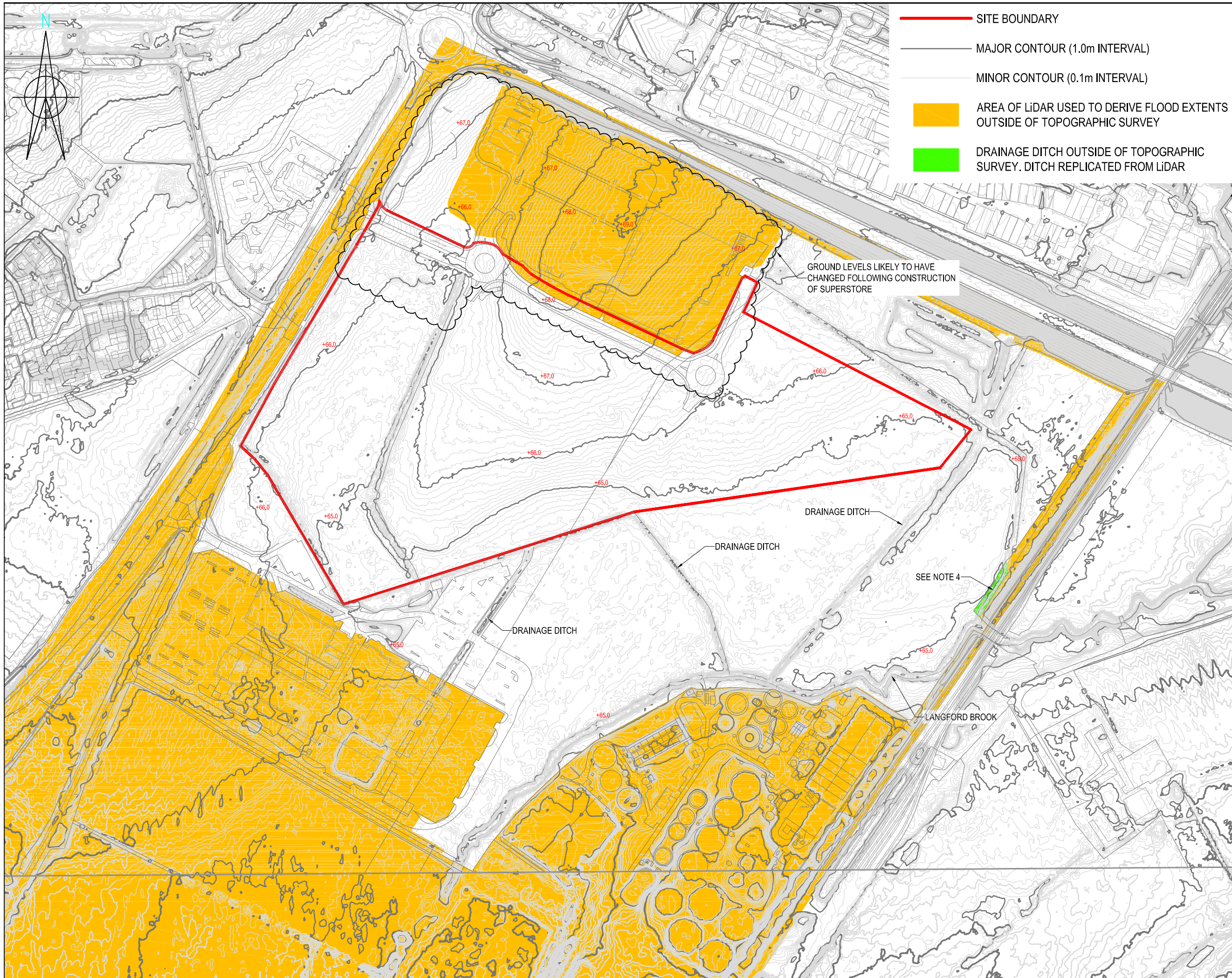
OS Buildings Surveyed Buildings

This survey has been orientated to the Ordnance Survey (O.S.) National Grid OSGB36(15) via Global Navigational Satellite Systems (GNSS) and the O.S. Active Network (OS Net).
A true OSGB36 coordinate has been established near to the site centre via a transformation using the OSTN15GB & OSGB15GB transformation models.
The survey has been correlated to this point and a further one or more OSGB36 (15) points established to create a true O.S. bearing for angle orientation.

No scale factor has been applied to the survey therefore the coordinates shown are arbitrary & not true O.S. Coordinates which have a scale factor applied.
Please refer to Survey Station Table to enable establishment of the on-site grid and datum.

Legend:

	Buildings		Overhead Cables		IC	Intersecting		IS	Islet
	Surveyed Buildings		Concrete pipe		OP	Open		IS	Isolated Islet
	Boundary		Cast iron pipe		OP	Open		IS	Isolated Islet
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	Boundary								



- SITE BOUNDARY
- MAJOR CONTOUR (1.0m INTERVAL)
- MINOR CONTOUR (0.1m INTERVAL)
- AREA OF LIDAR USED TO DERIVE FLOOD EXTENTS OUTSIDE OF TOPOGRAPHIC SURVEY
- DRAINAGE DITCH OUTSIDE OF TOPOGRAPHIC SURVEY. DITCH REPLICATED FROM LIDAR

- Notes
1. BACKGROUND OS TILE Ref. 100019980
 2. COMBINED LIDAR (2M RESOLUTION) DIGITAL TERRAIN MODEL DOWNLOADED FROM <http://environment.data.gov.uk>. CONTAINS PUBLIC SECTOR INFORMATION LICENSED UNDER THE OPEN GOVERNMENT LICENCE v3.0. LIDAR COMBINES DATA FROM 2003 AND 2011
 3. CONTOURS DERIVED FROM LIDAR SURVEY POINT LEVEL DATA
 4. SECTION OF DRAINAGE DITCH UNABLE TO BE SURVEYED DURING THE 2018 TOPOGRAPHIC SURVEY DUE TO BEING INACCESSIBLE. PLAN LOCATION BETWEEN LANGFORD BROOK AND DRAINAGE DITCH TAKEN FROM LIDAR WITH BANK LEVELS BEING INTERPOLATED USING LEVELS RECORDED IN THE 2018 TOPOGRAPHIC SURVEY.

03 NEW TOPO SURVEY EXTENT INFORMATION / 29.03.18	SM/CJ
02 NEW TOPO SURVEY EXTENT INFORMATION / 17.10.17	SM/CJ
01 AMENDED PROJECT NAME INFORMATION / 12.09.17	SM/CJ
00 INFORMATION / 16.08.17	SM/CJ
Rev Description/Date	Drm/Chk

INFORMATION

Status of drawing

BUROHAPPOLD
ENGINEERING

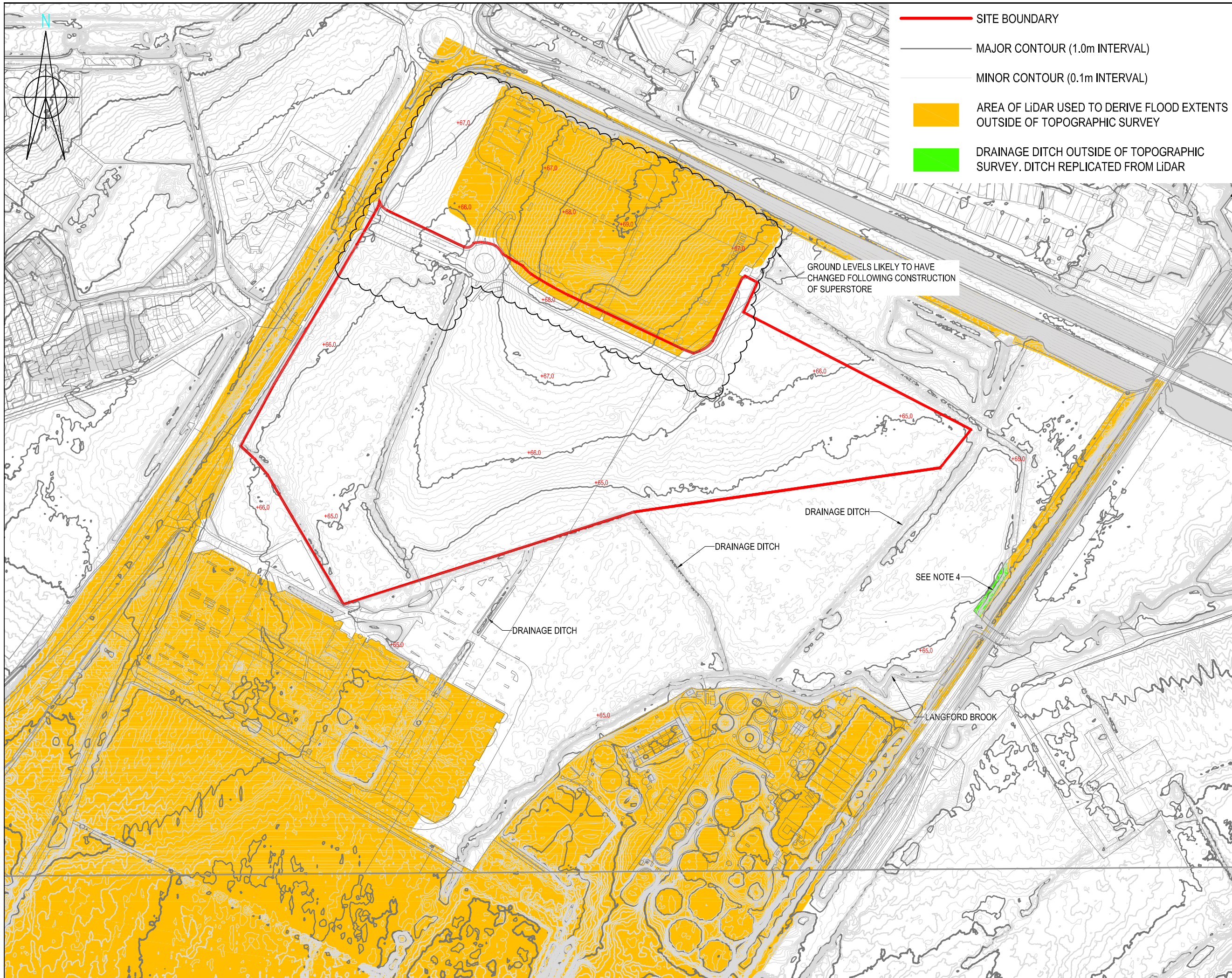
Camden Mill Lower Bristol Road Bath BA2 3DQ UK
Tel: +44 (0)1225 320600 Fax: +44 (0)870 787 4148 Email: 0040031@burohappold.com Web: www.burohappold.com

Architect *Bennett Associates*
Project **Bicester Office Park**
Drg Title **LiDAR SURVEY WITH 100mm INTERVAL CONTOURS**

Scales@A3 NTS
Drawn by SM
Checked by CJ
Date AUGUST 2017

Job No. **0040031**
Drg No. **WSK007**
Rev **03**

Simon Mudd \srv-jondon05\CAD\0040031 Bicester Business Park- Planning Support\F34 Water\Sheet\Sketches\WSK007.dwg



- SITE BOUNDARY
- MAJOR CONTOUR (1.0m INTERVAL)
- MINOR CONTOUR (0.1m INTERVAL)
- AREA OF LIDAR USED TO DERIVE FLOOD EXTENTS OUTSIDE OF TOPOGRAPHIC SURVEY
- DRAINAGE DITCH OUTSIDE OF TOPOGRAPHIC SURVEY. DITCH REPLICATED FROM LIDAR

- Notes
1. BACKGROUND OS TILE Ref. 100019980
 2. COMBINED LIDAR (2M RESOLUTION) DIGITAL TERRAIN MODEL DOWNLOADED FROM <http://environment.data.gov.uk>. CONTAINS PUBLIC SECTOR INFORMATION LICENSED UNDER THE OPEN GOVERNMENT LICENCE v3.0. LIDAR COMBINES DATA FROM 2003 AND 2011
 3. CONTOURS DERIVED FROM LIDAR SURVEY POINT LEVEL DATA
 4. SECTION OF DRAINAGE DITCH UNABLE TO BE SURVEYED DURING THE 2018 TOPOGRAPHIC SURVEY DUE TO BEING INACCESSIBLE. PLAN LOCATION BETWEEN LANGFORD BROOK AND DRAINAGE DITCH TAKEN FROM LIDAR WITH BANK LEVELS BEING INTERPOLATED USING LEVELS RECORDED IN THE 2018 TOPOGRAPHIC SURVEY.

03 NEW TOPO SURVEY EXTENT INFORMATION / 29.03.18	SM/CJ
02 NEW TOPO SURVEY EXTENT INFORMATION / 17.10.17	SM/CJ
01 AMENDED PROJECT NAME INFORMATION / 12.09.17	SM/CJ
00 INFORMATION / 16.08.17	SM/CJ
Rev Description/Date	Drn/Chk

INFORMATION

Status of drawing

**BUROHAPPOLD
ENGINEERING**

Camden Mill
Lower Bristol Road
Bath BA2 3DQ
UK

Tel: +44 (0)1225 320600
Fax: +44 (0)870 787 4148
Email: 0040031@burohappold.com
Web: www.burohappold.com

Architect *Bennett Associates*

Project **Bicester Office Park**

Drg Title **LiDAR SURVEY
WITH 100mm INTERVAL
CONTOURS**

Scales@A3 NTS

Drawn by SM

Checked by CJ

Date AUGUST 2017

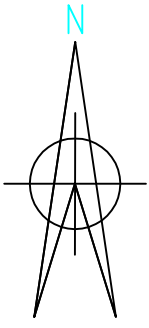
Job No. **0040031**

Drg No. **WSK007**


Rev **03**

Simon Mudd \lsrv-london05\CAD\0040031 Bicester Business Park- Planning Support\F34 Water\Sheet\Sketches\WSK007.dwg

Appendix C Flood Extents and Flood Levels Plan



POINT ID	X	Y	1 in 20 (mAOD)	1 in 100 (mAOD)	1 in 100 + 25%CC (mAOD)	1 in 100 + 35%CC (mAOD)	1 in 1000 (mAOD)
A	458310.6	221537.4	64.91	64.94	64.95	64.96	64.99
B	458224.2	221529.9	N/A	N/A	64.82	64.84	64.89
C	458161.6	221516.1	N/A	64.77	64.81	64.83	64.87
D	458087.7	221502.4	N/A	N/A	N/A	64.80	64.85
E	457863.6	221468.6	64.51	64.58	64.63	64.65	64.70
F	457695.8	221400.9	N/A	N/A	64.61	64.63	64.67
G	458256.2	221457.9	64.90	64.93	64.95	64.96	64.99
H	458057.7	221375.2	64.64	64.72	64.76	64.78	64.82
I	457895.8	221372.8	64.46	64.56	64.62	64.64	64.69

- SITE BOUNDARY
-  FLOOD LEVEL DATA POINT
- 2018 FLOOD EXTENTS**
- 1 IN 20 YEAR FLOOD EXTENT
- 1 IN 100 YEAR FLOOD EXTENT
- 1 IN 1000 YEAR FLOOD EXTENT
- 1 IN 100 YEAR +25%CC FLOOD EXTENT
- 1 IN 100 YEAR +35%CC FLOOD EXTENT

Notes
 BACKGROUND OS TILE Ref. 10019980
 COMBINED LIDAR (2M RESOLUTION) DIGITAL TERRAIN MODEL DOWNLOADED FROM <http://environment.data.gov.uk>. CONTAINS PUBLIC SECTOR INFORMATION LICENSED UNDER THE OPEN GOVERNMENT LICENCE v3.0.
 TOPOGRAPHIC SURVEY BY GREENHATCH (FEBRUARY 2018)
 ARCHITECT PARAMETER PLAN Ref. 1105_P_005
 FLOOD LEVEL INFORMATION DERIVED FROM THE BICESTER OFFICE PARK 2018 HYDRAULIC MODEL CREATED BY BUROHAPPOLD BASED ON THE ENVIRONMENT AGENCY'S PRODUCT 6 DATA JUNE 2017 (LANGFORD_BROOK_BICESTER_ & PRINGLE BACK BURE_2010 MODEL). CONTAINS ENVIRONMENT AGENCY INFORMATION © ENVIRONMENT AGENCY AND/OR DATABASE RIGHT.

THE FLOOD EXTENTS HAVE BEEN DERIVED BASED ON THE 2018 TOPOGRAPHIC SURVEY INFORMATION WHERE THIS EXISTS AND THE LIDAR DIGITAL TERRAIN MODEL WHERE THE TOPOGRAPHIC SURVEY DOES NOT EXIST.

06 UPDATED FLOOD EXTENTS FROM MODEL / 29.03.18	SM/CJ
05 MINOR CHANGES TO FLOOD EXTENTS / 29.11.17	SM/CJ
04 UPDATED FLOOD EXTENTS FOR SM/CJ NEW TOPO SURVEY / 17.10.17	
03 AMENDED PROJECT NAME INFORMATION / 12.09.17	SM/CJ
Rev Description/Date	Drn/Chk

INFORMATION

Status of drawing

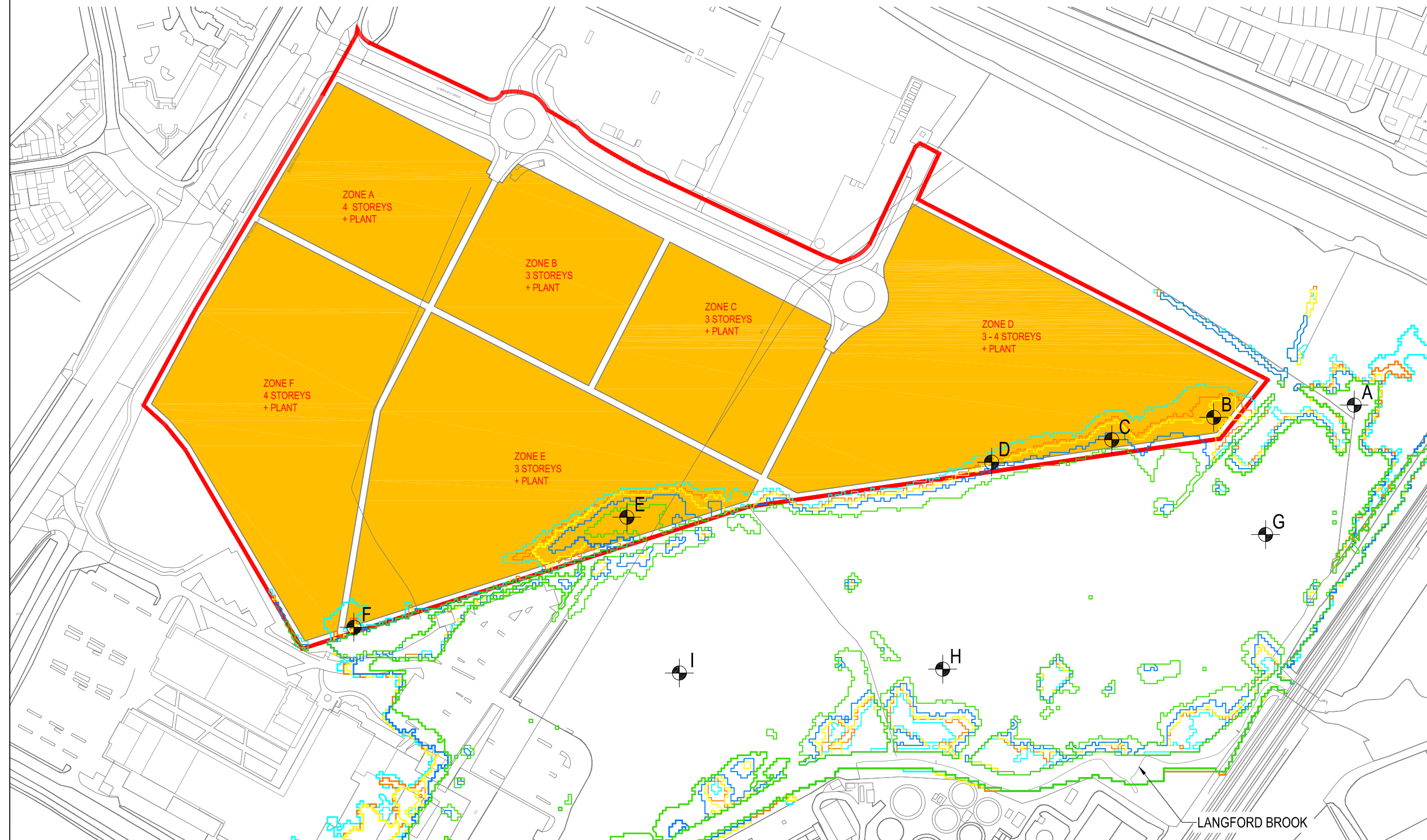
**BUROHAPPOLD
ENGINEERING**

Camden Mill Lower Bristol Road Bath BA2 3DQ UK
 Tel: +44 (0)1225 320600 Fax: +44 (0)870 787 4148 Email: 0040031@burohappold.com Web: www.burohappold.com

Architect *Bennett Associates*
 Project **Bicester Office Park**
 Drg Title **FLOOD EXTENTS DERIVED FROM COMBINED LIDAR AND 2018 TOPO SURVEY**

Scales@A3 1:2500
 Drawn by SM
 Checked by CJ
 Date JULY 2017

Job No. **0040031**
 Drg No. **WSK005**
 Rev **06**



Simon Mudd \lsrv-london05\CAD\0040031 Bicester Business Park- Planning Support\F34 WaterSheet\Sketches\WSK005.dwg

Appendix F Drainage Strategy

Bicester Office Park

Drainage Strategy

040031

3 July 2017

Revision 04

Revision	Description	Issued by	Date	Checked
00	Initial Issue	JW	9/8/17	LJ
01	Client name amended	JW	10/8/17	LJ
02	Red line plan amended	JW	16/8/17	LJ
03	Minor amendments	JW	08/9/17	LJ
04	Red line plan amended	JW	26/09/17	LJ

C:\Users\jwaiting\Documents\Bicester\170911 JW 040031 Drainage Strategy 04.docx

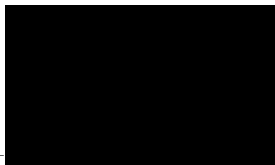
This report has been prepared for the sole benefit, use and information of Scenic Land Developments Ltd for the purposes set out in the report or instructions commissioning it. The liability of Buro Happold Limited in respect of the information contained in the report will not extend to any third party.

author **John Waiting**

date **8/8/17**

approved **Les Johnson**

signature



Date **9/8/17**

..

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1	Executive summary	8
2	Introduction	9
3	Planning history	10
4	Existing drainage networks	11
5	Drainage strategy	15
6	Sustainable Drainage Systems (SuDS)	17
7	Conclusions.	20
	Appendix A As built drainage network drawings	

1 Executive summary

- 1.1 This report has been prepared to set out the drainage strategy in support of an outline planning application for 60,000 m² B1 development at Bicester Office Park.
- 1.2 The majority of the site area covered by the current planning application was subject to an outline planning application submitted in 2007. It subsequently received approval.
- 1.3 In 2011 a detailed application was submitted for a the primary infrastructure and a retail development. To accompany this a revised drainage strategy document was prepared to show how the relevant planning conditions that were attached to the outline permission were to be discharged.
- 1.4 As part of the primary infrastructure contract both foul and surface water sewers have been constructed to serve the proposed development. These have capacity for the foul and surface water flow rates from the proposed 60,000 m² B1 development.
- 1.5 The surface water runoff from the development will be limited to greenfield flow rates and the primary infrastructure has been design to reflect this. The on-site surface water drainage network will incorporate the recommendations of Sustainable Drainage Systems (SuDS) good practice.
- 1.6 The proposed development density of the masterplan will allow the incorporation of a significant area of green infrastructure. This will facilitate the provision of a number of different SuDS components within the detailed design of the surface water network

2 Introduction

- 2.1 This drainage strategy has been produced in support of an outline planning application for a 13Ha site known as the Bicester Office Park. BuroHappold has been involved with the development since 2007 when the first outline planning application was submitted, and have produced both Flood Risk Assessments and Drainage Strategies in support of the initial phases of development.
- 2.2 The site is located on the western side of Bicester, adjacent to the Bicester Outlet Shopping Village. An aerial view of the site is shown in figure 1 below.



Figure 1

- 2.3 It can be seen in figure 1 that a spine road (Lakeview Drive) to serve the development has been constructed in addition to a Tesco store. As part of the primary infrastructure contract both surface water and foul water drainage networks were constructed. These were designed to provide capacity to serve the development proposals covered by the outline planning application.

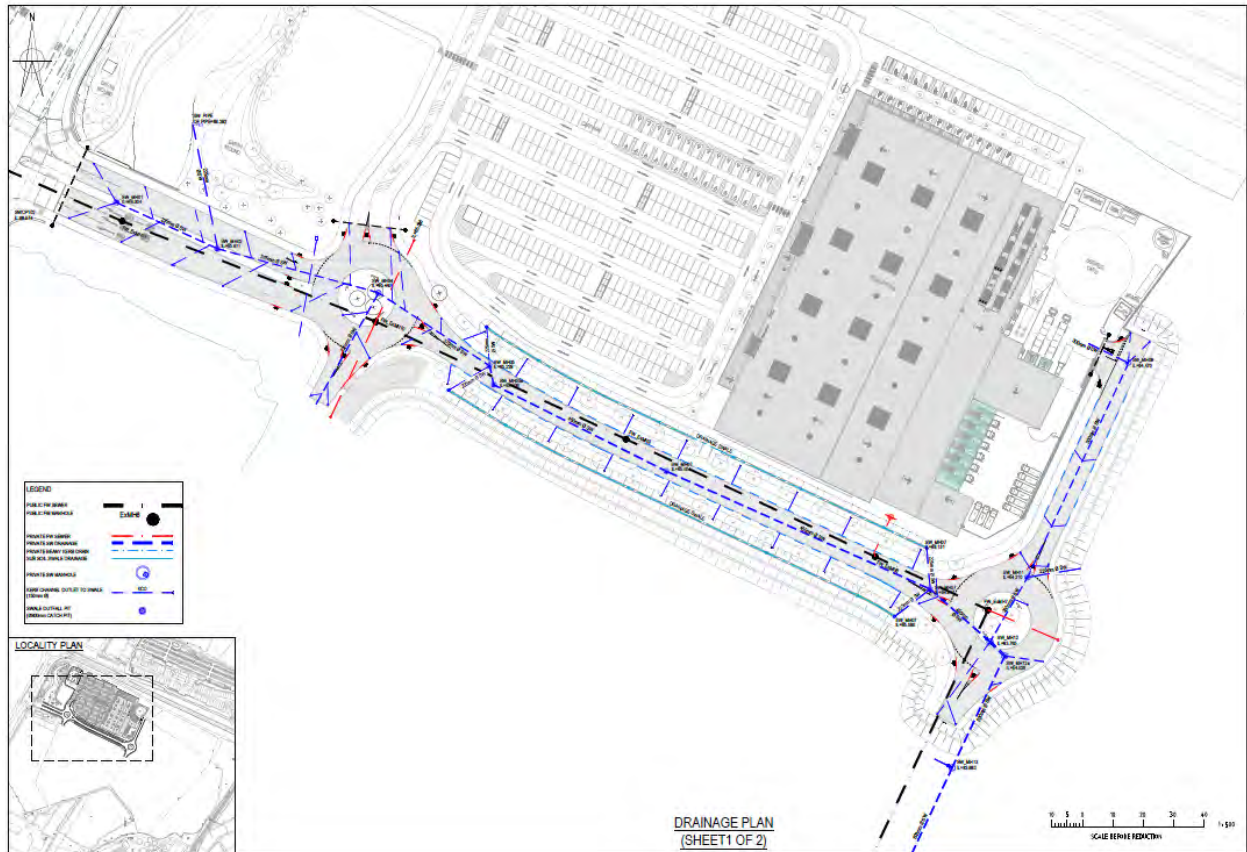
3 Planning history

- 3.1 An outline planning application which covered most of the area of the current application was submitted in 2007. It subsequently received approval.
- 3.2 In 2011 a detailed application was submitted for a the primary infrastructure and a retail development (Tesco Store). To accompany this, a revised drainage strategy document was prepared to show how the relevant planning conditions that were attached to the outline permission were to be discharged.
- 3.3 As part of the current application a new Flood Risk Assessment has been prepared and this assumes that the surface water runoff from the undeveloped part of the site will be limited to 'Greenfield' run of rates i.e. the runoff will not exceed the flow rates that occur at present. In addition the associated attenuation measures for the developed site will be designed to accommodate the increased rainfall intensities in accordance with the climate change recommendations issued by the Environment Agency in February 2016.

4 Existing drainage networks

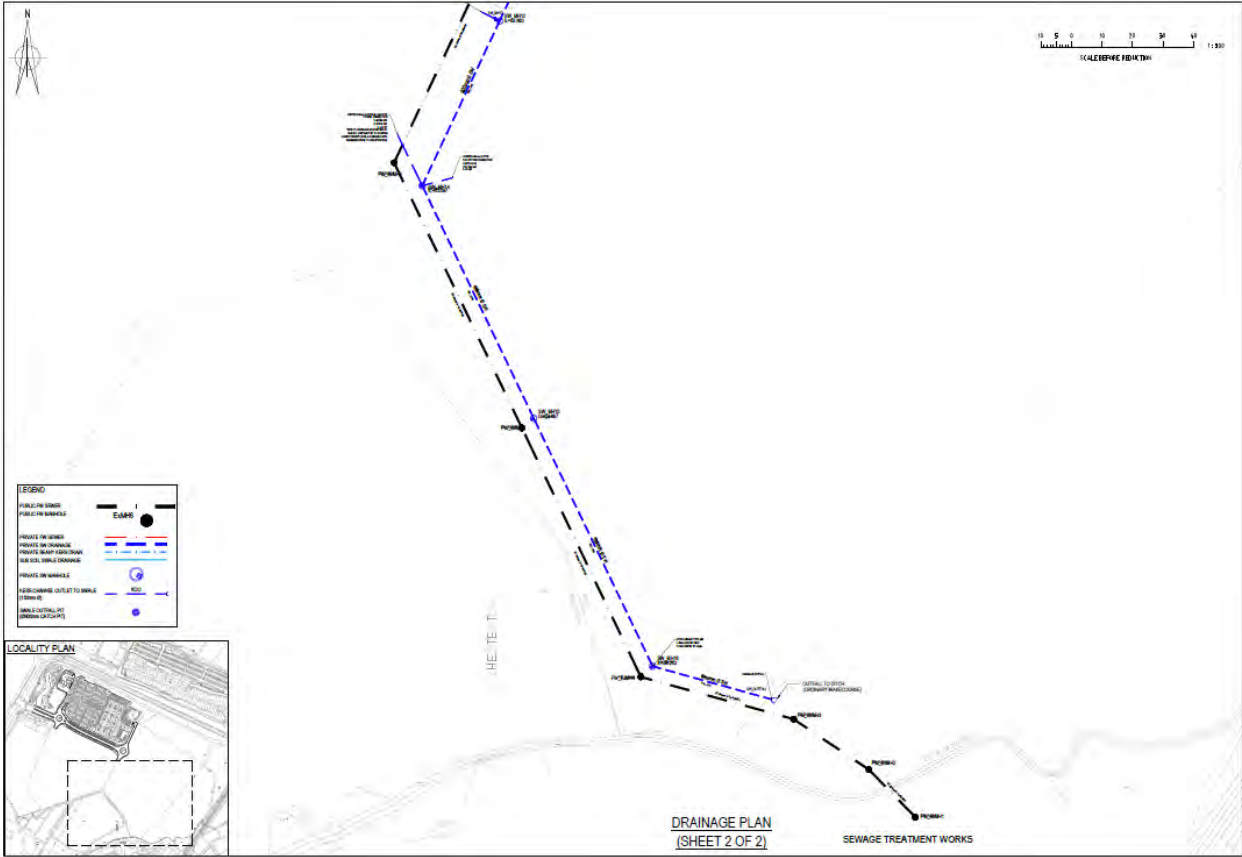
4.1 Surface Water

4.1.1 A surface water sewer network was constructed as part of the primary infrastructure works. The network associated with the access road is shown on the plan below.



The surface water infrastructure is shown by the blue dotted lines. Spurs have been left to facilitate drainage connections from the masterplan proposals.

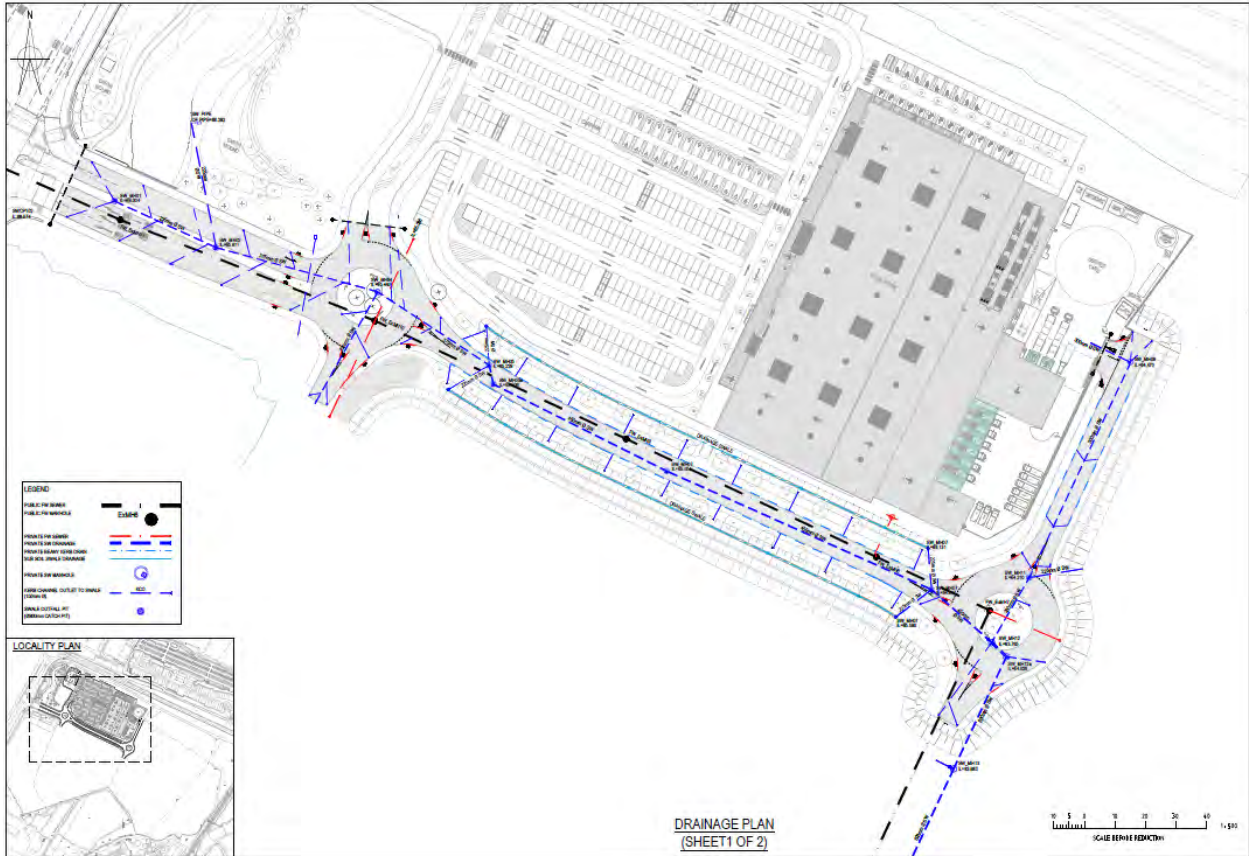
4.1.2 The plan below shows the route of the surface water sewer as it transverses the masterplan area before connecting to the ditch which then connects a stream known as the Langford Brook .



The surface water sewer is shown as a blue dotted line. As it will remain a private sewer in the ownership of the landowner there is no easement associated with it.

4.2 Foul Water

4.2.1 The route of the 600mm diameter public foul sewer under the access road is shown on the plan below



The foul sewer is shown by the dotted black line. The red lines show connection points that have been left to facilitate the future connection to serve the masterplan proposals.

5 Drainage strategy

5.1 Surface Water

5.1.1 Design parameters to be adopted

The surface water sewer was designed with a capacity to serve the masterplan proposals. In accordance with the previously agreed drainage strategy that surface water runoff from the developed site will be limited to current 'greenfield' runoff rates and onsite storage will be required. When carrying out the detailed design, the greenfield runoff rate will be estimated using the HR Wallingford *uksuds* tool. The sewer capacity of the constructed surface water drainage has been designed on this basis.

Surface water attenuation will be required to store the runoff from 1 in 100 year storm event + 20% climate change balanced against current Greenfield runoff rate for a 1 in 100 year storm. When the drainage strategy for the Tesco store was approved a Greenfield runoff rate for the site of 9.47 l/s/ha was agreed by the Local Drainage Authority (Oxfordshire County Council). When detailed planning application is made for the area within the red line the Greenfield runoff rate will need to be reconfirmed with the local Drainage Authority. The on-site attenuation/storage will be in accordance with Sustainable Drainage System (SuDS) design requirements.

5.1.2 Sustainable Drainage Systems (SuDS)

In order to limit the runoff of the current 'Greenfield' rates the drainage system to serve the development will incorporate the recommendations within the current good practice guidance for SuDS contained in CIRIA Report C753, issued in 2015. This will be used to design the onsite drainage network unless superseded in the future.

The current guidance has been reviewed and the table in section 6 indicates which SuDS methods may be applicable for the Bicester Office Park Development.

5.1.3 Water demand management.

As part of the primary infrastructure works a 150mm water main was laid under the access road and Thames Water have confirmed that this has sufficient capacity to meet the water demand requirements of the development proposals covered by the new outline planning application. However it is anticipated that rainwater harvesting may be suitable for the development and this would allow the water demand to be reduced as well providing attenuation in accordance with BS 8515:2009+A1 2013.

5.2 Foul Water

5.2.1 General

A 600 mm public foul sewer constructed as part of the primary infrastructure works with blank connection points to serve the proposed development. The flow rates from the proposed development has been estimated based on the benchmarks for B1 uses. The total flow rate is from the completed development will be very low in comparison with the capacity of public sewer. It is not anticipated that there will be any flow restrictions placed on the connections by Thames Water.

5.2.2 Design Criteria

The foul sewer network to serve the development will be designed in accordance with Sewers for Adoption 7th Edition or subsequent revisions.

6 Sustainable Drainage Systems (SuDS)

6.1 Sustainable Drainage Systems (SUDS)

SUDS will be utilised in the surface water drainage system in line with current good practice.

SUDS take account of the quality and quantity of surface water runoff together with the amenity value of surface water in the urban environment. These systems aim to provide a more sustainable solution than conventional drainage and should:

- Manage runoff flow rate, reducing the impact of urbanisation on flooding;
- Protect enhance water quality; and
- Be sympathetic to the environment setting and the needs of the local community.

There are several advantages to using SUDS that include:

- Effective control of peak flows;
- Improved water quality;
- Reduction in surface erosion;
- Reduced sewer surcharging and flooding as discharge flow rates are reduced; and
- Water conservation through rainwater harvesting and re-use.

The pollutants of concern that have been identified to include:

- Oils and Fuels. Sourced from leaks and spills;
- Suspended Solids. Sourced from traffic wear, and landscaping features;
- Chemicals. Typically detergents from washing activities;
- Litter. Sourced from bins and bin overflows, particularly within the public domain.

The surface water approach will incorporate various SUDS controls into the drainage system. It will include both source controls and larger downstream site (catchment) controls. These controls will work in series along the drainage system and it is envisaged they could include:

- Source Controls:
 - Provision of rainwater harvesting for individual buildings.
 - Use of green roofs.

Note Green roofs and rainwater harvesting would not be used in combination
- Catchment Controls including:
 - Trapped gullies as initial silt traps
 - End of line petrol interceptors.
 - Use of swales and ponds (see table 2)

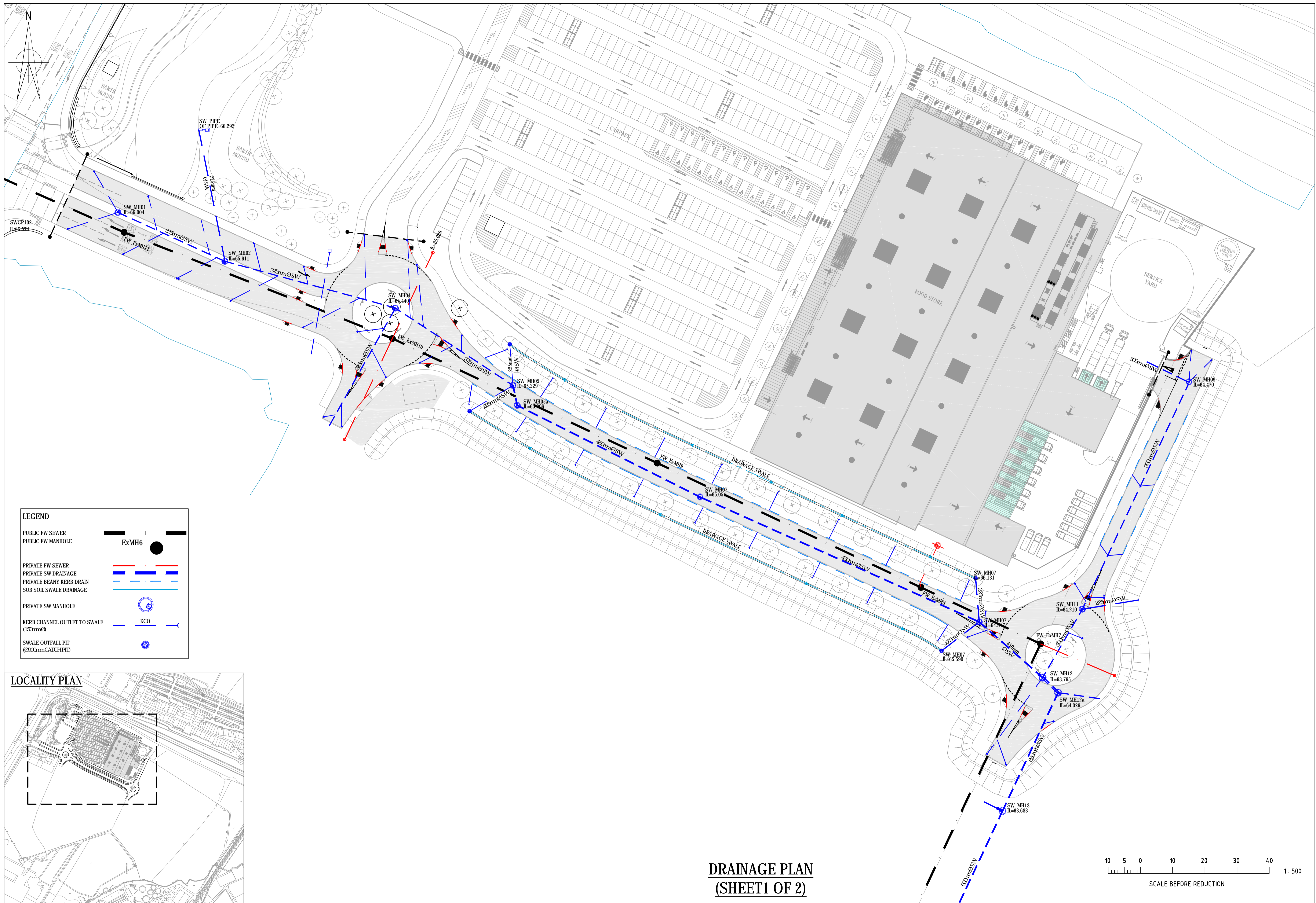
SUDS Systems	Suitability	Remarks
Ground Infiltration	x	Existing site constraints severely limit the application to this development area. These include: <ul style="list-style-type: none"> • Low design infiltration rate of 2.4×10^{-6} m/s; and • Majority of the site is underlain by clay.
Ponds/Wetlands	✓	A ponds or water features can be incorporated into the landscape proposals. The system would provide temporary storage required during storm events and promote pollutant removal.
Swales	✓	The swales were constructed adjacent to the access road to convey highway drainage. The system helps to reduce the rate of runoff provide infiltrations to the ground, and a degree of cleansing. These may be suitable for inclusion in the proposed landscaping.
French drains/Infiltration trenches	✓	An alternative to swales. The system helps to reduce the runoff, provide some infiltration or convey the storm water in pipes, and can be sited adjacent to the highways with little land take.
Below Ground Attenuation	✓	If insufficient storage can be provided above aground below ground storage tanks can be used. Note these can be used in combination with rainwater harvesting tanks see 5.1.3
Permeable Pavement	✓	Permeable pavement is recommended for all car parking areas. It is not suitable for servicing/waste storage areas

Table 2 SuDS Components

7 Conclusions.

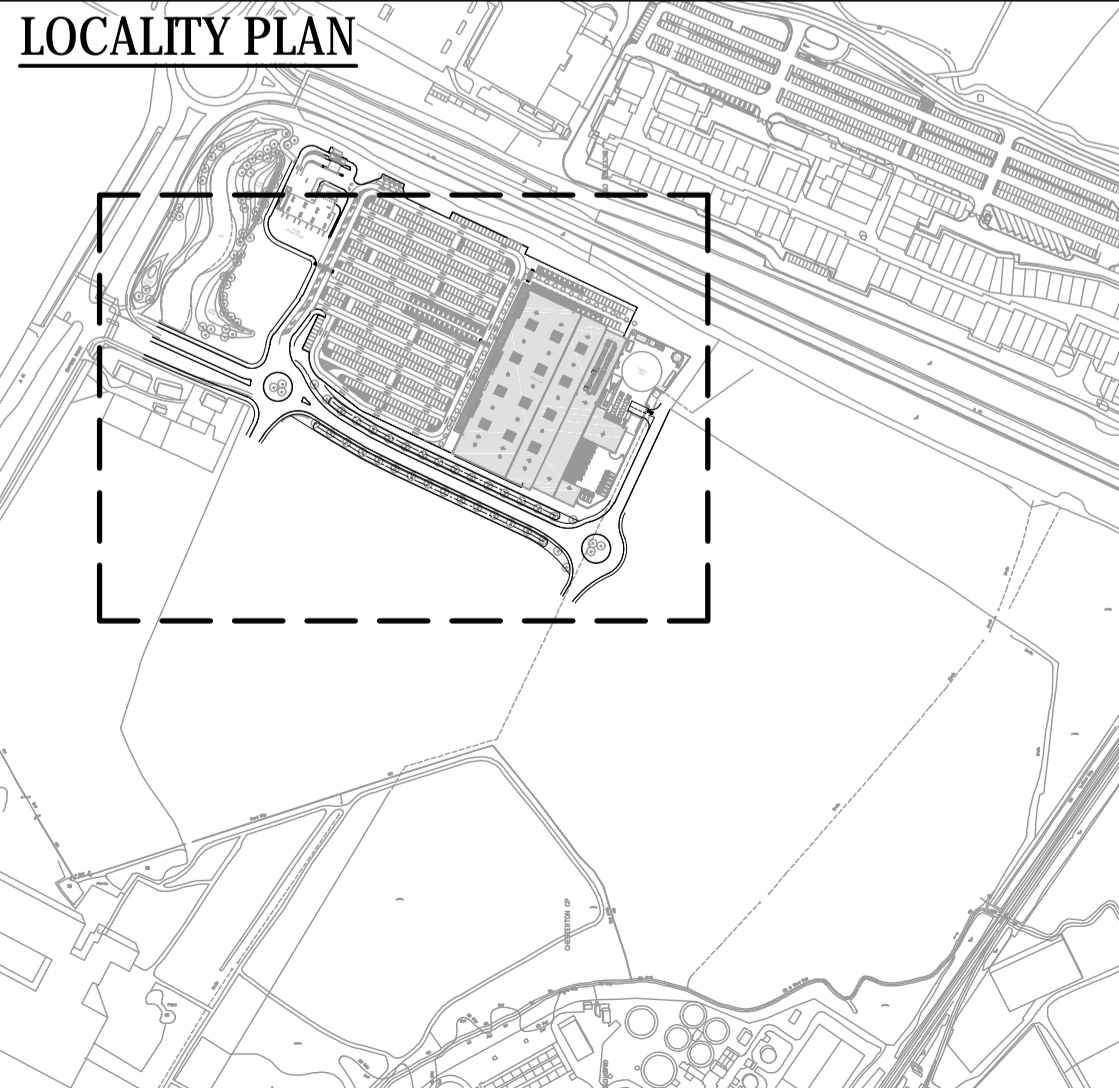
- 7.1 Primary drainage infrastructure has been constructed to serve the development proposed within the outline planning application. It has sufficient capacity to accept the proposed surface water and foul flows from the quantum and type of development proposed, without requiring any reinforcement.
- 7.2 The surface water network will incorporate SuDS good practice and the runoff will be limited to the current greenfield runoff rates. The rate will need to be confirmed with the Local Drainage Authority when detailed planning application(s) are submitted. The 1 in 100 year Greenfield runoff rate agreed for the Tesco development was 9.47 l/s/ha.
- 7.3 Green infrastructure will be provided which will facilitate a wide range of SuDS components and it is anticipated that providing the required onsite surface water storage will not present a significant challenge.
- 7.4 The public foul sewer located under the access road has sufficient capacity to serve proposed development and connections have been left to serve the development.
- 7.5 The development can incorporate rainwater harvesting as part of the SuDS strategy. In addition to providing surface water storage it also would contribute to a reduction in potable water demand as part of a water resource management strategy.

Appendix A As built drainage network drawings

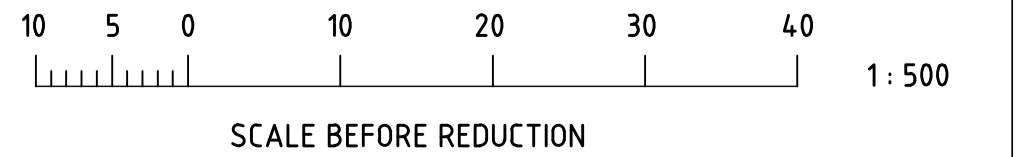


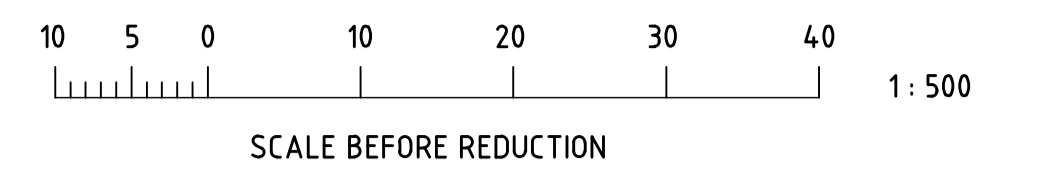
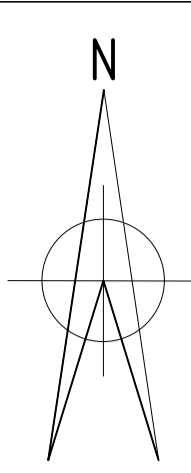
LEGEND

PUBLIC FW SEWER	
PUBLIC FW MANHOLE	
PRIVATE FW SEWER	
PRIVATE SW DRAINAGE	
PRIVATE BEANY KERB DRAIN	
SUB SOIL SWALE DRAINAGE	
PRIVATE SW MANHOLE	
KERB CHANNEL OUTLET TO SWALE (150mm Ø)	
SWALE OUTFALL PIT (Ø300mm CATCHPIT)	



**DRAINAGE PLAN
(SHEET 1 OF 2)**





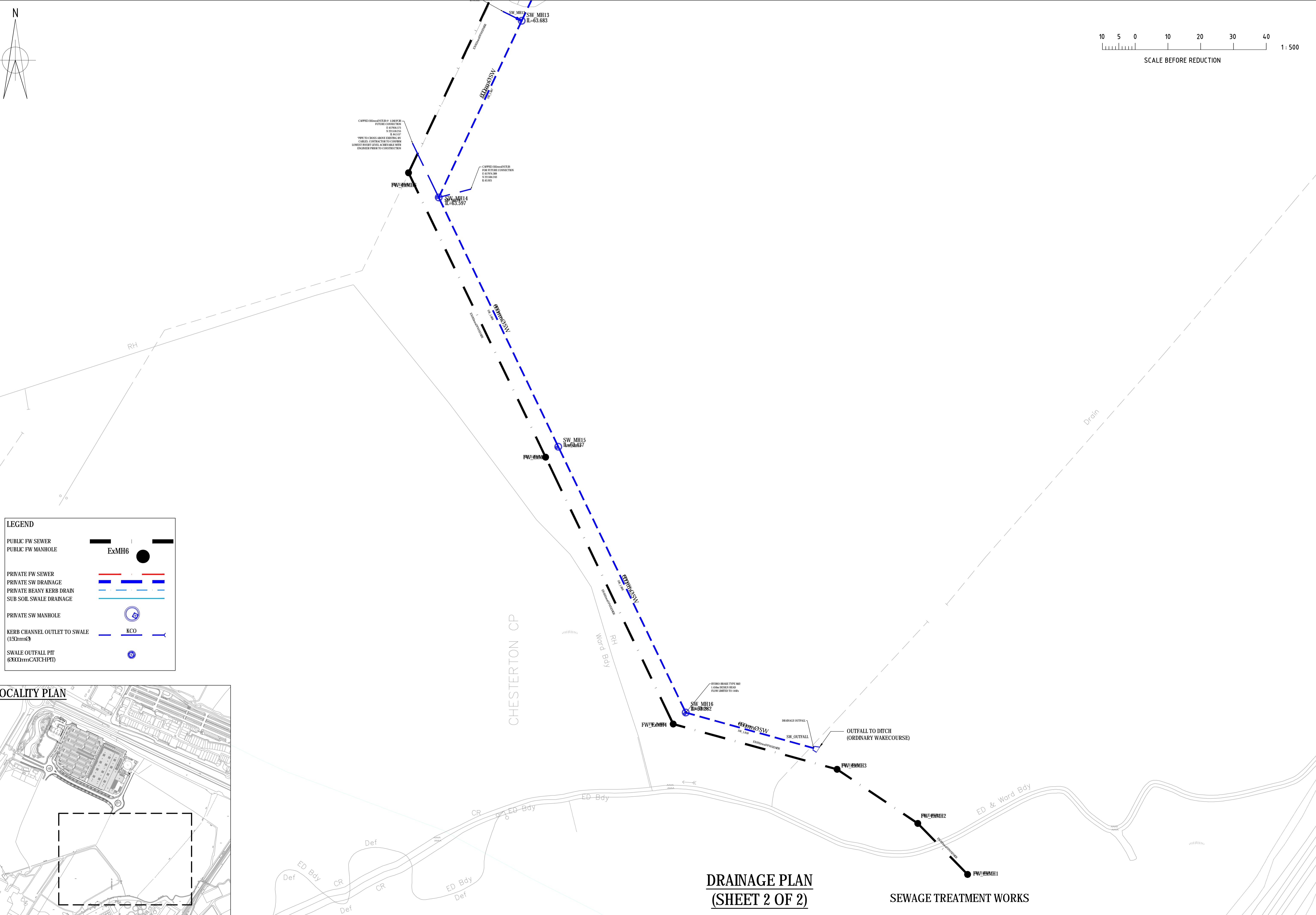
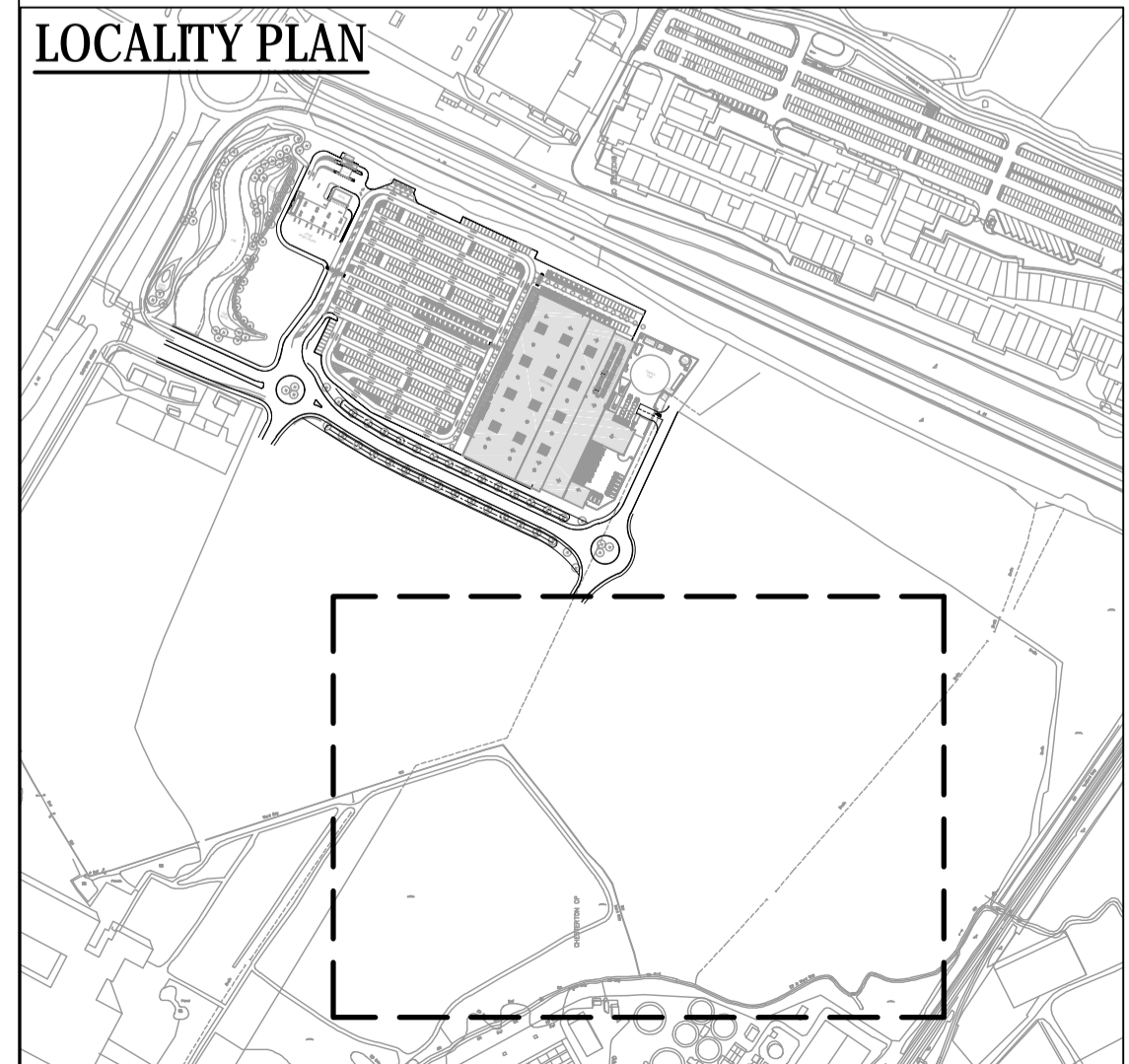
CAPPED MANHOLE FOR FUTURE CONNECTION
 & ADDRESS
 N 22184215
 N 22184217

PIPE TO BE OPEN AND EXPOSED BY
 CABLE CONTRACTOR TO CONFIRM
 LOWEST POINT LEVEL. MANHOLE WITH
 ENGLISH FROM TO CONSTRUCTION

CAPPED MANHOLE
 FOR FUTURE CONNECTION
 & ADDRESS
 N 22184242
 N 22184244

LEGEND

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PUBLIC FW MANHOLE	
PRIVATE FW SEWER	
PRIVATE SW DRAINAGE	
PRIVATE BEANY KERB DRAIN	
SUB SOIL SWALE DRAINAGE	
PRIVATE SW MANHOLE	
KERB CHANNEL OUTLET TO SWALE (150mm)	
SWALE OUTFALL PIT (200mm CATCH PIT)	



DRAINAGE PLAN
(SHEET 2 OF 2)

SEWAGE TREATMENT WORKS

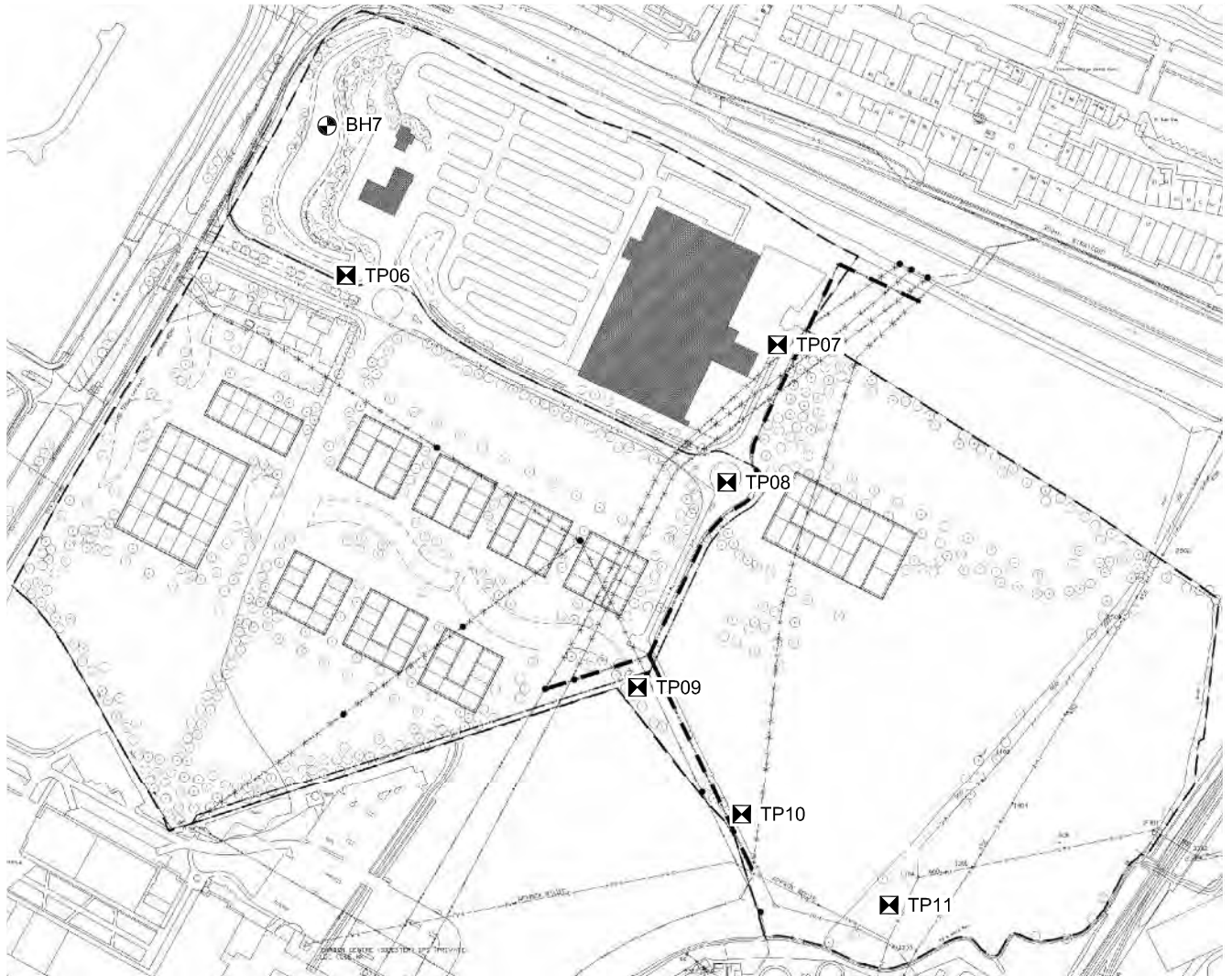
John Waiting
Buro Happold Limited
17 Newman Street
London
W1T 1PD
UK

T: +44 (0)207 927 9700



F: +44 (0)870 787 4145

Email: John.Waiting@BuroHappold.com

Appendix G Ground Investigation Location Plan



LEGEND

-  Borehole Location
-  Trial Pit Location

STRUCTURAL SOILS LIMITED



The Old School
 Stillhouse Lane
 Bedminster
 Bristol BS3 4EB
 Tel: 0117 947 1000
 Fax: 0117 947 1004
 ask@soils.co.uk
 www.soils.co.uk

CLIENT

London and Metropolitan International Developments LTD

PROJECT

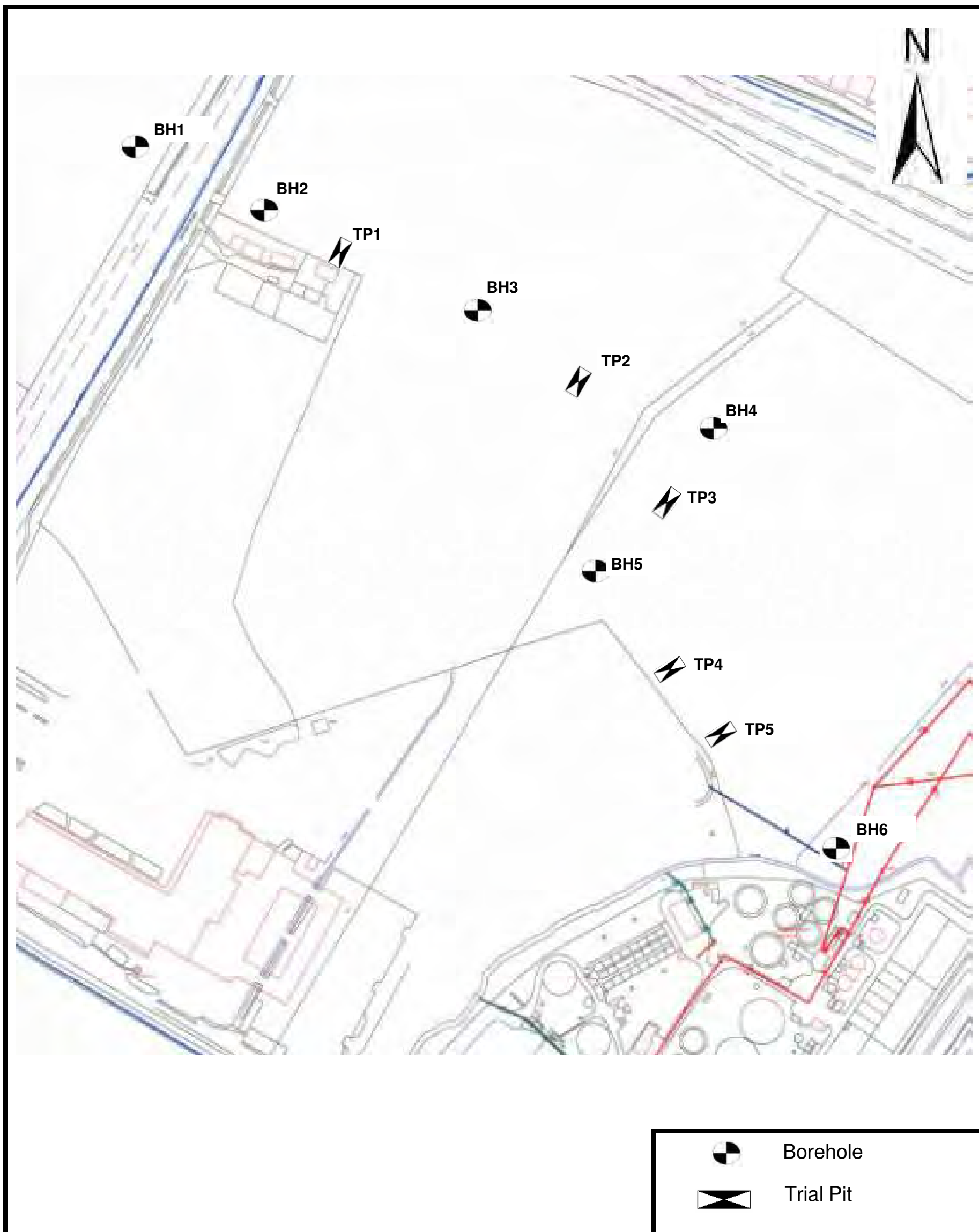
Bicester Business Park

TITLE

EXPLORATORY HOLE LOCATION PLAN

REV.	DATE	DESCRIPTION	BY	CHD.	APR.
00	29.01.2014	-	MW	WH	-

DIMENSION		SCALE	DRAWING STATUS		JOB NO.	SCALE BAR	ORIGIN SIZE	FIGURE
m		NTS	-		728724	Not To Scale	A4	2



EXPLORATORY HOLE LOCATION PLAN

Structural Soils Limited The Old School Stillhouse Lane Bedminster BS3 4EB	Site	Job no.		
	7RWG - Whitelands Farm Oxford Road FAS Diversion, Bicester	721026		Drawing no.
		2		Date
	Feb-08	Drawn by		TB
Client	Thames Water Utilities Ltd			

Clare Jones
Buro Happold Limited
Camden Mill
Lower Bristol Road
Bath
BA2 3DQ
UK

T: +44 (0)1225 320 600

F: +44 (0)870 787 4148

Email: clare.jones@burohappold.com

Appendix B: Prime Environment Biodiversity Impact Assessment

Biodiversity Impact Assessment Calculation and Skylark Plots

Site Name	Land adjacent to A41, Bicester - Ref: 17/02534/OUT
Project Number	358
Client	Trium
Work Undertaken	Biodiversity Impact Assessment Calculations
Date of Update	18/05/2018
Author	J Pedder

Method

Prime Environment have undertaken a Biodiversity Impact Assessment (BIA) as requested by Cherwell District Council (CDC). This has involved the creation of a GIS plan of the site's habitats and recorded parameters for entry into a Biodiversity Offsetting Matrix. The matrix used was the Warwickshire, Coventry and Solihull Biodiversity Offsetting Biodiversity Impact Assessment Calculator v18.

Various scenarios have been run to further understand the options available from the 'do nothing' approach, to a scheme involving a variety of on and off-site habitat creation.

In this document the red line area refers to the application site and blue line area refers to the area within the applicant's ownership and outside the application site; this is arable land with a tree line / woodland strip. As part of the application site has already been constructed with the new supermarket to the north (the access road) this has been excluded from the area in the BIA calculations. The area in used in the BIA is therefore 12.35ha as opposed to 13.1ha in the planning application. The blue line area is mostly within the flood zone, but note that this is not regularly inundated with water. Please see Figure 1 attached.

The BIA metric is used to calculate the biodiversity value of a site before and after development; this then calculates if the development is likely to cause a loss or gain to biodiversity. It is used to quantify the value of biodiversity at any site and can form an evidence base on required compensation for a development, the amount of residual biodiversity impact and if necessary the amount of required offsite compensation (Biodiversity Offsetting). Should the Biodiversity Impact

South and South East

0330 2233 806
southeast@primeenvironment.co.uk

Midlands and North

0330 2233 825
central@primeenvironment.co.uk

Assessment calculate a residual loss to biodiversity, once the mitigation hierarchy has been followed and the development is in accordance with all other local and national planning policy and law, it may be suitable to apply principals of biodiversity offsetting.

However, for the purposes of the BIA the whole area of the applicant's land had been included in the calculations (i.e. red line and blue line) with the aim of demonstrating whether a net gain to biodiversity can be achieved within the site through an appropriate landscape plan without providing Biodiversity Offsetting on third party land.

In each scenario it has been assumed that all boundary hedges and woodland will be retained, except the tree line along the A41/Oxford Road which has not been included within the indicative masterplan, and that the woodland strip that divides the two arable fields (in the Blue Line Area) will be retained. In accordance with CDC requirements, the baseline for the calculation also includes the wet ditch and hedge line in the west of the site that have recently been removed.

Results

Scenario 1

The first scenario is based on the EIA. This assumes that 9.4ha are built on and that there are areas of amenity grassland within the built area. The ecology chapter recommendations for wetland SUDs and a wildflower grassland strip would provide a net loss of -15.1.

By adding intensive green roofs to half of the build area, the impact is greatly reduced (to -4.66). Including a small orchard (0.87 ha) within the landscaping of the built area brings the AIA up to +1 (orchards score well because they are of high biodiversity value, they establish relatively quickly, and they have a low risk of failure).

A positive BIA is therefore possible without creating new habitats outside of the Red Line Area.

Given the expensive and restrictive nature of green roofs, green roofs have not been included in the following scenarios.

Scenario 2

In a scenario where the whole red line area (12.35ha) is required for buildings and hardstanding with no soft landscaping, the BIA is -92.73.

To investigate whether there is a 'quick fix' that allows maximum development, Prime Environment have run an example where the whole area is developed and relatively simple compensation is provided by creating grassland over the blue line area. This however also falls short and would result in a BIA of -25.62.

However, if orchard trees are planted on all of the blue line area, the whole 12.35 ha can be developed. The result for this is +7.78.

The Applicant can therefore rely on habitat creation within the blue line area to demonstrate that a scheme with a net biodiversity gain is easily achievable.

Scenario 3

However, although orchards are of value, there are specific impacts to wildlife that were recorded in the Environmental Statement as a result of the Proposed Development requiring mitigation with respect to ground nesting birds; a large orchard would not provide appropriate compensation as skylark plots would not work within a wooded landscape. Prime Environment have therefore calculated a final BIA using a mixed approach. This includes amenity grassland and wetland SUDs within the Red Line Area, a boundary hedgerow between the Red Line and Blue Line areas and compensation habitats (a small orchard, pond and a mixture of scrub and grassland) within the Blue Line Area. This results in **a BIA of +1, whilst also providing benefit to a range of species such as amphibians, reptiles and birds.**

A summary of the results is provided as an appendix. Copies of the BIA scenarios are included as an Appendix to this document.

Conclusion

During the detailed design of the scheme, an appropriate Landscaping and Ecology plan will be produced which details habitat creation within the site and includes an updated Biodiversity Impact Assessment demonstrating that the plan will provide a net gain. This BIA has demonstrated that there are several ways in which the scheme is able to achieve a net gain. This is possible by creating habitats within the applicant's land either within the Red Line Area or Blue Line Area, without resorting to third party land.

Sylark Plots

CDC has raised concerns that the proposed skylark mitigation plots will be too close to the built environment. To address this, we now propose an amended approach.

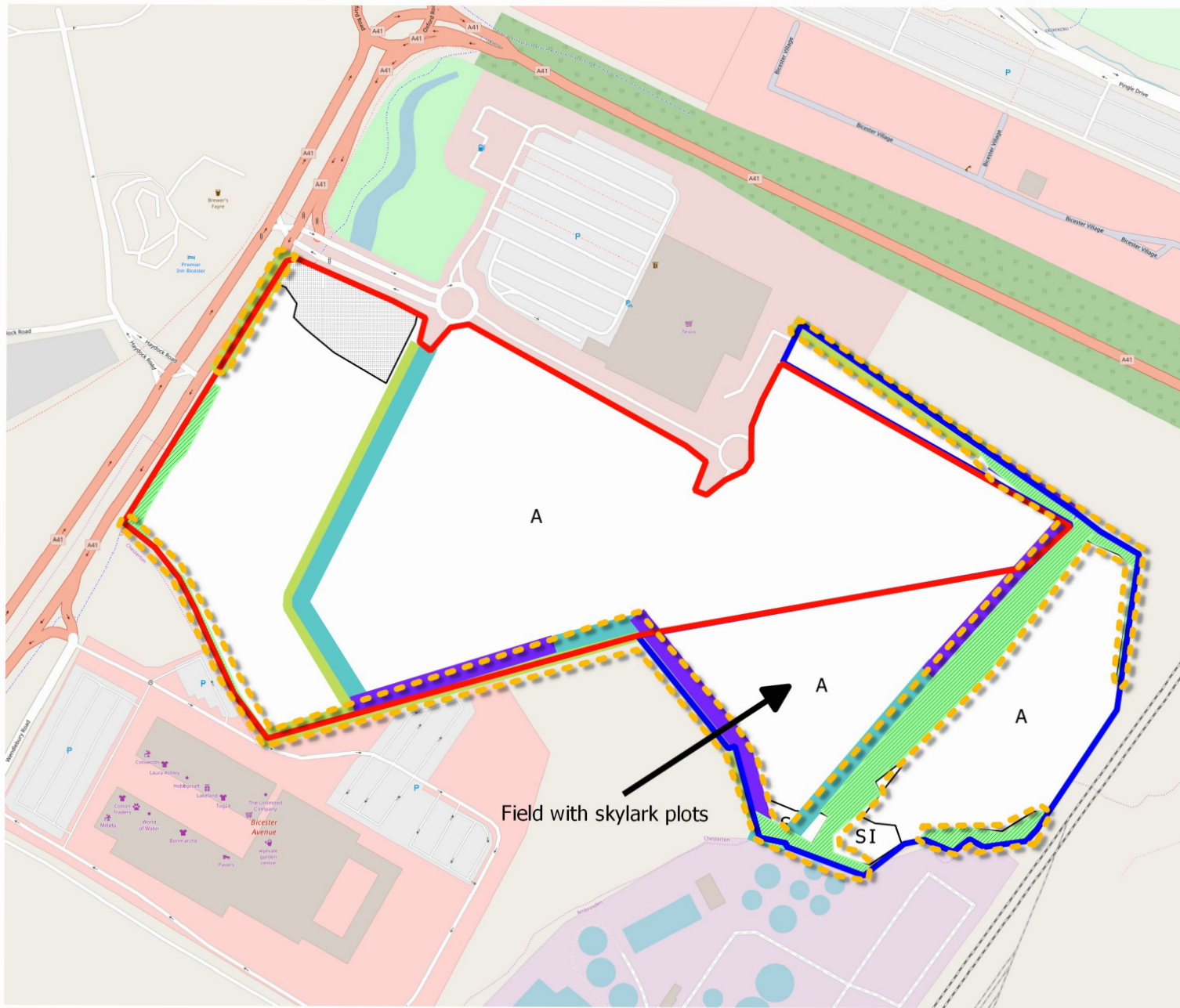
The skylark plots will be included within the Blue Line Area. These will be provided within the arable field, or within areas of habitat created to balance the BIA of the scheme.

Plots will follow the prescription of the *Higher Level Stewardship (HLS) Scheme AB4: skylark plots* as far as is possible (the retained habitat size is smaller than the recommended 10 ha minimum). The plots aim to provide a bare area close to ground cover for foraging and 50 m from trees, hedges and buildings. Each plot will be at least three meters wide and with a minimum area of 16 square metres. Four plots will be spaced across the larger of the two retained arable areas (or grassland if they are converted from crop) to provide a density of two plots per ha.

Appendix 1 – Figure 1

Bicester Office Park Biodiversity Impact Assessment

Figure 1 - Habitat Plan



Legend

- Red Line Area
- Blue Line Area
- Habitats retained in all scenarios

Area Measurements

- A Arable
- SI Poor semi-improved grassland
- Mixed Wood - Plantation
- Bare Ground

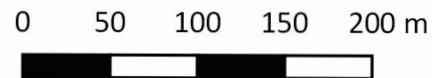
Linear Measurements

- Hedges
- Wet Ditch
- Ditch - Dry

Client: Trium Consulting

Date: 24/05/2018

Map data copyrighted OpenStreetMap contributors and available from <https://www.openstreetmap.org>



Indicative only. Do not scale.



Appendix 2 – BIA Scenarios

Summary

Notes		Building and Hardstanding	Habitat Score	Linear Habitat Score	Total
Scenario 1	Mitigation and compensation proposed in the EIA – amenity grassland, wildflower strip, planted wetland/SUDs within the Red Line Area. No change to Blue Line Area habitats (remain arable)	9.4 ha	-9.9	-5.2	-15.1
	Green roofs – all habitats within Red Line as above, plus half of the building area is an intensive green roof. No change to Blue Line Area (remain arable)	9.4 ha	+0.54	-5.2	-4.66
	Green roofs – with the addition of a ‘good’ quality orchard within the Red Line (0.87ha). No change to Blue Line Area (remain arable)	9.4 ha	6.2	-5.2	+1
Scenario 2	Complete loss of all habitats within the Red Line Area. No change to Blue Line Area.	12.35 ha	-87.53	-5.2	-92.73
	Complete loss of all habitats within the Red Line Area and marginal improvements to the Blue Line Area (conversion of arable to moderate quality semi improved grassland).	12.35 ha	-20.42	-5.2	-25.62
	Complete loss of all habitats within the Red Line Area and conversion of all non-woodland habitats in the Blue Line Area into good quality orchard.	12.35 ha	+16.09	-5.2	+7.78
Scenario 3	Balanced: Sensitive development within the Red Line Area with amenity grassland and planted SUDs, a hedge defining the boundary between the Red and Blue Line Areas. Habitat creation in the Blue Line Area includes wildflower meadow grassland, scattered scrub and a small orchard.	9.4 ha	+3.3	-2.3	+1

Warwickshire Coventry and Solihull - Biodiversity Impact Assessment Calculator

v. 18.3 08/08/2014
 Amendment from v18.2 only affects green roofs, for other habitats v18.2 still usable.
 Please fill in both tables

KEY	
No action required	
Enter value	
Drop-down menu	
Calculation	
Automatic lookup	
Result	

Local Planning Authority:	
Site name:	Bicester SCENARIO 1
Planning application reference number:	
Assessor:	
Date:	

Please do not edit the formulae or structure
To condense the form for display hide vacant rows, do not delete them
If additional rows are required, or to provide feedback on the calculator please contact WCC Ecological Services

Existing habitats on site			Habitat distinctiveness		Habitat condition		Habitats to be retained with no change within development		Habitats to be retained and enhanced within development		Habitats to be lost within development		Comment
T. Note	code	Phase 1 habitat description	Distinctiveness	Score	Condition	Score	Area (ha)	Existing value	Area (ha)	Existing value	Area (ha)	Existing value	
Direct Impacts and retained habitats			A		B		C		E		G		
			A x B x C = D		A x B x E = F		A x B x C = D		A x B x E = F		A x B x G = H		
J11		Other: Arable	Low	2	Poor	1	2.34	4.68	0.00		12.35	24.71	Poor arable field (west)
J11		Other: Arable	Low	2	Moderate	2	2.01	8.06	0.00				Moderate arable field (east)
J113		Grassland: Set-aside / Arable field margins	High	6	Poor	1	0.10	0.63	0.00		0.20	1.21	Arable margin
B22		Grassland: Semi-improved neutral grassland	Medium	4	Moderate	2	0.00	0.00	0.00		0.06	0.46	Ditch bank
B22		Grassland: Semi-improved neutral grassland	Medium	4	Poor	1	0.25	0.99	0.00				Semi-improved grass
J13		Other: Ephemeral/short perennial	Low	2	Moderate	2	0.00	0.00	0.00		0.05	0.18	Spoil piles
A112		Woodland: Broad-leaved plantation	Medium	4	Poor	1	0.56	2.22	0.00		0.12	0.49	Woodland / tree lines
Total							5.26	16.57	0.00	0.00	12.78	27.06	
											ΣD + ΣF + ΣH		
											Site habitat biodiversity value		43.63
Indirect Negative Impacts							Value of loss from indirect impacts						
Before/after impact			K				K x A x B = Li, Lii		Li - Lii				
Before													
After													
Before													
After													
Before													
After													
Before													
After													
Before													
After													
Total			0.00				M		0.00				
											HIS = J + M		
											Habitat Impact Score (HIS)		27.06

CAUTION - Destruction of habitats of high distinctiveness, e.g. lowland meadow or ancient woodland, may be against local policy. Has the mitigation hierarchy been followed, can impact to these habitats be avoided?
 Any unavoidable loss of habitats of high distinctiveness must be replaced like-for-like.

Proposed habitats on site (Onsite mitigation)			Target habitats distinctiveness		Target habitat condition		Time till target condition		Difficulty of creation / restoration		Habitat biodiversity value	Comment	
T. Note	code	Phase 1 habitat description	Distinctiveness	Score	Condition	Score	Time (years)	Score	Difficulty	Score			
Habitat Creation			N		O		Q		R		(N x O x P) / Q / R		
			N		O		Q		R				
B22		Grassland: Semi-improved neutral grassland	Medium	4	Good	3	5 years	1.2	Medium	1.5	3.83	wildflower meadow strip in ecology ES	
G1		Wetland: Standing water	High	6	Moderate	2	5 years	1.2	Medium	1.5	0.40	wetland prescribed in the ecology ES	
n/a		Built Environment: Buildings/hardstanding	none	0	Poor	1	5 years	1.2	Low	1	0.00	9.4 ha stated in the D&A - half which account for green roofs	
B22		Grassland: Semi-improved neutral grassland	Medium	4	Poor	1	5 years	1.2	Medium	1.5	10.44	green roofs on half the built surfaces	
A5		Woodland: Orchard	High	6	Good	3	15 years	1.7	Low	1	9.18	Minimum orchard area altered to provide +1 benefit.	
Total													
Habitat Enhancement							Existing value S (= F)				((N x O x P) - S) / Q / R		
Total			0.00								Trading down correction value		0.00
											Habitat Mitigation Score (HMS)		33.26
											HBIS = HMS - HIS		
											Habitat Biodiversity Impact Score		6.20
											Percentage of biodiversity impact loss		Gain

KEY	
No action required	
Action required	
Drop-down menu	
Calculation	
Automatic lookup	
Overall Result	
	Loss to biodiversity
	Gain to biodiversity

Biodiversity Impact Assessment Summary

Site name:	Bicester SCENARIO 1
Planning reference number:	

Habitats	Area (ha)	Habitat Biodiversity Value
Total existing area onsite	18.04	43.63
Habitats negatively impacted by development Habitat Impact Score	12.78	27.06
On site habitat mitigation Habitat Mitigation Score	12.78	33.26
Habitat Biodiversity Impact Score		
If -ve further compensation required		6.20
Percentage of biodiversity impact		
Linear features	Length (km)	Linear Biodiversity Value
Total existing length onsite	1.80	17.18
Linear features negatively impacted by development Linear Impact Score	0.29	5.20
On site linear mitigation Linear Mitigation Score	0.00	0.00
Linear Biodiversity Impact Score		
If -ve further compensation required		-5.20
Percentage of linear biodiversity impact		100.00

CAUTION - Destruction of habitats of high distinctiveness, e.g. lowland meadow, ancient woodland or species-rich hedgerows, may be against local policy. Has the mitigation hierarchy been followed, can impact to these habitats be avoided? Any unavoidable loss of habitats of high distinctiveness must be replaced like-for-

For any questions with regard to biodiversity impact and this development please contact Warwickshire County Council Ecological Services:

email: planningecology@warwickshire.gov.uk
tel: 01926 418060

If there is an anticipated loss to biodiversity and no further ecological enhancements can be incorporated within the development it may be possible to compensate for this loss through a biodiversity offsetting scheme.

Please contact The Environment Bank for discussions on potential receptor sites in your area:

email: Imartland@environmentbank.com
tel: 01926 412772



Warwickshire Coventry and Solihull - Biodiversity Impact Assessment Calculator

v. 18.3 08/08/2014
 Amendment from v18.2 only affects green roofs, for other habitats v18.2 still usable.
 Please fill in both tables

KEY	
No action required	
Enter value	
Drop-down menu	
Calculation	
Automatic lookup	
Result	

Local Planning Authority:	
Site name:	Bicester SCENARIO 2
Planning application reference number:	
Assessor:	
Date:	

Please do not edit the formulae or structure
To condense the form for display hide vacant rows, do not delete them
If additional rows are required, or to provide feedback on the calculator please contact WCC Ecological Services

Existing habitats on site Please enter all habitats within the site boundary			Habitat distinctiveness		Habitat condition		Habitats to be retained with no change within development		Habitats to be retained and enhanced within development		Habitats to be lost within development		Comment	
T. Note	code	Phase 1 habitat description	Habitat area (ha)	Distinctiveness	Score	Condition	Score	Area (ha)	Existing value	Area (ha)	Existing value	Area (ha)		Existing value
Direct Impacts and retained habitats			A	B	C	D	E	F	G	H	I	J		
J11		Other: Arable	14.69	Low	2	Poor	1	0.00	0.00	14.69	29.38			Poor arable field (west)
J11		Other: Arable	2.01	Low	2	Moderate	2	0.00	0.00	2.01	8.06			Moderate arable field (east)
J113		Grassland: Set-aside / Arable field margins	0.31	High	6	Poor	1	0.00	0.00	0.31	1.84			Arable margin
B22		Grassland: Semi-improved neutral grassland	0.06	Medium	4	Moderate	2	0.00	0.00	0.06	0.46			Ditch bank
B22		Grassland: Semi-improved neutral grassland	0.25	Medium	4	Poor	1	0.00	0.00	0.25	0.99			Semi-improved grass
J13		Other: Ephemeral/short perennial	0.05	Low	2	Moderate	2	0.00	0.00	0.05	0.18			Spoil piles
A112		Woodland: Broad-leaved plantation	0.68	Medium	4	Poor	1	0.56	2.22	0.12	0.49			Woodland / tree lines
Total			18.04					0.56	2.22	0.00	0.00	17.49	41.41	
Indirect Negative Impacts Including off site habitats			K	Value of loss from indirect impacts K x A x B = Li, Lii		Li - Lii		Site habitat biodiversity value		ΣD + ΣF + ΣH		43.63		
Before/after impact														
Before														
After														
Before														
After														
Before														
After														
Before														
After														
Before														
After														
Total			0.00					M	0.00				HIS = J + M	
Habitat Impact Score (HIS)												41.41		

CAUTION - Destruction of habitats of high distinctiveness, e.g. lowland meadow or ancient woodland, may be against local policy. Has the mitigation hierarchy been followed, can impact to these habitats be avoided?
 Any unavoidable loss of habitats of high distinctiveness must be replaced like-for-like.

Proposed habitats on site (Onsite mitigation)			Target habitats distinctiveness		Target habitat condition		Time till target condition		Difficulty of creation / restoration		Habitat biodiversity value (N x O x P) / Q / R	Comment	
T. Note	code	Phase 1 habitat description	Area (ha)	Distinctiveness	Score	Condition	Score	Time (years)	Score	Difficulty			Score
Habitat Creation			N	O	P	Q	R	S	T	U	V		
n/a		Built Environment: Buildings/hardstanding	12.35	none	0	Good	3	5 years	1.2	Low	1	0.00	whole Red Line Area
A5		Woodland: Orchard	5.14	High	6	Good	3	15 years	1.7	Low	1	54.39	
Total			17.49										
Habitat Enhancement			Existing value S (= F)								((N x O x P) - S) / Q / R		
Total			0.00										
Trading down correction value											0.00		
Habitat Mitigation Score (HMS)											54.39		
Habitat Biodiversity Impact Score											12.98	Gain	
Percentage of biodiversity impact loss													

KEY	
No action required	
Action required	
Drop-down menu	
Calculation	
Automatic lookup	
Overall Result	

Loss to biodiversity	
Gain to biodiversity	

Biodiversity Impact Assessment Summary

Site name:	Bicester SCENARIO 2
Planning reference number:	

Habitats	Area (ha)	Habitat Biodiversity Value
Total existing area onsite	18.04	43.63
Habitats negatively impacted by development Habitat Impact Score	17.49	41.41
On site habitat mitigation Habitat Mitigation Score	17.49	54.39
Habitat Biodiversity Impact Score		
If -ve further compensation required		12.98
Percentage of biodiversity impact		
Linear features	Length (km)	Linear Biodiversity Value
Total existing length onsite	1.80	17.18
Linear features negatively impacted by development Linear Impact Score	0.29	5.20
On site linear mitigation Linear Mitigation Score	0.00	0.00
Linear Biodiversity Impact Score		
If -ve further compensation required		-5.20
Percentage of linear biodiversity impact		100.00

CAUTION - Destruction of habitats of high distinctiveness, e.g. lowland meadow, ancient woodland or species-rich hedgerows, may be against local policy. Has the mitigation hierarchy been followed, can impact to these habitats be avoided? Any unavoidable loss of habitats of high distinctiveness must be replaced like-for-

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tel: 01926 412772



Warwickshire Coventry and Solihull - Biodiversity Impact Assessment Calculator - Linear Features

Please fill in both tables

KEY	
No action required	
Enter value	
Drop-down menu	
Calculation	
Automatic lookup	
Result	

Linear Features
Hedges and other linear features can offer a higher biodiversity value per length than a standard area of habitat due to factors such as connectivity and must therefore be compensated for in parallel to the standard metric.

Please do not edit the formulae or structure
To condense the form for display hide vacant rows, do not delete them
If additional rows are required, or to provide feedback on the calculator please contact WCC Ecological Services

T. Note	code	Existing linear features on site			Linear distinctiveness		Linear condition		Linear Biodiversity Value		Linear features to be lost within development		Comment	
		Phase 1 habitat description	Feature length (km)	Distinctiveness	Score	Condition	Score	Length (km)	Existing value	Length (km)	Existing value	Length (km)		Existing value
		Direct Impacts and retained features		A	B	C	D	E	F	G	H			
G2		Ditches: Running water	0.29	High	6	Good	3	0.00	0.00	0.29	5.20			
G2		Ditches: Running water	0.25	High	6	Moderate	2	0.25	3.05	0.00	0.00			
J26		Ditches: Dry ditch	0.53	Low	2	Poor	1	0.53	1.07	0.00	0.00			
J23		Hedges: Hedge with trees	0.43	Medium-High	5	Moderate	2	0.43	4.32	0.00	0.00			
J231		Hedges: Native species rich hedge with trees	0.30	High	6	Moderate	2	0.30	3.54	0.00	0.00			
Total			1.80					1.52	11.98	0.00	0.00	0.29	5.20	
										Site Linear Biodiversity Value		ΣD + ΣF + ΣH		17.18
Before/after impact		Indirect Negative Impacts			Value of loss from indirect impacts		K x A x B = L, Lii		L - Lii		M		HIS = J + M	
Before		K												
After														
Before														
After														
Before														
After														
Before														
After														
Before														
After														
Total			0.00					M	0.00			5.20		

CAUTION - Destruction of features of medium or high distinctiveness, e.g. hedgerows and streams, may be against local policy. Has the mitigation hierarchy been followed, can impact to these habitats be avoided? Any unavoidable loss of valuable habitats must be replaced like-for-like. E.G. Loss of hedgerows must be replaced with similar or better hedgerows. All newly planted hedges should be native species-rich hedgerows.

T. Note	code	Proposed linear features on site (Onsite mitigation)			Target linear distinctiveness		Target linear condition		Time till target condition		Difficulty of creation / restoration		Linear biodiversity value (N x O x P) / Q / R	Comment
		Phase 1 habitat description	Length (km)	Distinctiveness	Score	Condition	Score	Time (years)	Score	Difficulty	Score			
		Linear Creation	N	O	P	Q	R							
J211		Hedges: Native species rich intact hedge	0.27	High	6	Good	3	15 years	1.7	Low	1	2.90		
Total			0.27											
Linear Enhancement		Existing value S (= F)												
Total			0.00											
										Trading down correction value		0.00		
										Linear Mitigation Score (LMS)		2.90		
										LBIS = LMS - LIS		-2.30		
										Linear Biodiversity Impact Score		-2.30		
										Percentage of linear impact loss		44.23		

KEY	
No action required	
Action required	
Drop-down menu	
Calculation	
Automatic lookup	
Overall Result	Loss to biodiversity Gain to biodiversity

Biodiversity Impact Assessment Summary

Site name:	Bicester SCENARIO 3
Planning reference number:	

Habitats	Area (ha)	Habitat Biodiversity Value
Total existing area onsite	18.04	43.63
Habitats negatively impacted by development Habitat Impact Score	17.49	41.41
On site habitat mitigation Habitat Mitigation Score	17.49	44.71
Habitat Biodiversity Impact Score		
If -ve further compensation required		3.30
Percentage of biodiversity impact		
Linear features	Length (km)	Linear Biodiversity Value
Total existing length onsite	1.80	17.18
Linear features negatively impacted by development Linear Impact Score	0.29	5.20
On site linear mitigation Linear Mitigation Score	0.27	2.90
Linear Biodiversity Impact Score		
If -ve further compensation required		-2.30
Percentage of linear biodiversity impact		44.23

CAUTION - Destruction of habitats of high distinctiveness, e.g. lowland meadow, ancient woodland or species-rich hedgerows, may be against local policy. Has the mitigation hierarchy been followed, can impact to these habitats be avoided? Any unavoidable loss of habitats of high distinctiveness must be replaced like-for-

For any questions with regard to biodiversity impact and this development please contact Warwickshire County Council Ecological Services:

email: planningecology@warwickshire.gov.uk
tel: 01926 418060

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Appendix C: Initial Estimates of Greenfield Runoff and Onsite Storage

Technical Note

Project Bicester Office Park – Planning Support

Subject Bicester

Project no 004003

Date 13 March 2018

Revision	Description	Issued by	Date	Approved (signature)
00	Initial Estimates of Greenfield Runoff and Onsite Storage	JS	12/03/2018	JW
01	Updated with Latest Masterplan	JT	04/06/2018	JW

1 Introduction and Background

An outline planning application (OPA) has been submitted for the proposed Bicester Office Park and this was accompanied by a drainage strategy that identified that the surface water runoff from the completed development would need to be limited to greenfield runoff rates. It also noted that the greenfield runoff rates would be calculated using the methodology set out on the uksuds website.

Oxford County Council as drainage authority have made the following observation;

OCC considered that the drainage proposals were not adequately described within the strategy document. For an outline application, the proposal needs to describe the attenuation storage volumes that are required to provide mitigation and achieve compliance with the proposed allowable discharge rates. Typically the applicant must show by way of a sketch, which describes the SuDS features and demonstrates that they fit within the red line application boundary. A supporting calculation needs to be provided and for initial sizing calculations in support of outline application the toolkit provided by the 'UK Suds' website is acceptable to OCC. These considerations were absent from the application.

This technical note has been prepared to evaluate the greenfield discharge rate(s) for the Bicester Office Park site and to establish an estimate for the amount of onsite storm water storage that will be required. This is to show that the volume can be easily be accommodated given the development proposals contained within the parameter plan see figure 1-1 below.

The area of site within the OPA red line is 13.1 Ha, however this includes the access road and associated landscaping that is already in use and has an operational drainage system, when this is removed the greenfield undeveloped area is 10.85 Ha. In addition to this part of the developable area lies within flood zones 2 and 3. When considering both the greenfield runoff rates and the area available for storage these areas should be ignored. It has also been assumed that 33.3% of the area being developed will be soft landscaping and using this and the building footprint areas assumed within the parameter plans an approximate area of hard landscaping (roads, parking and servicing areas) was estimated. The derivation of these areas is set out in table 1.

Given the above the following areas have been assumed within the calculations

Greenfield area to calculate runoff rates 9.41 Ha

Proposed building and hardstanding (impermeable area) = 7.27 Ha

Proposed soft landscaping outside flood zones 2/3 = 2.14 Ha

Table 1 Approximate Site Areas

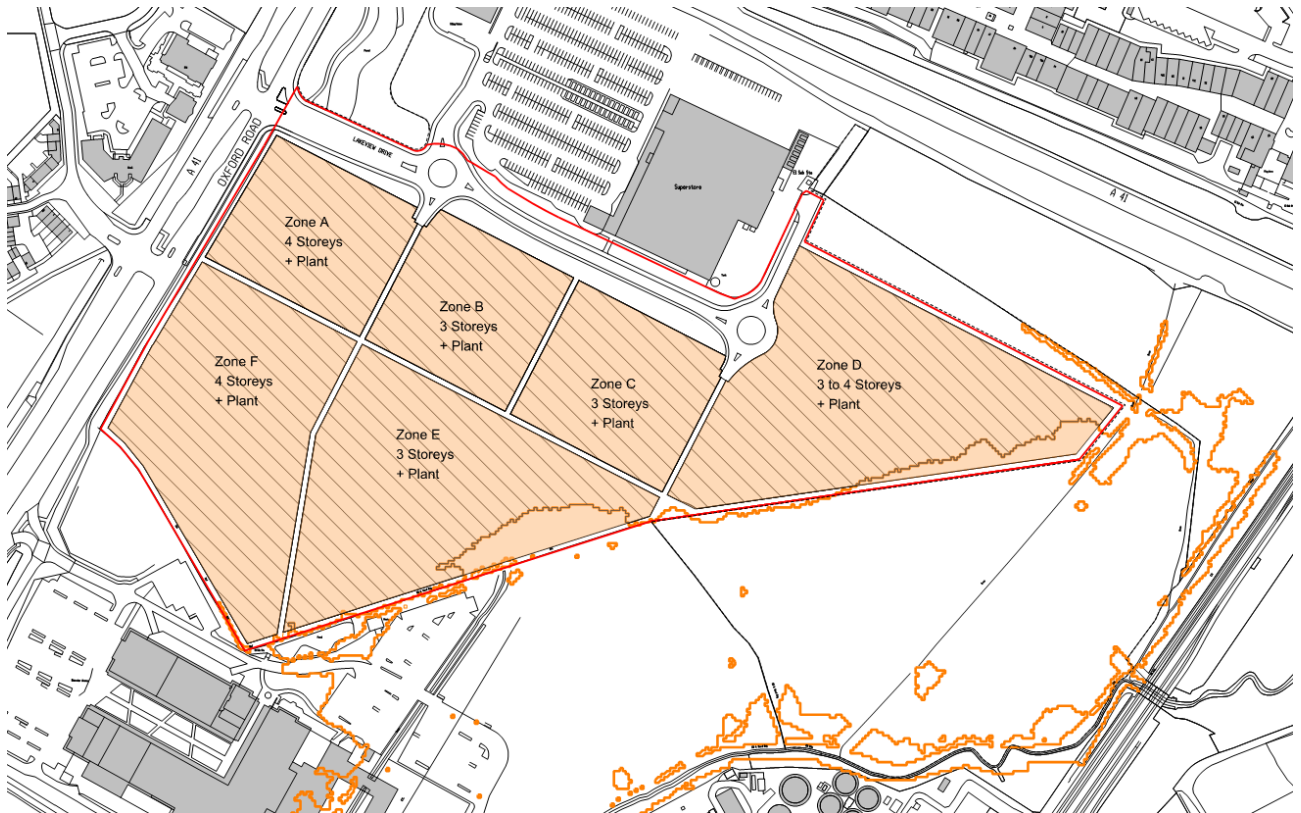
Zone	No of Storeys	Max floor area (m ²)	Building footprint (m ²)	Development Area (m ²)	Developable Area (m ²)	Flood zones 2/3 (m ²)	Soft Landscaping (m ²)	Hard Landscaping (m ²)
A	4	5460	1365	10875	10875	0	3589	5921
B	3	7740	2580	10745	10745	0	3546	4619
C	3	7740	2580	11650	11650	0	3845	5226
D	3	14390	4797	27730	20180	7550	1601	13782
E	3	11610	3870	25000	19750	5250	3000	12880
F	4	13060	3265	22470	20900	1570	5845	11790
Total	N/A	60000	18457	108470	94100	14370	21425	54218

Table 1 assumes buildings and hard landscaping occupy 66.7% of development area.

It is noted that non critical hardscaping can be located in flood zone 2 but this has been ignored in order to produce a robust assessment of greenfield run off rates and available storage areas.

The figure below shows the extent of the site boundary and the proposed development areas. The orange line indicates the limit of the 1 in 100 year flood extent.

Figure 1—1 Parameters Plan



The primary surface water drainage infrastructure to serve the site has already been constructed and sized to accommodate the greenfield run off rates for both this and the adjacent developed site occupied by a Tesco store. When the drainage strategy for the Tesco store was approved a greenfield runoff rate for the site of 9.47 l/s/ha was agreed by the Local Drainage Authority (Oxfordshire County Council). This rate was calculated to restrict the greenfield runoff rates for storm event up to and including a 1 in 100 year storm event plus climate change. This resulted in an overall discharge rate from the site was limited to 71l/s for storms up to and including a 1 in 100 year event plus climate change.

Trial pits and boreholes were carried out around the site as part of a ground investigation in 2014. It was discovered that upper soils within the site predominately consists of stiff firm clay. This type of soil makes infiltration into the ground difficult due to its very low permeability and this soil type has been assumed in order to calculate the greenfield run off rate and the calculation of the storage volume. Should additional investigations identify areas of the site with permeable soils the greenfield runoff rate will reduce but so will the required storage volume as infiltration of the surface water runoff will be part of the SuDS proposals.

2 Greenfield Runoff rates

2.1 Methodology

The greenfield runoff rate has been estimated using the HR Wallingford uksuds tool. Due to the nature of the soil of the site and its poor infiltration rate a soil type of 4 was specified for when using the IH124 method. The total area of greenfield land that within the development is 10.874 Ha. However, as the flood plain is not to be developed and the greenfield runoff rate is to be maintained in developed areas only, the flood plain has been removed from the total area, thus 9.41 Ha has been used in the calculations.

2.2 Results

Results calculated are found in the table below:

Table 2 Calculated Greenfield Runoff Rates

Greenfield Peak Flow Runoff	Greenfield Runoff rate (l/s)	Greenfield Runoff rate (l/s/Ha)
Mean annual, Q_{bar}	39.19	4.16
1 in 1 year, Q_{1yr}	33.31	3.54
1 in 30 year, Q_{30yr}	90.14	9.58
1 in 100 year, Q_{100yr}	125.03	13.29

Full results output from the tool can be found in Appendix A.

2.3 Conclusion

To be conservative the minimum greenfield runoff rate will be taken as the Q_{bar} value of 4.16l/s/Ha (total greenfield runoff of 39.19l/s) for storms up but excluding the 1 in 30 year storm return period. However, this value would be overly conservative for storm events above and including a 1 in 30 year event. This is because the runoff rate increases as the size of the storm return period increases. Thus, 9.58l/s/Ha and 13.29l/s/Ha will be used for 1 in 30 year and 1 in 100 year storms respectively when calculating the estimated proposed onsite storage requirements. This is in line with the rate agreed for the Tesco site.

3 Attenuation Storage

3.1 Methodology

The source control tool in MicroDrainage was used to calculate the estimated initial onsite attenuation quantities required. Using the greenfield runoff rates calculated in section 2, the storage requirement for a 1 in 100 year storm and a 1 in 30 year storm both plus climate change allowances. Due to the size of the site, two thirds of the developable area have been assumed to have a time of entry of 0-4mins and one third of the developable area has been assumed to have a time of entry between 4-8mins.

As the proposed site will consist of office developments with no basements, the site will be less vulnerable with regards to flood risk according to the Environmental Agency's guidance on climate change allowance. Therefore a 10% climate change allowance will be used for a 1 in 30 year storm and a 20% climate change allowance will be used for a 1 in 100 year storm.

MicroDrainage Source Control input values and assumption have been tabulated below.

Table 3 Tabulated Input Values for Source Control

Input values	1 in 30 year return period	1 in 100 year return period
Assumed time of entry	0-4mins for 6.273 Ha 4-8mins for 3.137 Ha	0-4mins for 6.273 Ha 4-8mins for 3.137 Ha
Climate Change	10%	20%
Discharge rate	90.1 l/s	125.0 l/s

3.2 Results

Figure 3—1: 1 in 30 year return period + climate change results

Storm Event	Rain (mm/hr)	Time to Vol Peak (mins)	Max Water Level (m)	Max Depth (m)	Flooded Volume (m ³)	Max Control (l/s)	Discharge Volume (m ³)	Max Filtration (l/s)	Σ Max Outflow (l/s)	Maximum Volume (m ³)	Status
15 min Summer	83.638	22	0.782	0.782	0.0	89.8	1898.6	0.0	89.8	1893.5	O K
30 min Summer	54.449	36	1.008	1.008	0.0	89.8	2485.6	0.0	89.8	2440.3	O K
60 min Summer	33.892	64	1.227	1.227	0.0	89.8	3153.8	0.0	89.8	2969.5	O K
120 min Summer	20.476	124	1.409	1.409	0.0	89.8	3815.9	0.0	89.8	3410.5	Flood Risk
180 min Summer	15.087	182	1.481	1.481	0.0	89.8	4219.7	0.0	89.8	3583.9	Flood Risk
240 min Summer	12.094	240	1.505	1.505	0.0	90.0	4511.6	0.0	90.0	3642.1	Flood Risk
360 min Summer	8.838	316	1.501	1.501	0.0	89.9	4946.9	0.0	89.9	3632.9	Flood Risk
480 min Summer	7.071	378	1.485	1.485	0.0	89.8	5277.9	0.0	89.8	3594.0	Flood Risk
600 min Summer	5.944	440	1.462	1.462	0.0	89.8	5546.1	0.0	89.8	3537.9	Flood Risk
720 min Summer	5.156	508	1.435	1.435	0.0	89.8	5772.9	0.0	89.8	3471.6	Flood Risk
960 min Summer	4.117	648	1.372	1.372	0.0	89.8	6145.1	0.0	89.8	3319.4	Flood Risk
1440 min Summer	2.995	924	1.235	1.235	0.0	89.8	6699.9	0.0	89.8	2989.1	O K
2160 min Summer	2.177	1320	1.009	1.009	0.0	89.8	7350.9	0.0	89.8	2442.6	O K
2880 min Summer	1.734	1676	0.814	0.814	0.0	89.8	7805.7	0.0	89.8	1968.8	O K
4320 min Summer	1.258	2376	0.542	0.542	0.0	89.7	8473.1	0.0	89.7	1310.9	O K
5760 min Summer	1.001	3008	0.396	0.396	0.0	87.2	9027.8	0.0	87.2	958.0	O K
7200 min Summer	0.838	3680	0.336	0.336	0.0	80.4	9445.1	0.0	80.4	813.2	O K
8640 min Summer	0.725	4408	0.304	0.304	0.0	71.3	9794.1	0.0	71.3	735.9	O K
10080 min Summer	0.641	5144	0.281	0.281	0.0	63.8	10085.6	0.0	63.8	679.8	O K
15 min Winter	83.638	21	0.783	0.783	0.0	89.8	1898.6	0.0	89.8	1893.9	O K
30 min Winter	54.449	35	1.008	1.008	0.0	89.8	2485.6	0.0	89.8	2440.0	O K
60 min Winter	33.892	64	1.227	1.227	0.0	89.8	3153.8	0.0	89.8	2968.7	O K
120 min Winter	20.476	120	1.410	1.410	0.0	89.8	3815.9	0.0	89.8	3411.4	Flood Risk
180 min Winter	15.087	178	1.483	1.483	0.0	89.8	4219.8	0.0	89.8	3587.7	Flood Risk
240 min Winter	12.094	234	1.510	1.510	0.0	90.1	4511.7	0.0	90.1	3650.9	Flood Risk
360 min Winter	8.838	340	1.504	1.504	0.0	90.0	4947.0	0.0	90.0	3638.7	Flood Risk
480 min Winter	7.071	386	1.473	1.473	0.0	89.8	5278.0	0.0	89.8	3565.3	Flood Risk
600 min Winter	5.944	462	1.441	1.441	0.0	89.8	5546.3	0.0	89.8	3486.7	Flood Risk
720 min Winter	5.156	540	1.401	1.401	0.0	89.8	5773.1	0.0	89.8	3389.3	Flood Risk
960 min Winter	4.117	694	1.307	1.307	0.0	89.8	6145.6	0.0	89.8	3162.8	Flood Risk
1440 min Winter	2.995	992	1.084	1.084	0.0	89.8	6702.2	0.0	89.8	2623.7	O K
2160 min Winter	2.177	1360	0.759	0.759	0.0	89.8	7351.1	0.0	89.8	1835.7	O K
2880 min Winter	1.734	1680	0.529	0.529	0.0	89.6	7806.0	0.0	89.6	1279.5	O K
4320 min Winter	1.258	2288	0.335	0.335	0.0	80.1	8474.9	0.0	80.1	811.8	O K
5760 min Winter	1.001	2992	0.284	0.284	0.0	64.9	9028.0	0.0	64.9	688.1	O K
7200 min Winter	0.838	3680	0.254	0.254	0.0	54.8	9445.6	0.0	54.8	613.5	O K
8640 min Winter	0.725	4416	0.232	0.232	0.0	47.4	9795.2	0.0	47.4	561.2	O K
10080 min Winter	0.641	5144	0.216	0.216	0.0	42.2	10089.2	0.0	42.2	521.6	O K

Figure 3—2: 1 in 100 year return period + climate change results

Storm Event	Rain (mm/hr)	Time to Vol Peak (mins)	Max Water Level (m)	Max Depth (m)	Flooded Volume (m³)	Max Control (l/s)	Discharge Volume (m³)	Max Filtration (l/s)	Σ Max Outflow (l/s)	Maximum Volume (m³)	Status
15 min Summer	118.417	22	0.776	0.776	0.0	123.8	2656.9	0.0	123.8	2686.0	O K
30 min Summer	77.747	36	1.010	1.010	0.0	123.8	3515.2	0.0	123.8	3492.9	O K
60 min Summer	48.611	64	1.236	1.236	0.0	123.8	4507.0	0.0	123.8	4276.7	O K
120 min Summer	29.354	124	1.424	1.424	0.0	123.8	5452.6	0.0	123.8	4926.5	Flood Risk
180 min Summer	21.556	182	1.496	1.496	0.0	123.8	6010.5	0.0	123.8	5174.6	Flood Risk
240 min Summer	17.210	240	1.522	1.522	0.0	124.8	6400.6	0.0	124.8	5250.7	Flood Risk
360 min Summer	12.501	320	1.511	1.511	0.0	124.4	6976.7	0.0	124.4	5224.0	Flood Risk
480 min Summer	9.962	382	1.492	1.492	0.0	123.8	7413.8	0.0	123.8	5161.1	Flood Risk
600 min Summer	8.347	444	1.467	1.467	0.0	123.8	7765.1	0.0	123.8	5076.2	Flood Risk
720 min Summer	7.221	510	1.439	1.439	0.0	123.8	8060.2	0.0	123.8	4978.1	Flood Risk
960 min Summer	5.740	648	1.374	1.374	0.0	123.8	8540.0	0.0	123.8	4755.5	Flood Risk
1440 min Summer	4.148	924	1.234	1.234	0.0	123.8	9244.7	0.0	123.8	4271.2	O K
2160 min Summer	2.992	1316	1.006	1.006	0.0	123.8	10092.0	0.0	123.8	3481.3	O K
2880 min Summer	2.371	1672	0.815	0.815	0.0	123.8	10655.6	0.0	123.8	2820.1	O K
4320 min Summer	1.705	2340	0.555	0.555	0.0	123.2	11462.3	0.0	123.2	1920.8	O K
5760 min Summer	1.348	3000	0.420	0.420	0.0	119.4	12154.8	0.0	119.4	1452.5	O K
7200 min Summer	1.123	3680	0.365	0.365	0.0	106.3	12650.9	0.0	106.3	1264.5	O K
8640 min Summer	0.967	4408	0.332	0.332	0.0	94.0	13059.2	0.0	94.0	1149.4	O K
10080 min Summer	0.852	5144	0.308	0.308	0.0	84.0	13390.4	0.0	84.0	1064.3	O K
15 min Winter	118.417	21	0.776	0.776	0.0	123.8	2656.9	0.0	123.8	2686.6	O K
30 min Winter	77.747	35	1.010	1.010	0.0	123.8	3515.2	0.0	123.8	3493.0	O K
60 min Winter	48.611	64	1.236	1.236	0.0	123.8	4507.1	0.0	123.8	4275.5	O K
120 min Winter	29.354	120	1.424	1.424	0.0	123.8	5452.7	0.0	123.8	4925.6	Flood Risk
180 min Winter	21.556	178	1.496	1.496	0.0	123.8	6010.6	0.0	123.8	5177.0	Flood Risk
240 min Winter	17.210	234	1.526	1.526	0.0	125.0	6400.7	0.0	125.0	5258.9	Flood Risk
360 min Winter	12.501	342	1.514	1.514	0.0	124.5	6976.9	0.0	124.5	5231.3	Flood Risk
480 min Winter	9.962	390	1.479	1.479	0.0	123.8	7414.1	0.0	123.8	5116.2	Flood Risk
600 min Winter	8.347	464	1.445	1.445	0.0	123.8	7765.5	0.0	123.8	5000.8	Flood Risk
720 min Winter	7.221	542	1.405	1.405	0.0	123.8	8060.7	0.0	123.8	4860.1	Flood Risk
960 min Winter	5.740	696	1.311	1.311	0.0	123.8	8541.0	0.0	123.8	4535.0	Flood Risk
1440 min Winter	4.148	984	1.087	1.087	0.0	123.8	9249.1	0.0	123.8	3759.6	O K
2160 min Winter	2.992	1360	0.771	0.771	0.0	123.8	10092.5	0.0	123.8	2668.3	O K
2880 min Winter	2.371	1676	0.551	0.551	0.0	123.2	10656.2	0.0	123.2	1905.8	O K
4320 min Winter	1.705	2292	0.370	0.370	0.0	107.8	11465.3	0.0	107.8	1280.1	O K
5760 min Winter	1.348	3000	0.315	0.315	0.0	87.0	12155.2	0.0	87.0	1090.5	O K
7200 min Winter	1.123	3744	0.281	0.281	0.0	73.0	12651.7	0.0	73.0	972.0	O K
8640 min Winter	0.967	4416	0.257	0.257	0.0	63.2	13061.1	0.0	63.2	887.8	O K
10080 min Winter	0.852	5152	0.238	0.238	0.0	55.8	13396.6	0.0	55.8	824.5	O K

3.3 Conclusion

The results from Section 3.2 are summarised the Table 4 which shows the volume of storage required to attenuate flows for both storm events:

Table 4 Masterplan Attenuation Requirements

Storm Criteria	Volume of Attenuation Required (m³)
1 in 30 year + climate change	3650
1 in 100 year + climate change	5260

The next section discusses the SuDS features that will be employed to meet the attenuation requirements outlined above.

4 Proposed SuDS

4.1 SuDS Hierarchy

As discussed in the Bicester Office Park drainage strategy report (July 17), SuDS will be used providing benefits to the water quality and surface water runoff rates together with the added amenity value to the urban environment.

Effective SuDS techniques manage surface water run-off as close to the source as possible in line with the following drainage hierarchy:

- 1 store rainwater for later use –**rainfall harvesting**
- 2 use infiltration techniques, such as porous surfaces in non-clay areas –**unlikely to be suitable due to geology**
- 3 attenuate rainwater in ponds or landscape bio retention areas/swales for gradual release –**shown on illustrative masterplan**
- 4 attenuate rainwater by storing in tanks or sealed water features for gradual release–**permeable paving shown on illustrative masterplan**
- 5 discharge rainwater direct to a watercourse –**not applicable**
- 6 discharge rainwater to a surface water sewer/drain –**not applicable**
- 7 discharge rainwater to the combined sewer. –**not applicable**

In line with the principles of this hierarchy the proposed masterplan will use ponds, permeable paving and rainwater harvesting to manage surface water runoff from the site. The location of these measures are shown on the illustrative masterplan in Appendix B.

4.2 Landscape Bio retention Areas/Swales/Ponds

The illustrative masterplan has a significant area of soft landscaping that can be utilised for surface water attenuation. The attenuation can be provided by:

- a) Bio retention system
- b) Swales
- c) Ponds

Or a combination of the above.

In order to calculate the volume of storage that can be provided it has been assumed that the maximum depth of stored water for a 1 in 100 year storm is 0.5m (ref *SuDS Manual Section 23.2*). Using this assumption the volume of attenuation available is:

- Total plan area available within landscape area = 10,100m²
- **Potential Volume of Attenuation within landscape area = 10100 x 0.5 = 5050m³**

4.3 Permeable Paving

Permeable surfaces are underlain with a pavement construction that allows rainwater to percolate through the surface, be treated and stored in the granular sub-base, and discharge either to the ground or to an outlet into a surface water network. For permeable paving to be most viable and effective, the area should be near flat.

Permeable paving is proposed for the car parks and will both attenuate surface water runoff and improve water quality to mitigate against the likelihood of contaminants entering the nearby watercourse. These will discharge to the surface water drainage network as infiltration is not suitable to the site conditions. It should be noted that a large buffer zone has been left adjacent to the limit of the 1 in 100 year flood level extent on the illustrative masterplan. At detailed design stage the area of permeable paving could extend closer to the edge of the flood zone so increasing the available storage.

The volume of attenuation that can be provided by permeable pavements has been estimated based on the following assumptions:

- Total plan area available for permeable paving = 30,800m²
- Depth of sub-base = 0.25m (ref *Marshall's Permeable Pavement Guidance document*)
- Void ratio of sub base = 0.3 (ref *Marshall's Permeable Pavement Guidance document*)
- **Potential Volume of Attenuation From Permeable Paving = 30800 x 0.25 x 0.3 = 2310m³**

4.4 Rainwater Harvesting

Rainwater harvesting storage tanks collect rainwater from rooftops and other hard surfaces before processing it ready for use. Rainwater storage tanks can either be buried under ground and out of site or store above ground in a discrete area at the side of a building. Water collected in the storage tanks is pumped to an elevated cistern and fed by gravity to the points of use.

Rainwater harvesting may be suitable for the proposed development and should be considered at the next stage of the design. The potential benefits associated with the attenuation of flows have not been estimated at this stage.

4.5 Available Volume of Attenuation for SuDS Techniques

The above calculations show that potential volume of attenuation from permeable paving and ponds is greater than the 5,260m³ of attenuation required for a 1 in 100 year storm event resulting in a surplus of 2,100m³.

Therefore the storage can be provided in accordance with the SuDS Manual good practice recommendations without the need for large below ground storage tanks.

5 Overall Summary and Conclusion

5.1 In response to OCC comments as follows

A supporting calculation needs to be provided and for initial sizing calculations in support of outline application the toolkit provided by the 'UK Suds' website is acceptable to OCC.

The greenfield runoff rate calculated using uksuds is 4.16l/s/Ha (39.2 l/s) for storms less than a 1 in 30 year return period, for 1 in 30 year storm it is 9.58l/s/Ha (90.1 l/s), and for 1 in 100 year storm it is 13.29l/s/Ha (125.0 l/s).

OCC considered that the drainage proposals were not adequately described within the strategy document. For an outline application, the proposal needs to describe the attenuation storage volumes that are required to provide mitigation and achieve compliance with the proposed allowable discharge rates.

Various storage scenarios were tested and the results show the required storage could be accommodated utilising the either above ground storage or more likely a combination of above ground storage, permeable parking areas with storage contained within shallow storm cells located immediately below the pavement construction.

If all storage for a 1 in 100 year event is provided above ground approximately 50% of the soft landscaped area proposed would be flooded to a depth of 500 mm

If the storage for storms of up to 1 in 30 year return is provided below the hard landscaping areas and the additional storage for storms of 1 in 100 year + climate change is provided above ground the 50% of the hard landscaping area will need storm cells 135mm deep and 50% of the soft landscaping area would be flooded to a depth of 150mm

Typically the applicant must show by way of a sketch, which describes the SuDS features and demonstrates that they fit within the red line application boundary.

It is likely that the final design of the attenuation will not be either of the two scenarios above but the calculations show that the storage can be easily accommodated within the development areas contained within the parameters plan. The estimated volume of attenuation that can be provided within the masterplan with the use the soft landscaping (e.g. bioretention areas/swales) and permeable paving is approximately 7300m³, significantly greater than the 5300m³ required to attenuate flows to Greenfield runoff rates. It is therefore not necessary to provide large, below ground, formal storage tanks in order to attenuate the flows.

Refer to Appendix B for a sketch which outlines the proposed SuDS techniques that will employed on the scheme and the anticipated volume of attenuation that each is able to provide.

Appendix A

Greenfield Runoff Rate UKSUDS Calculation

Calculated by: John Scandrett
 Site name: Bicester Business Park
 Site location: Bicester

Site coordinates
 Latitude: 51.88917° N
 Longitude: 1.15958° W

This is an estimation of the greenfield runoff rate limits that are needed to meet normal best practice criteria in line with Environment Agency guidance "Preliminary rainfall runoff management for developments", W5-074/A/TR1/1 rev. E (2012) and the SuDS Manual, C753 (Ciria, 2015). This information on greenfield runoff rates may be the basis for setting consents for the drainage of surface water runoff from sites.

Reference: 6322583
 Date: 2018-03-12T15:56:30

Methodology	IH124
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Site characteristics

Total site area (ha)	9.41
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Methodology

Qbar estimation method	Calculate from SPR and SAAR
SPR estimation method	Calculate from SOIL type

	Default	Edited
SOIL type	1	4
HOST class	---	---
SPR/SPRHOST	0.1	0.47

Hydrological characteristics

	Default	Edited
SAAR (mm)	617	617
Hydrological region	6	6
Growth curve factor: 1 year	0.85	0.85
Growth curve factor: 30 year	2.3	2.3
Growth curve factor: 100 year	3.19	3.19

Notes:

- (1) Is $Q_{BAR} < 2.0$ l/s/ha?
 Normally limiting discharge rates which are less than 2.0 l/s/ha are set at 2.0 l/s/ha.
- (2) Are flow rates < 5.0 l/s?
 Where flow rates are less than 5.0 l/s consents are usually set at 5.0 l/s if blockage from vegetation and other materials is possible. Lower consent flow rates may be set in which case blockage work must be addressed by using appropriate drainage elements
- (3) Is $SPR/SPRHOST \leq 0.3$?
 Where groundwater levels are low enough the use of soakaways to avoid discharge offsite may be a requirement for disposal of surface water runoff.







Greenfield runoff rates

	Default	Edited
Qbar (l/s)	1.36	39.19
1 in 1 year (l/s)	1.16	33.31
1 in 30 years (l/s)	3.14	90.14
1 in 100 years (l/s)	4.35	125.03

Appendix B

Location of attenuation storage within illustrative masterplan

LEGEND

-  SITE BOUNDARY
-  EXISTING PUBLIC FOUL SEWER
-  PROPOSED FOUL SEWER
-  SURFACE WATER SEWER
-  LANDSCAPED AREA USED FOR STORAGE
-  PERMEABLE PAVING

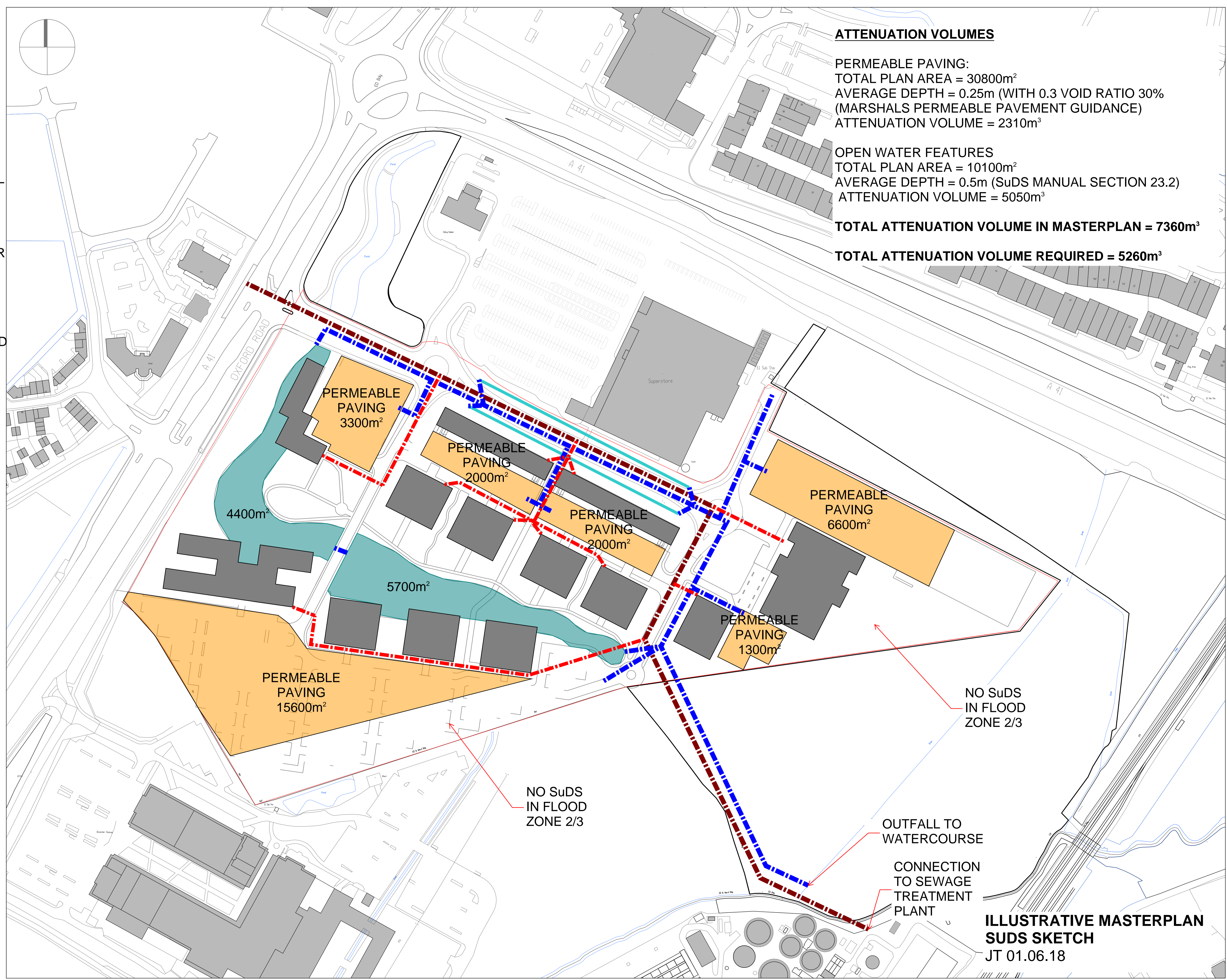
ATTENUATION VOLUMES

PERMEABLE PAVING:
TOTAL PLAN AREA = 30800m²
AVERAGE DEPTH = 0.25m (WITH 0.3 VOID RATIO 30% (MARSHALS PERMEABLE PAVEMENT GUIDANCE))
ATTENUATION VOLUME = 2310m³

OPEN WATER FEATURES
TOTAL PLAN AREA = 10100m²
AVERAGE DEPTH = 0.5m (SuDS MANUAL SECTION 23.2)
ATTENUATION VOLUME = 5050m³

TOTAL ATTENUATION VOLUME IN MASTERPLAN = 7360m³

TOTAL ATTENUATION VOLUME REQUIRED = 5260m³



**ILLUSTRATIVE MASTERPLAN
SuDS SKETCH**
JT 01.06.18

Appendix D: Bicester Office Park Energy Strategy

Technical Note

Project Bicester Office Park – Planning Support

Subject Energy Strategy

Project no 0040031

Date 18 June 2018

Revision	Description	Issued by	Date	Approved (signature)
00	Bicester Office Park Energy Strategy	OT	18.06.2018	JW

1 Introduction

This energy statement has been developed to support the outline planning application for the Bicester Office Park development in Oxfordshire. The site (13.1Ha of floor area) is predominately made up of B1(a) / B1(b) class buildings, split into six zones (see Figure 1—1).

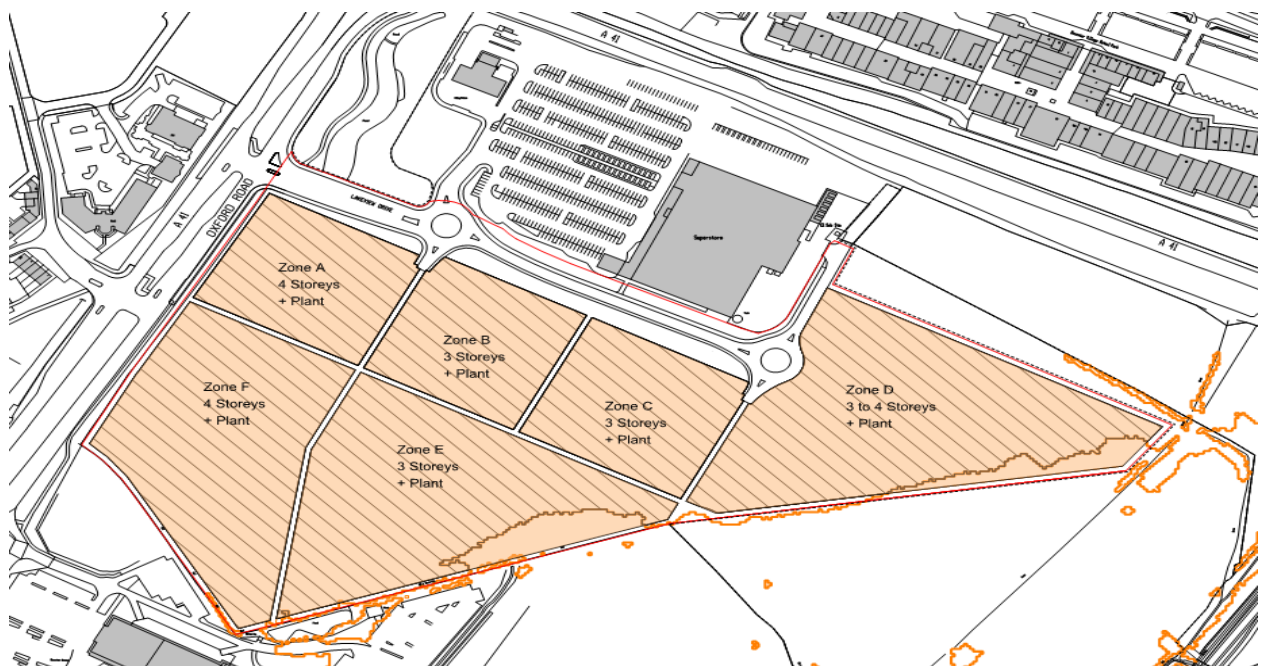


Figure 1—1 Bicester Office Park plan

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This is an outline planning application with an illustrative masterplan and therefore this document only discusses how the local planning policies could be complied with. The reserved matters applications will address the policy requirements in detail for the specific building designs for the scheme.

This document comprises of the following sections:

Section 2: This office park is under the Cherwell District Council planning authority. This section covers relevant local policies for this development.

Section 3: This section discusses energy strategies for the site.

2 Planning policy – Cherwell Local Plan 2011 – 2031 (July 2015)

Key planning policies relevant to the Bicester Office Park development have been discussed in this section. The Cherwell Local Plan requires the following energy and sustainability requirements to be met.

Policy ESD 2: Energy hierarchy and allowable solutions

In seeking to achieve carbon emissions reductions, we will promote and 'energy hierarchy' as follows:

- Reducing energy use, in particular by the use of sustainable design and construction measures
- Supplying energy efficiently and giving priority to decentralised energy supply
- Making use of renewable energy
- Making use of allowable solutions.

Policy ESD 3: Sustainable construction

All new non-residential development will be expected to meet at least BREEAM 'Very Good' with immediate effect, subject to review over the plan period to ensure the target remains relevant. The demonstration of the achievement of this standard should be set out in the Energy Statement.

All development proposals will be encouraged to reflect high quality design and high environmental standards, demonstrating sustainable construction methods including but not limited to:

- Minimising both energy demands and energy loss
- Maximising passive solar lighting and natural ventilation
- Maximising resource efficiency
- Incorporating the use of recycled and energy efficient materials
- Incorporating the use of locally sourced building materials
- Reducing waste and pollution and making adequate provision for the recycling of waste
- Making use of sustainable drainage methods
- Reducing the impact on the external environment and maximising opportunities for cooling and shading (by the provision of open space and water, planting, and green roofs, for example); and
- Making use of the embodied energy within buildings wherever possible and re-using materials where proposals involve demolition or redevelopment.

Policy ESD 4: Decentralised energy systems

The use of decentralised energy systems, providing either heating (District Heating (DH)) or heating and power (Combined Heat and Power (CHP)) will be encouraged in all new developments.

A feasibility assessment for DH/CHP, including consideration of biomass fuelled CHP, will be required for all applications for non-domestic developments above 1,000m² floor space.

Where feasibility assessments demonstrate that decentralised energy systems are deliverable and viable, such systems will be required as part of the development unless an alternative solution would deliver the same or increased benefit.

Policy ESD 5: Renewable energy

Planning applications involving renewable energy development will be encouraged provided that there is no unacceptable adverse impact, including cumulative impact, on the following issues, which are considered to be of particular local significance in Cherwell:

- Landscape and biodiversity including designations, protected habitats and species, and Conservation Target Areas
- Visual impacts on local landscapes
- The historic environment including designated and non-designated assets and their settings
- The Green Belt, particularly visual impacts on openness
- Aviation activities
- Highways and access issues, and
- Residential amenity.

A feasibility assessment of the potential for significant on site renewable energy provision (above any provision required to meet national building standards) will be required for all applications for non-domestic developments above 1000m² floor space.

Where feasibility assessments demonstrate that on site renewable energy provision is deliverable and viable, this will be required as part of the development unless an alternative solution would deliver the same or increased benefit. This may include consideration of 'allowable solutions' as Government Policy evolves.¹

3 Energy strategy

This section provides a summary of proposed energy strategies for the Bicester Office Park development. These strategies explain how local planning policy can be complied with and aim to minimise the carbon dioxide emissions of this scheme. The Bicester Office Park development will be assessed under the Cherwell Local Plan, hence requiring the development to achieve BREEAM 'Very Good' as a minimum. However, a higher BREEAM rating for the development will be targeted.

In accordance with policy ESD2, the proposed energy efficiency of the development has been achieved by following energy hierarchy mentioned in Section 2 above. The approach aims to minimise energy consumption from the outset through the use of low energy passive measures and efficient systems before the deployment of low and zero-carbon technologies (Figure 3—1).

¹ <https://www.cherwell.gov.uk/downloads/download/45/adopted-cherwell-local-plan-2011-2031-part-1-incorporating-policy-bicester-13-re-adopted-on-19-december-2016>

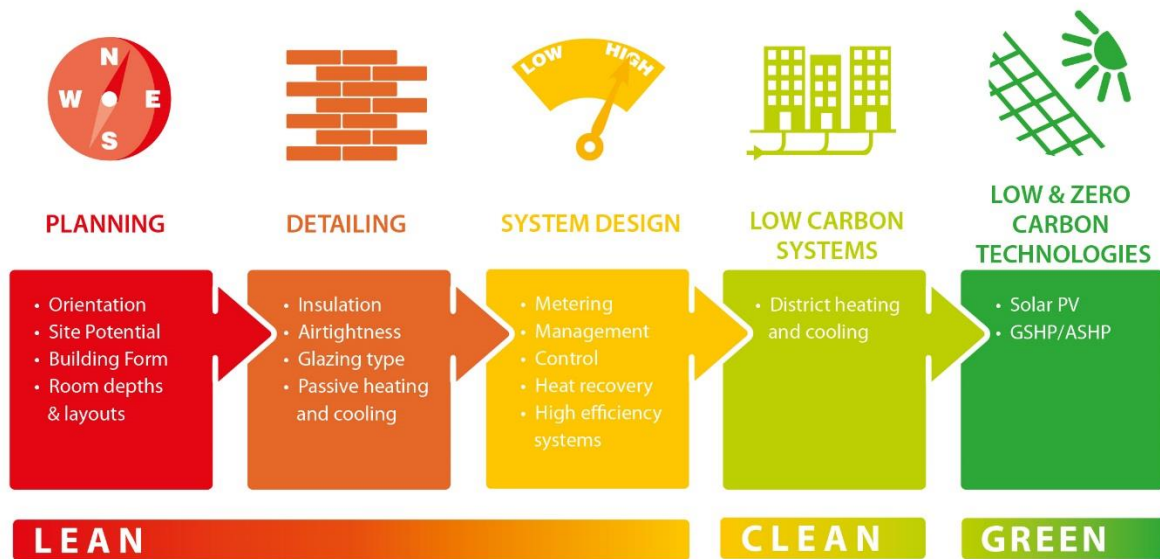


Figure 3—1 Energy hierarchy

Decentralised systems for cooling and heating is considered a ‘clean’ solution as specified in ESD2, however there are no known networks in the area which the Bicester Office Park development can be connected to at the point of issue of this document. The loads of this site are also too low to be economically viable to create a new heating or cooling network (see section 3.2 for more information).

3.1 Energy efficiency

As this document is for an outline planning application, there are currently no building designs and therefore the proposed energy efficiency measures cannot be described in detail. The buildings will exceed the energy efficiency requirements of Part L of the Building Regulations and look to include the following energy efficiency measure, which will be explored in more detail during detailed design and described in detail as part of the reserved matters application:

- All buildings on this development will target a BREEAM ‘Very Good’ rating. Efficient and airtight building fabric with glazing ratios appropriate to balance heating demands, cooling demands and energy saving from daylight. Modern office type buildings often have high internal heat gains and are self-heating for much of the year without the need for additional heat sources. Cooling requirements are likely to be an important consideration and the buildings will use solar control glass and shading to limit unwanted solar gain.
- An office building is likely to have a large amount of domestic hot water (DHW) secondary circulation pipework for a relatively small demand, which can lead to large distribution losses. System design and pipework insulation will aim to minimise this.
- It is likely that the nature of the buildings on-site will mean that mechanical ventilation is required but during detailed design opportunities for natural ventilation and mixed-mode operation will be explored. Wherever possible air supply will be controlled based upon CO₂ emissions to minimise fan energy and incorporate high efficiency heat recover to minimise heat demands.
- Any cooling requirement will be met by high efficiency systems.
- A high efficiency LED lighting scheme will be installed throughout with automatic daylight dimming and occupancy control where appropriate.

- Sub-metering and automatic monitoring of energy consumption will be included, with alerts sent to the building manager if out of typical range performance is recorded.

3.2 Decentralised energy

There are no known surrounding district heating or cooling which this development can connect to. District heating networks (DHNs) require a certain density of heat to be financially viable, for any given development site. Due to low levels of saleable heat compared to high capital costs for pipework, DHNs with low heat densities may not achieve this financial viability. As a rule of thumb, 26kWh/m²/yr is taken as the minimum area heat density as the threshold for district heating viability. The heat density for the Bicester Office Park development has been calculated as 19kWh/m²/yr, hence this site is not considered viable for a heat network. Heat and cooling will be delivered at building level.

3.3 On-site renewables

The potential of deploying various renewable energy supply technologies on the Bicester Office Park site has been assessed in Table 3—1.

Table 3—1 Energy supply options assessment

Technology	Capital / operational costs	Revenue Potential	Maturity of technology	CO ₂ abatement potential	Opportunity appraisal	Potential
<p>Biomass Typically deployed on DH networks as a solid fuel biomass boiler, using either pellets or wood chip as a fuel. Typically provide ~80% of heat consumption, with gas boilers usually providing top-up heat.</p>	Medium / Medium	Heat Sales and RHI	High	High	The sustainability of biomass as fuel source is uncertain. The spatial requirements to deploy this technology successfully also makes this technology impractical due to its requirement for fuel storage and delivery. Air quality impacts are a further risk. Therefore, biomass boilers are not considered appropriate for the scheme.	low
<p>Biofuel combined heat and power (CHP) engine Traditionally deployed with DH due to the potential for high revenues from electrical generation.</p>	Medium / High	Heat sales / electricity sales / capacity market income	Medium / Low	Medium – expected to drop as the power grid decarbonises	Due to the low heat demand of the development, Biofuel CHPs are not considered to be a viable energy supply option. Biofuel CHPs are expensive and unreliable at small scale owing to the high temperatures and pressures required.	low
<p>Anaerobic Digester (AD) Production of biogas which can be injected into the gas grid or used on-site, typically with CHP engines</p>	High / High	Heat Sales and RHI / FIT / electricity sales	Medium	High	ADs are not considered to be a viable option for this site. Amenity challenges are expected when delivering feedstock to this Office park. This technology is also not considered appropriate for a business development.	low
<p>Ground Source Heat Pump (GSHP)</p>	High / Medium	Heat Sales and RHI	Medium	Medium – improving with decarbonisation of grid	<p>GSHP is considered to be a viable option for this scheme. It can be used to provide both heating and cooling to the development.</p> <p>GSHP systems can either be closed loop (multiple borehole heat exchangers placed in the ground) or open loop (water is abstracted from a shallow below ground aquifer and passed through an above ground heat exchanger). Further analysis will be required to determine the most feasible solution for this site.</p>	high

Technology	Capital / operational costs	Revenue Potential	Maturity of technology	CO ₂ abatement potential	Opportunity appraisal	Potential
					Although the electricity demand of the site might increase due to the high demand required by this technology, carbon savings can be achieved. This will further improve with the decarbonisation of the grid.	
Air Source Heat Pump (ASHP) Low precedents for DH	High / High	Heat Sales and RHI	High	Medium – improving with decarbonisation of grid	ASHPs are suitable for this development as heating will be provided at building level. They have a lower efficiency than GSHPs. Although the electricity demand of the site might increase due to the high demand required by this technology, carbon savings can be achieved. This will further improve with the decarbonisation of the grid.	high
Water Source Heat Pump (WSHP) Low precedents for DH – low temperature output may be suited to new buildings.	High / Medium-High	Heat Sales and RHI	Medium	Medium – improving with decarbonisation of grid	Not viable for this site due to lack of nearby water source.	low
Sewage Heat Recovery Low precedents for DH – low temperature output may be suited to new buildings	High / Medium	Heat Sales and RHI	Low	Medium – improving with decarbonisation of grid	Sewage heat recovery could be feasible in this site, however the capacity of sewer pipes around this site will need to be investigated.	medium
Hydrogen Fuel Cells Low precedents for DH	High / Medium	Heat Sales	Low	Medium – improving with decarbonisation of grid	Hydrogen is not currently available as a fuel source hence fuel cells are not considered a viable option for this development.	low
Solar PV Panels	Medium / Low	Heat Sales and RHI	High	High	Solar PV panels convert direct and diffuse radiation from the sun into electrical energy. These can be installed on the roof of buildings on this development. It has been assumed at this early stage design that 30% of the roofs can be solar panels. Past project experience shows that this figure is realistic to allow for access, roof lights, plant and other roof top penetrations.	high

Technology	Capital / operational costs	Revenue Potential	Maturity of technology	CO ₂ abatement potential	Opportunity appraisal	Potential
Wind Turbines	Medium / Low	Heat Sales and RHI	High	High	The Cherwell planning guidance on the residential amenity impacts of wind turbine development states a minimum distance of 400m must be achieved between residential developments and wind turbines. This policy cannot be complied with, if turbines are to be installed for Bicester Office Park as there are residential buildings adjacent to the site. Hence wind energy generation is not viable for this development. ²	low

Based on the energy supply assessment above, solar PV panels and heat pumps (ASHP and GSHP) are considered as the most suitable and economically viable renewable energy options for the Bicester Office Park development.

An initial appraisal of PV generation capacity suggests that if 30% of the overall roof area is available for solar PV installation (as the remaining 70% is needed for other equipment such as roof chillers) up to 6% (see section 3.5 for calculation summary) of the energy demand for the site (electricity, heating and cooling) could be met by PV. Solar panels can be combined with green roofs.

If a heat pump with a seasonal coefficient of performance of 3.5 produced 60% of the annual heat on the site, the renewable heat would generated would represent approximately 25% of the overall annual energy demand (only includes regulated electricity) which has been estimated for the site. More detailed modelling may be able to demonstrate an improved performance.

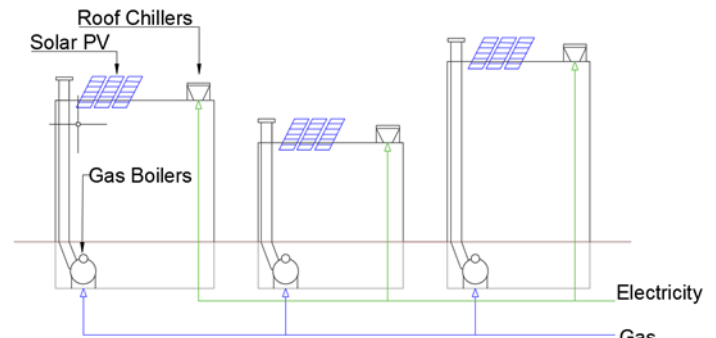
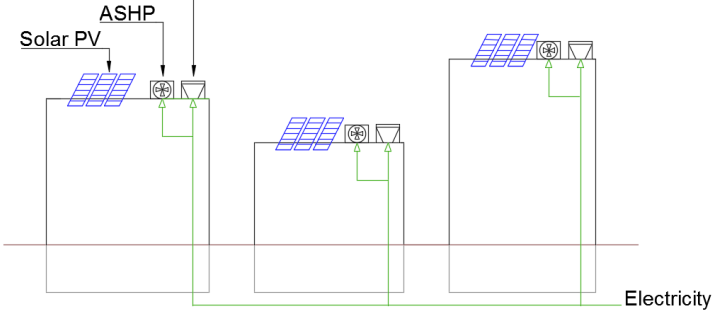
At the next stage, it is recommended that Bicester Office Park developers explores which options deliver the best value for money considering whole life costs and renewable contributions.

² <http://modgov.cherwell.gov.uk/documents/s8812/Executive%20Report%20-%20Key%20Decision%20for%20Planning%20Policy%20for%20Wind%20Energy%20Development%20enc.%201.pdf?txtonly=1>

3.4 Propose Energy Options

Two energy options for the Bicester Office Development have been qualitatively assessed. The options are summarised below in Table 3—2 and described in more detail in the following sub-sections.

Table 3—2 Energy strategy options

Option	Diagram	Description
1: Standalone solutions for each building –		Gas boilers in each building for heating and roof chillers for cooling.
2: Standalone solution for each building- All Electric		ASHPs (or GSHPs) in each building for heating and roof chillers for cooling.

Option 1 – standalone solutions for each building

This option includes gas boilers in each building for heating and roof chillers for cooling. This option is expected to be compliant with policy i.e. achieving BREEAM Very Good targets. The BREEAM energy credits look at CO₂ reduction, demand reduction and primary energy conversion, and detailed energy modelling based upon actual building design will be required to establish the number of energy credits achieved.

The strategy can be summarised as follows:

Electricity:

- Electricity connection to the power grid; and
- Solar PV panels installed on roofs.

Heating:

- Centralised gas boiler plant installed in each building hot water and heating demands; and
- Gas connection to the gas network to each plant room in each building to supply the boilers;

Cooling:

- Chillers for cooling, this will likely be air-cooled chillers on the roof of each building, powered by electricity

Space Requirements:

- This option requires a plant room in the basement or ground floor of each building including gas boilers, hot water storage, chillers and pumping equipment. Air-cooled chillers and solar panels would also require roof space.

Pros and Cons:

Pros	Cons
<ul style="list-style-type: none"> • Conventional “tried and tested” approach • Flexible with phasing • Can be combine with high efficiency fabric buildings - the lower heat and power demands of such design may favours simple stand-alone solutions rather than centralised/network-based solutions 	<ul style="list-style-type: none"> • Higher overall plant capacity provision due to reduced diversities • Gas infrastructure required to all buildings

Option 2- standalone solutions for each building (all electric option)

This option includes ASHPs (or GSHPs) in each building for heating and roof chillers for cooling. This option is expected to be compliant with policy i.e. achieving BREEAM Very Good targets. The strategy can be summarised as follows:

Electricity:

- Electricity connection to the power grid
- Solar PV panels installed on roofs for on-site power generation.

Heating / cooling:

- Reversible Heat Pump. A low-carbon alternative to a centralised gas boiler plant would involve using ASHPs to meet the buildings demand for space heating and hot water. Carbon factors for grid electricity are expected to progressively decrease as the share of renewables in the energy mix grows. ASHPs can therefore remain a long-term low-carbon option. Another option would be closed-loop ground-source heat pumps. This option would be more costly in the short term but would bring higher efficiencies, due to the ground being a more stable heat source than air, and therefore reduced operational costs in the long term. Reversible ASHP / GSHPs could also be utilised for the simultaneous heating and cooling often required in offices. GSHPs will also require more land space, however with the large carpark spaces available in this development, GSHPs are a viable option.
- Secondary side heating systems sized for lower flow temperatures ~45-50°C. DHW tanks may be required with an additional heating element to bring up the temperatures or chlorine dosing to comply with Legionella requirements.
- Gas Boilers may also be required for peak heat demands

Space Requirements:

- This option requires a plant room in the basement/ground-floor of each building for GSHP option, buffer tanks and back-up boilers. ASHPs and chillers would be located on the roof. If the GSHP system was selected consideration should be made for the underground borehole installations.

Pros and Cons:

Pros	Cons
<ul style="list-style-type: none"> • Flexible with phasing • Ideal energy strategy to combine with high efficiency fabric buildings - the lower heat and power demands of such design would favour simple stand-alone solutions rather than centralised/network-based solutions • Low-carbon: ASHP and GSHP offer a long term low-carbon solutions as grid electricity becomes increasingly decarbonised. 	<ul style="list-style-type: none"> • Higher overall plant room space take / plant capacity • GSHP: a potentially costly option requiring extra construction and installation works • Carbon competitiveness of the scheme dependent on grid decarbonisation • Additional roof space required for ASHPs

3.5 Summary

A summary of the energy strategy performance is shown in Table 3—3 and Figure 3—2. Overall the proposals (including solar PV and GSHPs) achieve a 29% reduction in CO₂ emissions compared to the baseline proposals.

Table 3—3 Energy and carbon demand summary table

	Energy demand* (MWh pa)	Energy saving achieved	Total CO ₂ emissions (tonnes pa)	Carbon saving achieved
Baseline energy demand and emissions (Chillers + Gas boilers)	4,510		1,770	
Energy demand and emissions after energy efficiency improvements (Chillers + Gas boilers)	4,600	-2%	1,500	15%
Energy demand and emissions with solar PV after energy efficiency improvements (Chillers + Gas boilers + Solar PV)	4,600		1,370	23%
Energy demand and emissions with solar PV and heat pump after energy efficiency improvements (Chillers + Gas boilers + Solar PV + heat pump)	4,950	-10%	1,250	29%

*includes only regulated electricity demands as BREEAM credits minimum requirements only consider regulated building energy consumption.

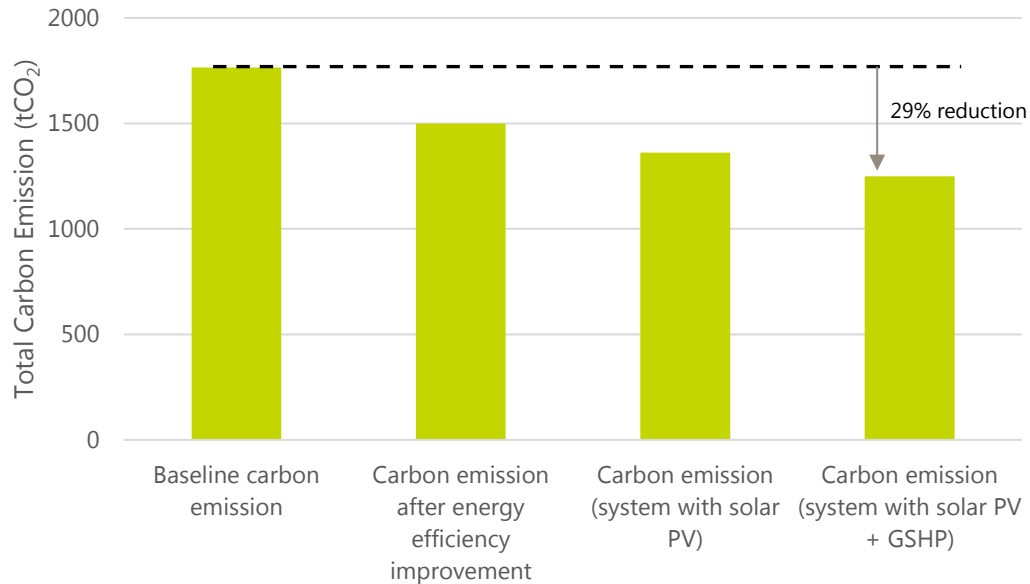


Figure 3—2 Energy strategy summary graph

4 Conclusion

This document outlines possible energy strategies which enable the Bicester Office Park development to meet Cherwell local planning policies. As per the energy hierarchy outlined in policy ESD2 in the Cherwell local plan, the proposed energy strategy includes:

- Energy efficiency building design and specification
- An assessment of the feasibility of hosting an energy centre and district heating network on-site. This was found to be not financially viable for this development.
- Utilisation of low carbon technologies including solar photovoltaic panels and heat pumps.

Overall, these proposals are predicted to achieve a 29% reduction in carbon emissions compared to the baseline development. This proposed strategy is expected to be compliant with the local planning policy which requires a minimum of BREEAM 'Very Good'. However, this energy strategy also forms a basis for achieving a higher BREEAM target (i.e. Excellent or Outstanding) for the Bicester Office Park development. Further analysis will be required to establish the number of energy credits achieved by this scheme, thus confirming its BREEAM rating.

Trium Environmental Consulting LLP
68-85 Tabernacle Street
London
EC2A 4BD
+44 (0) 20 3887 7118
hello@triumenv.co.uk
www.triumenvironmental.co.uk

Project Reference: TEC001

