

Bicester Office Park, Bicester

## Updated Transport Assessment

## Document Control Sheet

Updated Transport Assessment
Bicester Office Park, Bicester
Scenic Land Developments Ltd

This document has been issued and amended as follows:

| Date | I ssue | Prepared by | Approved by |
| :---: | :---: | :---: | :---: |
| $04 / 04 / 2018$ | Updated | KL | DL |
| $05 / 07 / 2018$ | Updated Rev A | KL | DL |
|  |  |  |  |

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### 1.0 I ntroduction

1.1 Motion has been appointed by Scenic Land Developments Ltd to prepare this Transport Assessment in relation to development proposals on land to the east of the A41 Oxford Road, Bicester within the administrative boundary of Cherwell District Council (CDC).
1.2 The site is currently undeveloped and is bound by the A41 Oxford Road to the west and Lakeview Drive to the north whilst Wyevale Garden Centre is located to the immediate south. The Bicester - Oxford railway line operates to the east and is separated from the site by undeveloped land.
1.3 The proposals comprise the redevelopment of the site to form up to 60,000 square metres (GEA) of B1(a)/B1(b) office space along with associated parking and landscaping. Vehicle access to the site would be via the two existing roundabout junctions on Lakeview Drive.

## Site History

1.4 Outline planning permission was granted in 2010 for the construction of a 60,000 -square metre office park comprising 53,000 square metres of $B 1(a) / B 1(b)$ office space and a 7,000-square metre C1 hotel, served by circa 1,837 car parking spaces (Planning Ref: 07/01106/OUT).
1.5 Detailed planning permission was subsequently granted in November 2013 for the construction of a Tesco food store of 8,135 square metres and petrol filling station on part of the permitted office park site (Planning Ref: 12/01193/F). That planning application was supported by a Transport Assessment which considered the effect of the development proposals on the highway network local to the site. The Tesco store has since been constructed and opened in April 2016.
1.6 The S106 Deed of Variation in relation to the permitted Tesco store and office park allows for the construction of up to 45,000 square metres of the $B 1(a) / B 1(b)$ office space being delivered on the remainder of the site, as part of the previous outline planning permission for an office park.

## Current Planning Application

1.7 The current development proposals seek outline planning permission for the construction of an office park providing up to 60,000 square metres of $B 1(a) / B 1(b)$ office space.
1.8 A planning application was submitted to CDC in December 2017 (Planning Ref: 17/02534/OUT) seeking outline planning permission of $\mathrm{B} 1(\mathrm{a}) / \mathrm{B} 1(\mathrm{~b})$ office space. The development would be accessed from Lakeview Drive via two existing roundabout junctions.
1.9 The current development proposals would supersede and replace the previous outline permission for an office park on the site. In comparison with the previous outline planning permission for an office park on the site, the current site area excludes the portion of the site, north of Lakeview Drive, which has since been developed for a Tesco store. However, the site area now includes a parcel of land along the frontage of the A41 Oxford Road, south of Lakeview Drive, which was previously not within the applicant's ownership and was not part of the previous outline planning permission for an office park.
1.10 A formal pre-application submission was made to Oxfordshire County Council (OCC) in April 2017 and a pre-application response was received from OCC in May 2017. A copy of the pre-application response is attached at Appendix $A$.
1.11 The planning application was supported by a Transport Assessment which considered the highways and transport matters associated with the development proposals and a Framework Travel Plan which set out the principles of a Travel Plan to encourage sustainable travel choices amongst future employees at the site. Following submission of the planning application comments have been received from Oxfordshire County Council (OCC) in a letter dated $27^{\text {th }}$ February 2018 and a further response dated $31^{\text {st }}$ May 2018, attached at Appendix B. As such, updated versions of the Transport Assessment and Framework Travel Plan have been prepared to address comments received.

Report Structure
1.12 This Updated Transport Assessment has been prepared in accordance with national and local guidance and considers the highways and transport matters associated with the current development proposals and, in particular, the effect of the development proposals on the highway network local to the site.
1.13 This Updated Transport Assessment has been prepared with reference to the pre-application response received from OCC and subsequent planning application consultation response from OCC and addresses the matters identified within each response.
1.14 An Updated Framework Travel Plan has been prepared and is submitted under separate cover.
1.15 Following this introduction, the remainder of this report comprises the following:

- Section 2 outlines the transport planning policies that are considered pertinent to this application;
- Section 3 considers the existing use of the site and reviews the accessibility by all modes of transport;
- Section 4 provides an overview of the proposed development;
- Section 5 details the assessment methodology and the trip attraction of the development proposals;
- Section 6 outlines the results of the junction modelling undertaken; and,
- Section 7 summarises the key findings and conclusions of the report.


### 2.0 Policy Context

2.1 This section summarises the relevant transport policy documents against which the development proposals would be considered at a national, regional and local level. The most relevant policy documents relating to this study are detailed below:

- National Planning Policy Framework (March 2012);
- Oxfordshire Local Transport Plan 2015-2031 (July 2015); and,
- Cherwell Local Plan 2011-2031 (re-adopted December 2016).


## National Planning Policy

National Planning Policy Framework (March 2012)
2.2 The National Planning Policy Framework (NPPF) was published in March 2012, and replaces the previous national planning policies that were set out in the various Planning Policy Guidance Notes / Statements. With regard to transport, the NPPF replaces policy contained within PPG13 (Transport).
2.3 The NPPF sets out a presumption in favour of sustainable development that recognises the importance of transport policies in facilitating sustainable development, and that planning decisions should have regard to local circumstances. In this regard, paragraph 29 of the NPPF states that:
"The transport system needs to be balanced in favour of sustainable transport modes, giving people a real choice about how they travel. However, the Government recognises that different policies and measures will be required in different communities and opportunities to maximise sustainable transport solutions will vary from urban to rural areas."
2.4 Paragraph 32 states that:
"Development should only be prevented or refused on transport grounds where the residual cumulative impacts of development are severe."
2.5 In order to promote opportunities for the use of sustainable travel, the NPPF advises that:

- "..developments should be located and designed where practical to accommodate the efficient delivery of goods and supplies;
- give priority to pedestrian and cycle movements, and have access to high quality public transport facilities;
- create safe and secure layouts which minimise conflicts between traffic and cyclists or pedestrians, avoiding street clutter and where appropriate establishing home zones;
- Incorporate facilities for charging plug-in and other ultra-low emission vehicles; and consider the needs of people with disabilities by all modes of transport."
2.6 The NPPF also details the situations in which a local authority may utilise planning conditions or obligations in order to make a development acceptable. Paragraph 204 of the NPPF states that:
"Planning obligations should only be sought where they meet all of the following tests:
- necessary to make the development acceptable in planning terms;
- directly related to the development; and
- fairly and reasonably related in scale and kind to the development."


## Local Planning Policy

Cherwell Local Plan 2011-2031 (December 2016)
2.7 The Cherwell Local Plan is the key planning policy document within the district and sets out the overarching planning policies upon which planning applications will be determined.
2.8 Policy SLE 4 considers transport and connections and states:
"All development where reasonable to do so, should facilitate the use of sustainable modes of transport to make the fullest possible use of public transport, walking and cycling. Encouragement will be given to solutions which support reductions in greenhouse gas emissions and reduce congestion. Development which is not suitable for the roads that serve the development and which have a severe traffic impact will not be supported."
2.9 The current application site is allocated within the Cherwell Local Plan under Policy Bicester 4 which sets out:
"... This site to the south west of Bicester, bounded by the A41 to the north and west, is proposed for employment generating development in the form of a high-quality office scheme."
2.10 It is further stated in paragraph C. 65 that:
"There is a sustainable opportunity for the provision of strategic employment space to the south of Bicester Town Centre and adjoining the A41. The Bicester Business Park site has planning permission for a $60,000 \mathrm{~m} 2$ business park incorporating offices (B1) and hotel (C1) use. This development area is located immediately to the east of the South West Bicester (Kingsmere) urban extension, less than 1 km from Bicester Village Railway Station and close to major retail uses and town centre facilities. The site has immediate access to the strategic highway network (Oxford-Aylesbury) with Junction 9 of the M40 motorway situated about 3 km to the south. Major growth is planned nearby with the redevelopment of Graven Hill (Policy Bicester 2: Graven Hill, phase 2 of the South West Bicester extension (Policy Bicester 3: South West Bicester Phase 2 and the expansion of the centre of the town."

## Summary

2.11 It is evident that the policies set out within the NPPF and the Cherwell Local Plan focus on a presumption in favour of sustainable development and that development should only be resisted or refused on transport grounds where residual impacts of development are severe.
2.12 Furthermore, the application site is allocated for office use within the Cherwell Local Plan, confirming that the principle of office development is appropriate and in accordance with local planning policies.

### 3.0 Baseline Conditions

3.1 The site is located to the east of the A41, Oxford Road, and to the west of the Bicester - Oxford railway line. Both Bicester Village and town centre are located to the north of the site. The surrounding land uses comprise predominantly residential and retail uses with undeveloped land located to the east of the site.
3.2 The site location in relation to the surrounding area is shown in Figure 3.1.

Local Highway Network
3.3 Lakeview Drive forms the northern boundary of the site and the site would be accessed from Lakeview Drive via two existing roundabout junctions. The two existing roundabouts on Lakeview Drive, at the eastern end of Lakeview Drive and centrally on Lakeview Drive, currently include a southern arm on each roundabout which would form the vehicle accesses to the site. The roundabout at the eastern end of Lakeview Drive also provides access to the Tesco service yard while the central roundabout on Lakeview Drive also provides customer access to the existing Tesco store. At its western end, Lakeview Drive connects via the signalled controlled junction with the A41 Oxford Road. The A41 Oxford Road runs on a broadly north-south alignment and connects north to Bicester town and south to the M40.
3.4 North-east of the application site the A41 Oxford Road connects with the A41 at a junction known as the Esso roundabout. From the Esso roundabout, the A41 connects east towards Aylesbury. North of the Esso roundabout, Oxford Road connects north towards Bicester town centre.
3.5 As part of the recent developments of Bicester Village Phase 4 and the Tesco store a significant package of highway works was approved and has been implemented. The highway works included improvements to the Oxford Road junctions with Pingle Drive, Esso roundabout and Lakeview Drive.
3.6 Planning permission has recently been granted for a retail park scheme, known as 'Bicester Gateway Retail Park' on a site to the west of the A41 Oxford Road (Planning Ref: 16/02505/OUT). The permitted development proposals at Bicester Gateway Retail Park include further improvements to the A41 junctions with Lakeview Drive and Pioneer Way. The permitted highway improvements associated with Bicester Gateway Retail Park also include the provision of a new bus stop and lay-by on the A41 Oxford Road just south of Lakeview Drive, directly adjacent to the current application site.
3.7 In addition, planning permission has recently been granted for a business park scheme known as 'Bicester Gateway Business Park' to the south of the current application site (Planning Ref: 16/02586/OUT). The permitted development proposals at Bicester Gateway Business Park included improvements to the roundabout junction between the A41 and Vendee Drive.
3.8 The Rodney House roundabout is situated to the north-east of the application site at the junction between the A41, the A4421 and London Road and currently forms a conventional roundabout. As part of permitted development proposals at Graven Hill it is proposed that the Rodney House roundabout is upgraded to a signal-controlled roundabout and it is understood that these works are scheduled to commence later this year.

## Sustainable Transport Accessibility

3.9 It is generally accepted that walking and cycling provide important alternatives to the private car and should also be encouraged to form part of longer journeys via public transport. Indeed, it is noteworthy that the Institution of Highways and Transportation (IHT) has prepared several guidance documents that provide advice with respect to the provision of sustainable travel in conjunction with new developments. Within these documents it is suggested that:

- Most people will walk to a destination that is less than one mile (Planning for Walking, 2015);
- The bicycle is a potential mode of transport for all journeys under five miles (approximately 8 kilometres) (Planning for Cycling, 2015); and,
- Walking distances to bus stops should not exceed 400 metres, whilst people are prepared to walk twice as far to rail stations (Planning for Walking, 2015).
3.10 The Institution of Highways and Transportation (IHT) 'Guidelines for Providing Journeys on Foot' (2000) suggests acceptable, desirable and preferred maximum walking distances ('acceptable' walking distances would vary between individuals). Table 3.1 summarises the suggested walking distances for pedestrians without mobility impairment for some common trip purposes.

|  | Town Centres | Commuting/ Schools | Elsewhere |
| :--- | :---: | :---: | :---: |
| Desirable | 200 | 500 | 400 |
| Acceptable | 400 | 1,000 | 800 |
| Preferred Maximum | 800 | 2,000 | 1,200 |
| Source: 'Providing for Journeys on Foot', IHT, 2000 |  |  |  |

Table 3.1 Suggested Walking Distances (metres)
3.11 The following sections consider the opportunities for sustainable travel that are available in the vicinity of the site.

## Pedestrian Facilities

3.12 Footways are provided along both sides of Lakeview Drive adjacent to the site and these connect with footway along both sides of the A41 Oxford Road. Signalised pedestrian crossing facilities are provided at the junction between the A41 Oxford Road and Lakeview Drive and these provide a convenient crossing opportunity across both Lakeview Drive and the A41 Oxford Road.
3.13 The highway improvements have recently been completed at the A41 Esso roundabout and the A41 junctions with Pingle Drive and Pioneer Way which include signal-controlled pedestrian crossing facilities which connect to the wider pedestrian network in the vicinity.
3.14 In addition, the site is well located with regard to local footpaths which offer off-road connections between the site and local villages including Wendlebury and Chesterton. Figure 3.2 attached provides details of the local footpaths in the vicinity of the site.
3.15 It is evident that the pedestrian facilities in the vicinity of the application site provide connections to local retail opportunities, residential areas and public transport facilities. It is therefore evident that the application site is well placed for future employees and visitors to undertake journeys to and from the site on foot.

## Cycle Facilities

3.16 National Cycle Network Route 51 (NCN51), runs alongside the A41 Oxford Road directly past the application site and is a traffic-free shared pedestrian cycle route. NCN51 provides a signed cycle route connecting south towards Wendlebury, Kidlington and Oxford. North of the application site, NCN51 connects to Bicester Village and Bicester Town Centre.
3.17 There are further signed cycle routes in the vicinity of the site which operate throughout Bicester as well as connecting to Audley, Poundon and Langford Village. Figure 3.3 summarises the local cycle routes.
3.18 It is evident that the cycle facilities in the vicinity of the application site provide connections to local retail opportunities, residential areas and public transport facilities. It is therefore evident that the application site is well placed for future employees and visitors to undertake journeys to and from the site by cycle.

## Bus Services

3.19 The nearest bus stop to the site is situated on the A41 Oxford Road northbound, just north of the junction between the A41 Oxford Road and Lakeview Drive. The northbound bus stop is an approximately 120 metre walk from the north-western corner of the application site and is accessible via the existing signal-controlled pedestrian crossing facilities at the junction between A41 Oxford Road and Lakeview Drive. The bus stop is served by the S5 and X5 services. The S5 operates every 15 minutes Monday to Friday and every 30 minutes on Saturdays and Sundays between Oxford City Centre and Launton, as well as the Bicester Park \& Ride facility. The X5 operates twice an hour on weekdays and hourly on weekends between Cambridge Parkside Bus Station and Oxford City Centre via Milton Keynes Railway Station.
3.20 There is not currently a southbound bus stop directly adjacent to the site. However, as part of highways works associated with the permitted development proposals at Bicester Gateway Retail Park a new southbound bus stop and lay-by on the A41 Oxford Road would be provided. The new bus stop would be directly adjacent to the application site on the eastern side of the A41 Oxford Road. It is envisaged that the additional southbound bus stop would also be served by the S5 and X5 services
3.21 Additional bus stops are situated north of the Pingle Drive roundabout, approximately 500 metres north on Oxford Road and these are also served by the S5 and X5 services as well as the No. 26 bus service which provides a circular bus service between Bicester Town Centre, Kingsmere and Oxford Road. A further bus stop is located on Pingle Drive approximately 800 metres to the north east and is served by the Bicester Village Shuttle operating towards Bicester North Railway Station.
3.22 Table 3.2, summarises the bus routes and frequency of those routes which stop in the vicinity of the application site. Figure 3.4, attached, details the location of local bus stops and bus routes and destinations served by the bus services.

| Route No. | Route | Frequency |
| :---: | :---: | :---: |
| 8 | Middle Barton to Bicester | 2 per day (Friday only) |
| 26 | Bicester to Kingsmere | Every 30 minutes |
| NS5 | Oxford to Gosford \& Bicester | Every 60 minutes (night bus) |
| S5 | Oxford to Gosford \& Bicester | Every 10 to 20 minutes |
| X5 | Cambridge to Bedford \& Oxford | Every 30 minutes |

Table 3.2 Local Bus Services
3.23 Based on the bus services accessible from the bus stops in the vicinity of the site Table 3.3 provides a summary of the frequency of services to key local destinations

| Destination | Route | Peak Period Frequency |
| :---: | :---: | :---: |
| Bicester Town Centre | 8, NS5, S5, X5 | Every 5 to 10 minutes (6 per hour) |
| Bicester Village Train Station | 8,26, NS5, S5, X5 | Every 5 to 10 minutes (8 per hour) |
| Bicester North Train Station | $26, \mathrm{X} 5$ | Every 10 to 20 minutes (5 per hour) |
| Bicester Park \& Ride | NS5, S5 | Every 15 minutes (4 per hour) |
| Highfield | NS5, S5 | Every 15 minutes (4 per hour) |
| Kingsmere | 8,26, NS5, S5 | Every 10 to 15 minutes (6 per hour) |
| Oxford | NS5, S5 | Every 15 minutes (4 per hour) |

Table 3.3 Local Bus Frequency
3.24 Tables 3.2 and 3.3, along with Figure 3.4 demonstrate that the application site is well served by existing bus services. The existing bus services running adjacent to the application site on the A41 Oxford Road provide frequent connections to local destinations such as the town centre, residential areas such as Kingsmere and Highfield, as well as regular connections to both Bicester Village and Bicester North train stations.

Train Services
3.25 The nearest station is Bicester Village Railway Station located approximately 1.4 kilometres to the north east of the site. Bicester Village Station is located on the Oxford to London Marylebone line with services operating in each direction every 30 minutes.
3.26 Bicester North Railway Station is located approximately 1.8 kilometres to the north of the site and offers connections to London Marylebone, Banbury and Birmingham Moor Street and Snow Hill. Services run up to twice per hour in each direction.
3.27 As demonstrated at Table 3.3 and Figure 3.4 there are a number of bus services running directly adjacent to the site which provide a frequency connection between the application site and both Bicester Village and Bicester North railway stations.
3.28 It is evident that the application site is well placed for access to public transport facilities and provides future employees and visitors to the site to undertake journeys by public transport.

## Personal Injury Accident Data

3.29 Personal Injury Accident (PIA) data recorded in the immediate vicinity of the site has been obtained from Thames Valley Police for the most recent five-year period available covering 01/07/2012 to $01 / 07 / 2017$. Full details of the study area and accident records are attached at Appendix C. Over this period there were 47 incidents recorded of which 40 resulted in slight injury, 5 in serious injury and 2 resulted in fatality.
3.30 A further set of traffic accident data has been obtained from the Road Safety Officer at OCC and that data is also attached at Appendix C. The data provided by OCC is consistent with that provided by Thames Valley Police, other than the OCC covers a more recent period up to the end of December 2017.
3.31 Both the Thames Valley Police and OCC data show only one incident occurred at the junction between A41 Oxford Road and Lakeview Drive during the assessment period. The incident was slight in severity and the primary causation factor of the incident is identified by both the OCC and Thames Valley Police data as a failure to obey the traffic signals.
3.32 The Thames Valley data identified a total of 4 incidents occurred at the junction between the A41 and Pioneer Way (Kingsmere Access) and each is identified as slight in severity. The causation factors of each of the incidents relate to driver error or poor driver behaviour including failure to look properly, following too close, failed to judge other person path or speed, avoiding an animal in the carriageway and exceeding the speed limit. The OCC data indicates some further incidents of slight severity at the Pioneer Way junction all of which have causation factors relating to driver error or poor driver behaviour including failed to look properly, disobeyed automatic traffic signal and failed to judge other person path or speed.
3.33 None of the incidents recorded at the Lakeview Drive or Pioneer Way junctions have causation factors identified that relate to the design or layout of the highway in this area.
3.34 The incident reports in relation to the two incidents which result in a fatality, identify that they were as a result of a failure to judge other vehicle speeds and distraction within the vehicle. A review of the remaining incidents on the local network indicates that the identified causation factors were predominantly driver error or poor driver behaviour and, as such, are unrelated to the existing design or layout of the highway.
3.35 As such, it is concluded that there are no inherent safety issues associated with the existing highway network and junction arrangements in the vicinity of the site. Discussions with Officers at OCC have confirmed that they agree with this conclusion.

## Previous Planning Consent

3.36 As previously highlighted the application site has previously been subject to a planning application for an office park development with outline planning permission granted in 2010 for the construction of a 60,000 -square metre office park comprising 53,000 square metres of $B 1(a) / B 1(b)$ office space and a 7,000-square metre C1 hotel, served by circa 1,837 car parking spaces (Planning Ref: 07/01106/OUT). A subsequent planning application for the Tesco development at the site allowed for the construction of up to 45,000 square metres of the $B 1(a) / B 1(b)$ office space being delivered on the remainder of the site, as part of the previous outline planning permission for an office park.
3.37 Tables 3.4 to 3.7 below summaries the results of the junction modelling of the local road network with the previously consented 45,000 square metres of the $B 1(a) / B 1(b)$ office space in place. Junction model output files are attached at Appendix K.

| Approach | AM Peak |  | PM Peak |  |
| :--- | :---: | :---: | :---: | :---: |
|  | RFC | Queue (veh) | RFC | Queue (veh) |
| Middleton Stoney Road | 0.74 | 2.9 | 0.76 | 3.1 |
| Kings End | 1.18 | 209.6 | 0.88 | 7.2 |
| Oxford Road | 0.50 | 1.0 | 0.86 | 6.0 |

Table 3.4: Oxford Road/ Middleton Stoney Road/ Kings End - 2026 With 45,000sqm Office Park

| Junction | AM Peak |  | PM Peak |  |
| :--- | :---: | :---: | :---: | :---: |
|  | DoS | MMQ | DoS | MMQ |
| Oxford Road/ Pingle Drive | 48.8 | - | $61.8 \%$ | - |
| Esso Roundabout | 96.2 | - | $100.3 \%$ | - |
| Oxford Road/ Pioneer Way | 78.3 | - | $91.4 \%$ | - |
| Oxford Road/ Lakeview Drive | 88.8 | - | $100.8 \%$ | - |
| Oxford Road n/b (Ahead) | 54.6 | 9 | $82.4 \%$ | 33 |
| Oxford Road n/b (Ahead) | 61.1 | 25 | $83.3 \%$ | 34 |
| Oxford Road n/b (Ahead/ Right) | 88.8 | 25 | $94.7 \%$ | 25 |
| Oxford Road s/b (Left) | 53.1 | 6 | $27.7 \%$ | 4 |
| Oxford Road s/b (Ahead) | 79.3 | 25 | $100.1 \%$ | 49 |
| Oxford Road s/b (Ahead) | 83.0 | 31 | $100.7 \%$ | 51 |
| Lakeview Drive (Left/ Right) | 39.4 | 6 | $100.8 \%$ | 46 |
| Lakeview Drive (Right) | 76.0 | 5 | $97.9 \%$ | 27 |
| Overall PRC |  | $-6.9 \%$ |  | $-12.0 \%$ |

Table 3.5: Oxford Road Corridor - 2026 With 45,000sqm Office Park

| Approach | AM Peak |  | PM Peak |  |
| :--- | :---: | :---: | :---: | :---: |
|  | RFC | Queue (veh) | RFC | Queue (veh) |
| Vendee Drive | 0.33 | 0.5 | 0.26 | 0.3 |
| A41 (North) | 0.68 | 2.1 | 0.84 | 5.4 |
| Unnamed Road | 0.14 | 0.2 | 0.38 | 0.6 |
| A41 (South) | 0.72 | 2.6 | 0.80 | 3.9 |
| Bicester Park and Ride | 0.02 | 0.0 | 0.14 | 0.2 |

Table 3.6: A41/ Vendee Drive - 2026 With 45,000sqm Office Park

| Approach | AM Peak |  | PM Peak |  |
| :--- | :---: | :---: | :---: | :---: |
|  | DoS | MMQ | DoS | MMQ |
| A41 (Left/ Ahead) | $73.3 \%$ | 11.6 | $83.1 \%$ | 17.8 |
| A41 (Ahead) | $8.1 \%$ | 1.1 | $20.2 \%$ | 2.9 |
| Graven Hill Road (Left) | $58.9 \%$ | 5.7 | $56.7 \%$ | 4.7 |
| Graven Hill Road (Ahead) | $41.6 \%$ | 3.5 | $34.8 \%$ | 2.7 |
| A41 (Left/ Ahead) | $53.9 \%$ | 5.9 | $76.4 \%$ | 11.1 |
| A41 (Ahead) | $43.8 \%$ | 7.2 | $45.7 \%$ | 7.5 |
| B4100 (Left/ Ahead) | $33.3 \%$ | 2.6 | $46.5 \%$ | 3.7 |
| B4100 (Ahead) | $50.0 \%$ | 4.1 | $61.2 \%$ | 5.2 |
| A4421 (Left/ Ahead) | $63.6 \%$ | 8.5 | $64.5 \%$ | 8.4 |
| A4421 (Ahead) | $36.6 \%$ | 4.6 | $35.7 \%$ | 4.1 |
| Overall PRC |  |  |  | $+7.8 \%$ |

Table 3.7: Rodney House Roundabout - 2026 With 45,000sqm Office Park
3.38 The analysis demonstrates that if the previous consented 45,000sqm development and associated highway improvements were implemented, the junction between Oxford Road and Middleton Stoney Road would be expected to operate over capacity during the morning peak hour. The A41 corridor junctions would be expected to operate over theoretical capacity during the evening peak hour and within theoretical capacity but with negative practical reserve capacity during the morning peak hour. The junction of the A41/ Vendee Drive and the Rodney House roundabout would operate within capacity.

### 4.0 Development Proposals

4.1 The current planning application seeks outline planning permission, with all matters reserved except access, for the development of up to 60,000 square metres (GEA) of $B 1(a) / B 1(b)$ office space. Vehicle access to the site would be via the two existing roundabout junctions on Lakeview Drive. The parameters plan of the current outline application is attached at Appendix $D$.

## Site History

4.2 As previously highlighted, outline planning permission was granted in 2010 for the construction of a 60,000 -square metre office park comprising 53,000 square metres of $B 1(a) / B 1$ (b) office space and a 7,000-square metre C1 hotel, served by circa 1,837 car parking spaces (Planning Ref: 07/01106/OUT).
4.3 Detailed planning permission was subsequently granted in November 2013 for the construction of a Tesco food store of 8,135 square metres and petrol filing station on part of the permitted office park site (Planning Ref: 12/01193/F). That planning application was supported by a Transport Assessment which considered the effect of the development proposals on the highway network local to the site. The Tesco store has since been constructed and opened in April 2016.
4.4 The S106 Deed of Variation in relation to the permitted Tesco store and office park allows for the construction of up to 45,000 square metres of the $B 1(a) / B 1(b)$ office space being delivered on the remainder of the site, as part of the previous outline planning permission for an office park.

## Current Planning Application

4.5 The current development proposals seek outline planning permission for the construction of an office park providing up to 60,000 square metres of $B 1(a) / B 1(b)$ office space.
4.6 The current development proposals would supersede and replace the previous outline permission for an office park on the site. In comparison with the previous outline planning permission for an office park on the site, the current site area excludes the portion of the site, north of Lakeview Drive, which has since been developed for a Tesco store. However, the site area now includes a parcel of land along the frontage of the A41 Oxford Road, south of Lakeview Drive, which was previously not within the applicant's ownership and was not part of the previous outline planning permission for an office park.
4.7 The previous outline planning permission on the site allows for the construction of up to 45,000 square metres of the $B 1(a) / B 1(b)$ office space. The current planning application therefore provides an additional 15,000 square metres of $B 1(a) / B 1(b)$ office space in comparison with the previous outline planning permission. However, this Transport Assessment consider the effect of a new 60,000 square metres of $\mathrm{Bl}(\mathrm{a}) / \mathrm{Bl}(\mathrm{b})$ office space and this assessment is presented at Sections 5 and 6.

## Vehicle Access Arrangements

4.8 Lakeview Drive forms the northern boundary of the site and vehicle access to the site would be taken from Lakeview Drive via the two existing roundabout junctions. The two existing roundabouts on Lakeview Drive, at the eastern end of Lakeview Drive and centrally on Lakeview Drive, currently include a southern arm on each roundabout which would form the vehicle accesses to the site. The roundabout at the eastern end of Lakeview Drive also provides access to the Tesco service yard while the central roundabout on Lakeview Drive also provides customer access to the existing Tesco store.

## Pedestrian and Cycle Access

4.9 A pedestrian footway is currently provided along both sides of Lakeview Drive adjacent to the application site and this extends along the southern arms of the existing roundabout junctions. This footway will provide the main pedestrian access to the site and connects west to existing signalcontrolled pedestrian crossing facilities at the junction between Lakeview Drive at the A41 Oxford Road.
4.10 It is proposed that a further pedestrian access is provided on the western boundary of the site with A41 Oxford Road. Given that the current planning application is in outline form the position of buildings and internal street layout within the site is not known. The position and alignment of the pedestrian connection to A41 Oxford Road would be dependent on the desire line of pedestrians into the site and the position of buildings and internal street. It is envisaged that this will be positioned to provide a convenient connection with the existing pedestrian crossing facilities on the A41 Oxford Road at its junction with the Prior Way, with materials to match with existing, subject to agreement with the local highway authority. The provision of the pedestrian access from A41 Oxford Road would be delivered as part of the Section 278 agreement associated with the proposed highways works. The proposed access arrangements to the site are illustrated on the Highways Access Plan, attached at Appendix E.
4.11 Given that the current application is in outline form, the internal site layout has not been designed at this stage. A parameters plan is attached at Appendix D. Full details of the internal site layout including internal road layout and internal pedestrian network will be provided at the reserved matters stage and with consideration of local design guidance.
4.12 A shared pedestrian/ cycle route is currently provided along the eastern side of A41 Oxford Road between the junctions of Lakeview Drive and Pioneer Way along the western boundary of the site. The existing pedestrian/ cycle route is approximately 2 metres wide. As part of the Section 278 highway works the shared pedestrian/ cycle route on the eastern side of A41 Oxford Road will be widened to 3 metres wide from the junction with Lakeview Drive to the crossing at the Pioneer Way junction and this is shown on the proposed highway works drawings attached at Appendix F.

## Public Transport Access

4.13 As demonstrated in Section 3 of this Transport Assessment the application site is currently well served by existing bus services. The existing bus services running adjacent to the application site on the A41 Oxford Road provide frequent connections to local destinations such as the town centre, residential areas such as Kingsmere and Highfield, as well as regular connections to both Bicester Village and Bicester North train stations.
4.14 Planning permission has recently been granted for the Bicester Gateway Retail Park on a site to the west of the A41 Oxford Road (Planning Ref: 16/02505/OUT). The permitted development proposals at Bicester Gateway Retail Park include the provision of a new bus stop and bus lay-by on the A41 Oxford Road southbound just south of Lakeview Drive, directly adjacent to the current application site. Should the current development come forward in advance of the Bicester Gateway Retail Park the it would provide the southbound bus stop on Oxford Road, as shown on the proposed highway works drawings attached at Appendix $F$.
4.15 As set out above, the development will provide a new pedestrian/ cycle link between the application site and the A41 Oxford Road. The position of that pedestrian connection would be dependent on the internal site layout which has not been detailed at this outline stage. However, it is envisaged that this will connect to footway/ cycleway on the A41 Oxford Road in the vicinity of the A41/ Pioneer Way junction and provide a convenient connection towards the proposed southbound bus stop.
4.16 On the basis of the proximity of the bus stops and the frequency of the bus services it is considered that the development is adequately served by bus services which connect to local destinations and railway stations.

## Parking

4.17 Car parking will be provided in accordance with OCC maximum parking standards. OCC parking standards allow the provision 1 space per 30 square metres of B1 office floor space. The proposed office park will therefore provide 2,000 car parking spaces to serve the development. The proposed car parking provision is in accordance with OCC parking standards and is considered appropriate to meet the needs of the development.
4.18 Traffic Advisory Leaflet 5/95 'Parking for Disabled People' advises that for employment uses providing over 200 car parking spaces, disabled parking should be provided at a ratio of 6 bays plus $2 \%$ of total capacity. Disabled parking will be provided in accordance with this guidance and based on the provision of 2,000 car parking spaces it is envisaged that 46 disabled car parking spaces will be provided. It is proposed that a Car Park Management Plan would be secured by planning condition.
4.19 Cycle parking can be accommodated in accordance with OCC standards and will provide a mixture of long-stay parking for employees and short stay parking for visitors. For B1 employment use, OCC standards require the provision of 1 cycle parking space per 150 square metres for long stay employee cycle parking and 1 space per 500 square metres for short stay visitor parking and these could be accommodated on site, if required.

## Servicing and Deliveries

4.20 Servicing and deliveries associated with the development, including refuse collection, will be undertaken on site and off the public highway.
4.21 Given that the current application is in outline form, the internal site layout has not been designed at this stage. A parameters plan is attached at Appendix D. Full details of the internal site layout including internal road layout will be provided at the reserved matters stage and with consideration of local design guidance, vehicle requirements and with swept path analysis where required.

## Proposed Highways Works

4.22 Following an assessment of the effect of the development proposals on the highway network local to the site, highway mitigation works have been identified at two junctions, namely; the A41 Oxford Road/ Lakeview Drive junction and the Oxford Road/ Middleton Stoney Road junction.
4.23 Further details of the assessment of the development proposals on the local highway network and the proposed off-site highways works are detailed at Section 6 of this Transport Assessment and drawings showing the proposed highway mitigation works are provided at Appendix F.
4.24 Drawings 170221-07 and 170221-08 detail the proposed highway works at the junction between A41 Oxford Road and Lakeview Drive to provide additional capacity at the junction. The proposed highway improvement works include the provision of a southbound bus stop and layby on the A41 Oxford Road, should this not be provided by other permitted developments. The works also include widening of the foot/cycleway along the eastern side of Oxford Road to provide a 3 -metre wide foot/cycleway between the Lakeview Drive junction and the Pioneer Way junction.
4.25 Swept path analysis has been undertaken for the right turn movements from Lakeview Drive to Oxford Road northbound and Oxford Road northbound to Lakeview Drive. Drawings 170221-TK02 and 170221-TK03, attached at Appendix G, show the swept path analysis and demonstrate that vehicles can undertake both movements appropriately.
4.26 For both movements the swept path analysis demonstrates that a car and an articulated vehicle can undertake the two right turn movements simultaneously. This is considered a robust assessment and unlikely to occur in reality as should an articulated vehicle and car be stationary at the stopline in adjacent lanes it is expected that the car would accelerate away from the stopline quick than the articulated vehicle and therefore reach the entry between the islands in advance of the articulated vehicle.
4.27 Swept path analysis is also provided of two cars and two transit type vans simultaneously undertaking the two right turn movements and demonstrates that both can comfortably be accommodated.
4.28 Drawings 170221-09, attached, demonstrates that the central refuge island in the vicinity of the Lakeview Drive junction is of sufficient size to accommodate traffic signal poles and heads.
4.29 The proposed highway improvement works have been subject to a Stage 1 Road Safety Audit (S1RSA), which is attached at Appendix H. The S1RSA does not raise any material concerns with the proposed highway improvements works at the junction.

## Planning Obligations

4.30 With regard bus infrastructure and services, this Updated Transport Assessment has demonstrated that the site is adequately served by a number of bus services on the A41 Oxford Road including the No. 6, 26, NS5, S5 and X5 services. The existing bus routes provide a frequent connection to local key destinations including Bicester Town Centre (every 10 minutes) and Bicester Village Station (every 510 minutes).
4.31 There is an existing northbound bus stop on the A41 Oxford Road and a new southbound bus stop will be provided on Oxford Road directly adjacent to the site through a Section 278 Agreement, as shown on the submitted drawings. It is not proposed to provide a bus stop on Lakeview Drive as it is considered that the existing and proposed bus stops on Oxford Road provide appropriate access to bus services. Furthermore, it is highly unlikely that a bus operator will divert a bus service into the site from Oxford Road as this would add delay to bus journey times.
4.32 Bus infrastructure and services in the vicinity of the application site have been significantly improved since the time of the previous outline planning permission at the site. At that time there were no bus stops on the A41 Oxford Road in the vicinity of the application site and the nearest bus stops to the site were either located north of Pingle Drive or south of the site in the vicinity of the junction connecting towards Chesterton. Furthermore, at that time, there were 3 to 4 bus services per hour passing the application site on Oxford Road during peak periods. At present there are 8 bus services per hour passing the application site on Oxford Road during peak periods, double the frequency of service at the time of the previous outline planning permission. It is evident that the bus infrastructure and services in the vicinity of the site and are adequate to serve the development and no obligation in this regard is considered necessary or justified in planning terms.
4.33 The Section 106 Agreement in relation to the previous outline planning permission include a contribution towards strategic rail infrastructure, however, this was limited to the improvements of the railway line between Bicester Village and Oxford. These improvements have subsequently been completed and rail services from Bicester Village Station (previously known as Bicester Town) have been significantly enhanced since the time of the previous outline planning permission.
4.34 At the time of the previous outline planning permission Bicester Village Station was at the end of a local line connecting to Oxford and provided just 7 trains per day between Bicester and Oxford only. The upgraded services at Bicester Village station now provide 2 trains per hour in each direction connecting between London Marylebone and Oxford. It is evident that the enhancements to train services through Bicester Village station expected as part of the original planning permission have since been implemented and therefore no obligation in this regard is considered necessary or justified in planning terms.
4.35 The highway capacity assessment, presented at Section 6 of this Transport Assessment has concluded that, subject to the identified highway mitigation works, the development proposals would not result in a material effect on the operation of the highway network local to the site. As such, no further mitigation measures or Section 106 obligations towards further highway infrastructure schemes are considered necessary or justified in planning terms.

### 5.0 Assessment Methodology and Trip Attraction

5.1 This section of the report considers the expected trip attraction of the development proposals and the methodology for assessing the effect of the development proposals on the highway network local to the site.

## Scope of Assessment

5.2 As part of pre-application scoping discussions, Officers at OCC have requested that the following junctions be assessed as part of the Transport Assessment:

- Oxford Road (A41) / Lakeview Drive signalised junction;
- Oxford Road / A41 signalised roundabout;
- Oxford Road / Pingle Drive roundabout;
- Oxford Road / Middleton Stoney Road mini-roundabout;
- Oxford Road (A41) / Pioneer Way signalised junction;
- A41 Oxford Road/ Vendee Drive; and
- A41/ A4421 Rodney House Roundabout.
5.3 As previously identified, highway improvement works have recently been completed at a number of the junctions listed above. In addition, further highway improvement works are permitted at some junctions listed above in association with recently permitted development proposals. The highway capacity assessment undertaken within this Transport Assessment considers the permitted junction improvements at the junctions listed above.


## Baseline Traffic Flows, Committed Developments \& Assessment Periods

5.4 As part of the pre-application scoping discussions Officers at OCC have requested that the assessment of the highway network local to the site be undertaken using traffic flow information provided from the Bicester Transport Model (BTM).
5.5 The BTM is based on a future assessment of 2026, 9 years in advance of the current application submission date. The assessment of a future baseline year 9 years after the submission of a planning application is considered a robust assessment of the local highway network.
5.6 The current planning application is for a $B 1(a) / B 1(b)$ office park and, as such, the primary effect of the development proposals on the highway network local to the site will be during the weekday morning and evening peak periods. Given the proposed office use of the site it is considered that outside these periods and, in particular during the weekend Saturday and Sunday peak periods, the development will attract negligible vehicle trips and, as such, would not have a material effect on the operation of the highway network at these times. As such, this Transport Assessment will consider the effect of the development proposals on the highway network during the weekday morning and evening peaks.
5.7 OCC has provided outputs from the BTM for the weekday morning and evening peak hours. BTM outputs provided by OCC are attached at Appendix I. In addition, Figures 5.1 and 5.2, attached, summarises the 2026 baseline traffic flows for the weekday morning and evening peak hours which will form the base for the assessment.
5.8 Following submission of the planning application, OCC has confirmed that there are two further committed developments which are not included with the BTM base traffic flows but should be considered as part of the assessment, these are:

- Bicester Gateway Retail Park (Planning Ref: 16/02505/OUT);
- McDonalds, Lakeview Drive (Planning Ref: 17/00889/FUL); and,


## - Bicester Gateway Phase 4 (Planning Ref: 12/01209/F)

5.9 Details of the expected trip generation of the above permitted developments has been extracted from the respective Transport Assessment, and subsequent Addendums and Technical Notes, submitted in connection with those planning applications.
5.10 Vehicle trips associated with the permitted Bicester Gateway Retail Park use have been extracted from the Transport Assessment submitted alongside that planning application (Figure 2-18 of the Bicester Gateway submitted Transport Assessment). Where the scope of junctions considered as part of the Bicester Gateway Retail Park assessment does not cover the scope of junctions currently being assessed (Vendee Drive, Rodney House and Middleton Stoney Road roundabouts) the vehicle trips have been distributed based on the traffic distribution presented at Figure 5.10 of this Transport Assessment and agreed with Officers at OCC. The expected traffic flows for the Bicester Gateway Retail Park development are presented at Figures 5.3, attached, and include consideration of pass-by and linked trips associated with that permitted development.
5.11 Vehicle trips associated with the permitted McDonalds development have been extracted from the Transport Assessment submitted alongside that planning application. The Transport Assessment included consideration of pass-by and linked trips associated with the development. The Transport Assessment did not include a single Figure detailed the total trip attraction of the McDonalds development proposals and as such this has been calculated by subtracting the baseline traffic flows for the weekday morning and evening peaks (as presented at Figures 9 and 10 of the McDonalds Transport Assessment) from the 'With Development' baseline traffic flows for the weekday morning and evening peaks (as presented at Figures 13 and 14 of the McDonalds Transport Assessment). Figures 5.4 and 5.5 , attached, show the vehicle trips associated with the McDonalds development for the weekday morning and evening peak, respectively.
5.12 Vehicle trips associated with Bicester Village Phase 4 (BV Phase 4) development have been extracted from the Transport Assessment submitted alongside that planning application. That Transport Assessment considered the weekend evening peak period but did not assess the weekday morning peak period. Traffic flows associated with the BV Phase 4 during the weekday evening peak period have been extracted from that Transport Assessment and are presented at Figure 5.6. The BV Phase 4 Transport Assessment demonstrated that that development would result in an average increase of $10 \%$ in traffic flow associated with the site across the peak periods assessed. It is therefore considered reasonable that the BV Phase 4 development would result in a $10 \%$ increase in traffic flow during the morning peak period. A $10 \%$ growth factor has therefore been applied to the baseline Bicester Village traffic flows presented at Figure 5.1. Figure 5.7, attached, shows the vehicle trips associated with the BV Phase 4 development for the weekday morning peak period.
5.13 The traffic flows associated with the Bicester Gateway Retail Park, McDonalds and BV Phase 4 developments, as shown at Figures 5.3 to 5.7 have been added to the baseline BTM traffic flows provided by OCC, as shown at Figures 5.1 and 5.2. Baseline BTM traffic inclusive of the additional developments during the weekday morning and evening peak periods are shown at Figures 5.8 and 5.9.

## Trip Attraction

5.14 As previously highlighted the application site has previously been subject to a planning application for an office park development with outline planning permission granted in 2010 for the construction of a 60,000 -square metre office park comprising 53,000 square metres of $B 1(a) / B 1$ (b) office space and a 7,000-square metre C1 hotel, served by circa 1,837 car parking spaces (Planning Ref: 07/01106/OUT). Detailed planning permission was subsequently granted in November 2013 for the construction of a Tesco food store of 8,135 square metres and petrol filing station on part of the permitted office park site (Planning Ref: 12/01193/F). The Tesco store has since been constructed and opened in April 2016.
5.15 The planning application for the Tesco development was supported by a Transport Assessment which considered the effect of the Tesco development proposals on the highway network local to the site. The S106 Deed of Variation in relation to the permitted Tesco store and office park allows for the construction of up to 45,000 square metres of the $B 1(a) / B 1(b)$ office space being delivered on the remainder of the site, as part of the previous outline planning permission for an office park.
5.16 The Transport Assessment supporting the Tesco development proposals assessed the effect of 45,000 square metres of office park development coming forward on the current application site. To this extent, the junction between Lakeview Drive and the A41 Oxford Road has been designed and was previously assessed to accommodate traffic associated with up to 45,000 square metres of the B1(a)/B1(b) office space in addition to the constructed Tesco store. Furthermore, the Tesco Transport Assessment assessed the effect of up to 45,000 square metres of the $B 1(a) / B 1(b)$ office space, in addition to the constructed Tesco store, on the highway network local to site. As such the highway improvements designed and implemented as part of the Tesco development included consideration of 45,000 square metres of $B 1(a) / B 1(b)$ office space on the application site.
5.17 It is therefore evident that the current outline planning application for 60,000 square metres of $B 1(a) / B 1(b)$ office space comprise an additional 15,000 square metres of office space in comparison with that previous assessed on the local highway network.
5.18 Whilst planning consent has previously been granted for 45,000 square metres of the $B 1(a) / B 1(b)$ office space on the application site, this Transport Assessment assesses the effect of a new 60,000 square metres of $B 1(a) / B 1(b)$ of office space on the local highway network.
5.19 The pre-application response from OCC requested that the expected trip attraction of the current development proposals be considered with reference to trip rates presented within the Transport Assessment supporting the recently permitted development proposals at Bicester Gateway Business Park (Planning Ref: 16/02586/OUT).
5.20 Table 5.1 below summarises the vehicle trip rates and expected vehicle trips associated with the proposed 45,000 square metres of $\mathrm{Bl}(\mathrm{a}) / \mathrm{B} 1(\mathrm{~b})$ office floorspace during the weekday morning and evening peak periods.

|  | Trip Rate (per 100sqm) |  |  | Total Trips (45,000sqm) |  |  |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: |
|  | In | Out | Total | In | Out | Total |
| Morning Peak Hour | 1.533 | 0.141 | 1.674 | 690 | 63 | 753 |
| Evening Peak Hour | 0.111 | 1.602 | 1.713 | 50 | 721 | 771 |

Table 5.1: Trip Rates and Vehicle Trips - Office Park (45,000 square metres)
5.21 Table 5.1 demonstrates that the previously permitted office park development would be expected to result in 753 vehicle trips during the morning peak hour and 771 vehicle trips during the evening peak hour.
5.22 The current planning application seeks permission for 60,000 square metres of $\mathrm{B} 1(\mathrm{a}) / \mathrm{B} 1(\mathrm{~b})$ office floorspace, 15,000 square metres greater than the previously permitted scheme on the site. Table 5.2 below summarises the vehicle trip rates and expected vehicle trips associated with the proposed 15,000 square metres of $B 1(a) / B 1(b)$ office floorspace during the weekday morning and evening peak periods.

|  | Trip Rate (per 100sqm) |  |  | Total Trips (15,000sqm) |  |  |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: |
|  | In | Out | Total | In | Out | Total |
| Morning Peak Hour | 1.533 | 0.141 | 1.674 | 230 | 21 | 251 |
| Evening Peak Hour | 0.111 | 1.602 | 1.713 | 17 | 240 | 257 |

Table 5.2: Trip Rates and Vehicle Trips - Office Park (15,000 square metres)
5.23 Table 5.2 demonstrates that the current office park development would be expected to result in an additional 251 vehicle trips during the morning peak hour and 257 vehicle trips during the evening peak hour in comparison with the previously permitted office park development.
5.24 Table 5.3 below summarises the vehicle trip rates and expected vehicle trips associated with the proposed 60,000 square metres of $\mathrm{Bl}(\mathrm{a}) / \mathrm{B} 1(\mathrm{~b})$ office floorspace during the weekday morning and evening peak periods.

|  | Trip Rate (per 100sqm) |  |  | Total Trips (60,000sqm) |  |  |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: |
|  | In | Out | Total | In | Out | Total |
| Morning Peak Hour | 1.533 | 0.141 | 1.674 | 920 | 85 | 1,004 |
| Evening Peak Hour | 0.111 | 1.602 | 1.713 | 67 | 961 | 1,028 |

Table 5.3: Trip Rates and Vehicle Trips - Office Park (60,000 square metres)
5.25 Table 5.3 demonstrates that the entire office park development is expected to result in 1,004 vehicle trips during the morning peak hour and 1,028 vehicle trips during the evening peak hour.

## Trip Distribution

5.26 In order to determine the likely distribution of vehicle trips on the local road network, reference has been made to journey to work data from the 2011 Census for the Cherwell 015 output area in which the application site is located.
5.27 Census data and trip distribution calculations are provided at Appendix J and Figure 5.10, attached, details the expected distribution of vehicle trips on the local highway network and this is summarised below:

| P A41 South | $27 \%$ |  |
| :--- | :--- | :--- |
| $>$ Vendee Drive | $12 \%$ |  |
| $>$ | Kingsmere | $3 \%$ |
| $>$ A41 East | $23 \%$ |  |
| $>$ A41 North | $35 \%$ |  |

5.28 Vehicle trips associated with the development proposals have been assigned on the local road network based on the distribution set out at Figure 5.10. The proposed traffic distribution has been agreed with Officers at OCC.
5.29 Figures 5.11 and 5.12 show the expected distribution of vehicle trips associated with the previously permitted 60,000 square metres of office park during the weekday morning and evening peak hours, respectively.

## 'With Development' Assessment

5.30 As set out above, Figures 5.8 and 5.9, attached, present 2026 baseline traffic flows from the BTM inclusive of the additional Bicester Gateway Retail Park, McDonalds and BV Phase 4 developments for the weekday morning and evening peak hours respectively.
5.31 Traffic flows associated with the proposed 60,000 square metres of office park development, as shown at Figures 5.11 and 5.12, have been added to the baseline traffic flows. Figures 5.13 and 5.14 show the traffic flows from the BTM for inclusive of the additional Bicester Gateway Retail Park, McDonalds and BV Phase 4 developments and the proposed 60,000 square metres of office park development at the site for the weekday morning and evening peak hours, respectively.

### 6.0 Effect of Development

6.1 This section of the report considers the effect of the development on the highway network local to the site based on junction capacity modelling of the junctions agreed with Officers at OCC during preapplication scoping discussions.
6.2 As part of pre-application scoping discussions, Officers at OCC have requested that the following junctions be assessed as part of the Transport Assessment:

- Oxford Road (A41) / Lakeview Drive signalised junction;
- Oxford Road / A41 signalised roundabout;
- Oxford Road / Pingle Drive roundabout;
- Oxford Road / Middleton Stoney Road mini-roundabout;
- Oxford Road (A41) / Pioneer Way signalised junction;
- A41 Oxford Road/ Vendee Drive; and
- A41/ A4421 Rodney House Roundabout.
6.3 As previously identified, highway improvement works have recently been completed at a number of the junctions listed above. In addition, further highway improvement works are permitted at some junctions listed above in association with recently permitted development proposals. The highway capacity assessment undertaken within this Transport Assessment considers the operation of the junctions with these improvements in place. Junction capacity modelling has been undertaken using the industry standard modelling package for each junction type i.e. ARCADY for conventional roundabouts and mini-roundabouts and LinSig for signal-controlled junctions and signal-controlled roundabouts.
6.4 The assessment has considered three scenarios at each of junctions within the scope as follows:
- Scenario 1-2026 BTM plus McDonalds, Bicester Gateway Retail Park and BV Phase 4
- Traffic Flow based Figures 5.8 and 5.9
- Scenario 2-2026 BTM plus committed and 60,000 square metres Office Park.
- Traffic Flows based on Figures 5.13 and 5.14
- Scenario 3 - As above. With mitigation, if required.

Oxford Road/ Middleton Stoney Road/ Kings End
6.5 The mini-roundabout junction between Oxford Road, Middleton Stoney Road and Kings End has been modelled using ARCADY. It is noted that ARCADY is subject to limitations when assessing the operation of mini-roundabouts and can be unrepresentative of observed operation. To this extent it is considered more appropriate to assess the operation of the junction as a conventional roundabout within ARCADY.
6.6 Table 6.1 shows the operation of the junction in the 2026 baseline BTM (plus committed developments) scenario. Model output files are attached at Appendix K.

| Approach | AM Peak |  | PM Peak |  |
| :--- | :---: | :---: | :---: | :---: |
|  | RFC | Queue (veh) | RFC | Queue (veh) |
| Middleton Stoney Road | 0.65 | 1.8 | 0.69 | 2.2 |
| Kings End | 0.94 | 12.5 | 0.87 | 6.3 |
| Oxford Road | 0.49 | 0.9 | 0.74 | 2.9 |

Table 6.1: Oxford Road/ Middleton Stoney Road/ Kings End - 2026 Baseline plus Committed Operation
6.7 The analysis shows that the junction is expected to operate within theoretical capacity (RFC less than 1) during the both the morning and evening peak periods, although the RFC of Kings End exceeds 0.9 during the morning peak hour.
6.8 Table 6.2 shows the operation of the junction in the 2026 baseline BTM (plus committed developments) scenario including the proposed 60,000 square metres of Office Park at the application site. Model output files are attached at Appendix K.

| Approach | AM Peak |  | PM Peak |  |
| :--- | :---: | :---: | :---: | :---: |
|  | RFC | Queue (veh) | RFC | Queue (veh) |
| Middleton Stoney Road | 0.78 | 3.4 | 0.79 | 3.6 |
| Kings End | 1.26 | 290.9 | 0.89 | 7.4 |
| Oxford Road | 0.50 | 1.0 | 0.90 | 8.5 |

Table 6.2: Oxford Road/ Middleton Stoney Road/ Kings End - 2026 Baseline Operation (including 60,000 square metres of office space)
6.9 The analysis shows that the Kings End arm of the junction would be expected to operate over capacity during the morning peak hour. During the evening peak hour, the junction would operate within capacity. On that basis it is concluded that highway mitigation would be provided at this junction to mitigate the effect of the development at this junction. The proposed highway improvement scheme is detailed at Appendix F.
6.10 Table 6.3 shows the operation of the junction in the 2026 scenario with the proposed development of 60,000 square metres of office space and the proposed highway mitigation works in place. Model output files are attached at Appendix K.

| Approach | AM Peak |  | PM Peak |  |
| :--- | :---: | :---: | :---: | :---: |
|  | RFC | Queue (veh) | RFC | Queue (veh) |
| Middleton Stoney Road | 0.78 | 3.4 | 0.79 | 3.6 |
| Kings End | 0.91 | 9.4 | 0.66 | 1.9 |
| Oxford Road | 0.50 | 1.0 | 0.90 | 8.5 |

Table 6.3: Oxford Road/ Middleton Stoney Road/ Kings End - 2026 With Development Operation (including 60,000 square metres of office space) and mitigation
6.11 The result of the analysis demonstrate that the junction would operate within theoretical capacity (RFC less than 1) during both the morning and evening peak periods. The peak RFC in the 'With Development' scenario is 0.91 , less than the peak RFC of 0.94 in the baseline scenario. Furthermore, the peak queue reported in the in the 'With Development' scenario is less than the peak queue reported in the baseline scenario.
6.12 To this extent, proposed highway works provide a slight betterment to the operation of the junction, in comparison with the baseline operation of the junction. On that basis it is considered that the highway works mitigate for the effect of the development at this junction and no further mitigation works or assessment of this junction are considered necessary.

## A41 Highway Network

6.13 As part of the permitted development proposals for Bicester Village Phase 4 and the constructed Tesco store, a package of highway works is under construction covering the following junctions:

- Oxford Road / Pingle Drive roundabout;
- A41 Oxford Road / Oxford Road signalised roundabout (Esso roundabout);
- A41 Oxford Road (A41) / Lakeview Drive signalised junction;
- A41 Oxford Road (A41) / Pioneer Way signalised junction;
6.14 In addition, further highway improvements have been permitted at the A41 Oxford Road junctions with Pioneer Way and Lakeview Drive as part of the recently permitted development proposals at Bicester Gateway Retail Park (Planning Ref: 16/02505/OUT).
6.15 The operation of the above junctions has been assessed using the industry standard package for signal-controlled junctions, LinSig. In line with assessments undertaken from the permitted Bicester Village Phase 4, Tesco and Bicester Gateway Retail Park schemes the four junctions have been modelled within a single LinSig model. LinSig model parameters have been based on the most recently approved LinSig model for the Bicester Gateway Retail Park development and, as such, include the permitted highway works.
6.16 The traffic signal-controlled junctions on the Oxford Road corridor operate under Microprocessor Optimised Vehicle Actuation (MOVA). MOVA responds dynamically to variations in traffic flow and to this extent has a positive effect on the operation of the junctions, reducing the potential for underutilised green time at the junctions.
6.17 The LinSig modelling software is not able to model the benefit of adaptive traffic control such as MOVA as it assumes that signal timings remain fixed throughout the assessment period. Therefore, the results presented in this assessment will represent a worst-case scenario and, in reality, junction operation will be better due to the adaptive MOVA control already in place.
6.18 Table 6.4 shows the operation of the junctions in the 2026 baseline BTM (plus committed developments) scenario. Given the extent of model and the number of links, the below Table provides a summary of the operation of each junction and full link details for the A41/ Lakeview Drive junction. Full model output files are attached at Appendix K.

| Junction | AM Peak |  | PM Peak |  |
| :--- | :---: | :---: | :---: | :---: |
|  | DoS | MMQ | DoS | MMQ |
| Oxford Road/ Pingle Drive | $43.0 \%$ | - | $58.7 \%$ | - |
| Esso Roundabout | $91.2 \%$ | - | $100.3 \%$ | - |
| Oxford Road/ Pioneer Way | $75.1 \%$ | - | $90.8 \%$ | - |
| Oxford Road/ Lakeview Drive | $70.4 \%$ | - | $85.8 \%$ | - |
| Oxford Road n/b (Ahead) | $29.9 \%$ | 2 | $42.8 \%$ | 6 |
| Oxford Road n/b (Ahead) | $38.7 \%$ | 3 | $47.8 \%$ | 5 |
| Oxford Road n/b (Ahead/ Right) | $70.4 \%$ | 30 | $63.7 \%$ | 37 |
| Oxford Road s/b (Left) | $16.6 \%$ | 3 | $28.5 \%$ | 5 |
| Oxford Road s/b (Ahead) | $66.0 \%$ | 21 | $79.0 \%$ | 28 |
| Oxford Road s/b (Ahead) | $70.2 \%$ | 20 | $83.9 \%$ | 22 |
| Lakeview Drive (Left/ Right) | $44.4 \%$ | 7 | $85.8 \%$ | 21 |
| Lakeview Drive (Right) | $44.5 \%$ | 3 | $68.8 \%$ | 6 |
| Overall PRC |  | $-1.4 \%$ |  | $-11.5 \%$ |

Table 6.4: Oxford Road Corridor - 2026 Baseline plus Committed Operation
6.19 The analysis demonstrate that the junctions are expected with negative Practical Reserve Capacity during both the morning and evening peak periods in the 2026 baseline scenario.
6.20 Table 6.5 shows the operation of the junction in the 2026 baseline BTM (plus committed developments) scenario including the proposed 60,000 square metres of Office Park at the application site. Model output files are attached at Appendix K.

| Junction | AM Peak |  | PM Peak |  |  |  |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: |
|  | DoS | MMQ | DoS | MMQ |  |  |
| Oxford Road/ Pingle Drive | $50.5 \%$ | - | $66.9 \%$ | - |  |  |
| Esso Roundabout | $98.9 \%$ | - | $102.5 \%$ | - |  |  |
| Oxford Road/ Pioneer Way | $78.3 \%$ | - | $90.4 \%$ | - |  |  |
| Oxford Road/ Lakeview Drive | $93.7 \%$ | - | $107.6 \%$ | - |  |  |
| Oxford Road n/b (Ahead) | $49.3 \%$ | 6 | $95.0 \%$ | 45 |  |  |
| Oxford Road n/b (Ahead) | $63.5 \%$ | 27 | $96.5 \%$ | 48 |  |  |
| Oxford Road n/b (Ahead/Right) | $93.7 \%$ | 40 | $97.6 \%$ | 19 |  |  |
| Oxford Road s/b (Left) | $67.9 \%$ | 14 | $28.1 \%$ | 5 |  |  |
| Oxford Road s/b (Ahead) | $80.0 \%$ | 29 | $106.5 \%$ | 75 |  |  |
| Oxford Road s/b (Ahead) | $91.9 \%$ | 38 | $107.3 \%$ | 78 |  |  |
| Lakeview Drive (Left/ Right) | $61.3 \%$ | 7 | $107.6 \%$ | 83 |  |  |
| Lakeview Drive (Right) | $70.3 \%$ | 4 | $103.4 \%$ | 44 |  |  |
| Overall PRC |  |  |  |  |  |  |

Table 6.5: Oxford Road Corridor - 2026 Baseline Operation (including 60,000 square metres of office space)
6.21 The analysis shows that with the development in place, but no highway mitigation works, the junctions are expected to operate with negative Practical Reserve Capacity during both the morning and evening peak periods in the 2026 scenario. At the Lakeview Drive junction, in particular during the evening peak period, there is an increase in expected degree of saturation and queuing in comparison with the baseline scenario.
6.22 On that basis it is concluded that highway mitigation would be provided at this junction to mitigate the effect of the development at this junction. The proposed highway improvement scheme is detailed at Appendix F.
6.23 Table 6.6 shows the operation of the junctions along the Oxford Road corridor in the 2026 scenario with the proposed development of 60,000 square metres of office space and the proposed highway works in place. Model output files are attached at Appendix K.

| Junction | AM Peak |  | PM Peak |  |  |  |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: |
|  | DoS | MMQ | DoS | MMQ |  |  |
| Oxford Road/ Pingle Drive | $51.7 \%$ | - | $67.5 \%$ | - |  |  |
| Esso Roundabout | $90.4 \%$ | - | $94.5 \%$ | - |  |  |
| Oxford Road/ Pioneer Way | $75.1 \%$ | - | $91.8 \%$ | - |  |  |
| Oxford Road/ Lakeview Drive | $92.9 \%$ | - | $97.7 \%$ | - |  |  |
| Oxford Road n/b (Ahead) | $37.2 \%$ | 6 | $78.0 \%$ | 24 |  |  |
| Oxford Road n/b (Ahead) | $39.2 \%$ | 7 | $78.8 \%$ | 24 |  |  |
| Oxford Road n/b (Ahead) | $48.3 \%$ | 11 | $78.8 \%$ | 24 |  |  |
| Oxford Road n/b (Right) | $92.4 \%$ | 25 | $80.0 \%$ | 6 |  |  |
| Oxford Road s/b (Left/ Ahead) | $92.9 \%$ | 24 | $89.5 \%$ | 28 |  |  |
| Oxford Road s/b (Ahead) | $67.8 \%$ | 12 | $89.0 \%$ | 28 |  |  |
| Oxford Road s/b (Ahead) | $76.5 \%$ | 24 | $89.8 \%$ | 26 |  |  |
| Lakeview Drive (Left/ Right) | $34.0 \%$ | 6 | $97.7 \%$ | 42 |  |  |
| Lakeview Drive (Right) | $76.9 \%$ | 6 | $89.5 \%$ | 27 |  |  |
| Overall PRC |  |  |  |  |  |  |

Table 6.6: Oxford Road Corridor - 2026 With Development Operation (including 60,000 square metres of office space) and highway mitigation.
6.24 The results of the analysis demonstrate that with the development proposals and mitigation works in place the junction is expected to operate within theoretical capacity (DoS of less than $100 \%$ ) during both morning and evening peak periods. Some links are expected to operate with negative Practical Reserve Capacity with the proposed development in place, however, this is comparable with the baseline situation.
6.25 The Lakeview Drive arm of the junction would operate with negative Practical Reserve Capacity during the weekday evening peak period but it is highlighted that queuing on this arm would be accommodated on Lakeview within the site and off the public highway.
6.26 To this extent, the analysis demonstrates that the proposed highways works mitigate the effect of the development proposals a of the junction between the A41 Oxford Road during both peak periods. As such, it is concluded that, subject to the mitigation works identified, the development would not have a material effect on the operation of this junction and no further assessment or mitigation is considered necessary.
6.27 As identified above, the LinSig modelling software is not able to assess the benefit of MOVA traffic control that is already in place at the junctions. The results presented above therefore represent a worst-case scenario and, in reality, junction operation will be better due to the adaptive MOVA control.

A41 / Bicester Park \& Ride / Vendee Drive
6.28 The conventional roundabout junction between the A41, Vendee Drive and Bicester Park and Ride has been assessed using the industry standard software package for roundabout junctions, ARCADY.
6.29 The permitted development proposals at Bicester Gateway Business Park (Planning Ref: 16/02586/OUT) include highway improvement works to the A41, Vendee Drive junction. The operation of the junction has been modelled inclusive of the permitted junction improvements.
6.30 Table 6.7 shows the operation of the junction in the 2026 baseline BTM (plus committed developments) scenario. Model output files are attached at Appendix K.

| Approach | AM Peak |  | PM Peak |  |
| :--- | :---: | :---: | :---: | :---: |
|  | RFC | Queue (veh) | RFC | Queue (veh) |
| Vendee Drive | 0.22 | 0.3 | 0.25 | 0.3 |
| A41 (North) | 0.67 | 2.0 | 0.76 | 3.1 |
| Unnamed Road | 0.14 | 0.2 | 0.29 | 0.4 |
| A41 (South) | 0.66 | 1.9 | 0.77 | 3.3 |
| Bicester Park and Ride | 0.02 | 0.0 | 0.12 | 0.1 |

Table 6.7: A41/ Vendee Drive - 2026 Baseline Operation plus Committed Operation
6.31 The analysis demonstrates that the junction is expected to operate within theoretical capacity during both the morning and evening peak periods in the 2026 baseline scenario.
6.32 Table 6.8 below shows the operation of the junction in the 2026 with the proposed development of 60,000 square metres of office space in place. Model output files are attached at Appendix K.

| Approach | AM Peak |  | PM Peak |  |
| :--- | :---: | :---: | :---: | :---: |
|  | RFC | Queue (veh) | RFC | Queue (veh) |
| Vendee Drive | 0.35 | 0.5 | 0.26 | 0.4 |
| A41 (North) | 0.68 | 2.1 | 0.87 | 6.8 |
| Unnamed Road | 0.14 | 0.2 | 0.42 | 0.7 |
| A41 (South) | 0.74 | 2.9 | 0.81 | 4.1 |
| Bicester Park and Ride | 0.02 | 0.0 | 0.15 | 0.2 |

Table 6.8: A41/ Vendee Drive - 2026 With Development Operation (including 60,000 square metres of office space)
6.33 The analysis demonstrates that the junction is expected to operate within theoretical capacity (RFC of less than 1) during both the morning and evening peak periods in 2026 with the proposed development in place.
6.34 The change in both RFC and expected queuing between the baseline and 'With Development' scenario is not considered to be material and, on that basis, no further analysis of the A41/ Vendee Drive junction and no highway mitigation works are considered necessary.

A41/ A4421 - Rodney House Roundabout
6.35 The Rodney House roundabout is currently a conventional roundabout. As part of permitted development proposals at Graven Hill, highway improvement works are proposed at the Rodney House roundabout which include the signalisation of the junction. Officers at OCC have provided Motion with plans of the permitted highway works at the junction. Capacity modelling for the Rodney House roundabout has therefore been undertaken using the industry standard package for signal-controlled roundabouts, LinSig. Junction geometries and parameters have been based on the permitted highways works drawing provided by OCC.

Table 6.9 below shows the operation of the junction in the 2026 baseline BTM (plus committed developments) scenario. Model output files are attached at Appendix K.

| Approach | AM Peak |  | PM Peak |  |
| :--- | :---: | :---: | :---: | :---: |
|  | DoS | MMQ | DoS | MMQ |
| A41 (Left/ Ahead) | $67.8 \%$ | 7.9 | $82.7 \%$ | 17.7 |
| A41 (Ahead) | $8.4 \%$ | 1.1 | $20.2 \%$ | 2.9 |
| Graven Hill Road (Left) | $58.9 \%$ | 5.7 | $60.8 \%$ | 4.9 |
| Graven Hill Road (Ahead) | $46.9 \%$ | 4.1 | $36.1 \%$ | 2.7 |
| A41 (Left/ Ahead) | $55.1 \%$ | 5.9 | $69.5 \%$ | 9.4 |
| A41 (Ahead) | $38.2 \%$ | 5.7 | $44.3 \%$ | 7.3 |
| B4100 (Left/ Ahead) | $42.4 \%$ | 2.8 | $46.5 \%$ | 3.7 |
| B4100 (Ahead) | $46.5 \%$ | 3.1 | $60.3 \%$ | 5.1 |
| A4421 (Left/ Ahead) | $66.3 \%$ | 8.0 | $64.1 \%$ | 8.3 |
| A4421 (Ahead) | $45.3 \%$ | 5.1 | $35.5 \%$ | 4.1 |
| Overall PRC | $+32.7 \%$ |  |  |  |

Table 6.9: Rodney House Roundabout - 2026 Baseline Operation
6.37 The results of the analysis demonstrate that the junction is expected to operate within capacity during both the morning and evening peak periods.
6.38 Table 6.10 below shows the operation of the junction in the 2026 with the proposed development of 60,000 square metres of office space in place. Model output files are attached at Appendix K.

| Approach | AM Peak |  | PM Peak |  |  |  |  |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | DoS | MMQ | DoS | MMQ |  |  |  |
| A41 (Left/ Ahead) | $74.2 \%$ | 11.1 | $83.2 \%$ | 17.9 |  |  |  |
| A41 (Ahead) | $8.1 \%$ | 1.1 | $20.2 \%$ | 2.9 |  |  |  |
| Graven Hill Road (Left) | $62.3 \%$ | 5.9 | $60.8 \%$ | 4.9 |  |  |  |
| Graven Hill Road (Ahead) | $47.4 \%$ | 4.0 | $36.4 \%$ | 2.8 |  |  |  |
| A41 (Left/ Ahead) | $52.6 \%$ | 5.2 | $79.2 \%$ | 11.9 |  |  |  |
| A41 (Ahead) | $39.1 \%$ | 5.7 | $46.7 \%$ | 7.8 |  |  |  |
| B4100 (Left/ Ahead) | $29.2 \%$ | 2.5 | $46.5 \%$ | 3.7 |  |  |  |
| B4100 (Ahead) | $47.4 \%$ | 4.2 | $61.6 \%$ | 5.3 |  |  |  |
| A4421 (Left/ Ahead) | $58.9 \%$ | 7.5 | $64.6 \%$ | 8.4 |  |  |  |
| A4421 (Ahead) | $47.0 \%$ | 6.0 | $35.7 \%$ | 4.1 |  |  |  |
| Overall PRC | $+21.3 \%$ |  |  |  |  |  |  |

Table 6.10: Rodney House Roundabout - 2026 With Development Operation (including 60,000 square metres of office space)
6.39 The results of the analysis demonstrate that the junction is expected to operate within capacity during both the morning and evening peak periods in the 2026 with the proposed development in place. On that basis, it is evident that the proposed development would not have a material effect on the operation of this junction and no further assessment or mitigation measures is considered necessary.

## Summary

6.40 The effect of the development proposals on the local highway network has been assessed at the following junctions, as agreed with OCC:

- A41 Oxford Road / Lakeview Drive signalised junction;
- Oxford Road / A41 signalised roundabout;
- Oxford Road / Pingle Drive roundabout;
- Oxford Road / Middleton Stoney Road mini-roundabout;
- A41 Oxford Road / Pioneer Way signalised junction;
- A41 Oxford Road/ Vendee Drive; and
- A41/ A4421 Rodney House Roundabout.
6.41 The results of detailed junction capacity analysis demonstrate that, subject to the highway mitigation works identified at the junctions between A41 Oxford Road/ Lakeview Drive and at the junction between Oxford Road and Middleton Stoney Road, the development proposals would not result in a material effect in the operation of the highway network local to the site.
6.42 As such it concluded that the proposed highway works, as shown in drawings presented at Appendix F, are sufficient to mitigate the effect of the development on the local highway network. To this extent no further assessment, mitigation measures or Section 106 obligation towards further transport schemes, such as the South-Eastern Perimeter Rad (SEPR), are considered necessary or justified in planning terms.
6.43 The highway mitigation works presented at Appendix $F$, are to mitigate for the effect of traffic associated with the full development proposals of 60,000 square metres of $B 1(a) / B 1(b)$ office space.


### 7.0 Summary and Conclusions

7.1 Motion has been appointed by Scenic Land Developments Ltd to prepare this Transport Assessment in relation to development proposals on land to the east of the A41 Oxford Road, Bicester within the administrative boundary of CDC.
7.2 The site is currently undeveloped and is bound by the A41 Oxford Road to the west and Lakeview Drive to the north whilst Wyevale Garden Centre is located to the immediate south. The Bicester - Oxford railway line operates to the east and is separated from the site by undeveloped land.
7.3 Outline planning permission was granted in 2010 for the construction of a 60,000 -square metre office park comprising 53,000 square metres of $B 1(a) / B 1$ (b) office space and a 7,000-square metre C1 hotel, served by circa 1,837 car parking spaces (Planning Ref: 07/01106/OUT).
7.4 Detailed planning permission was subsequently granted in November 2013 for the construction of a Tesco food store of 8,135 square metres and petrol filling station on part of the permitted office park site (Planning Ref: 12/01193/F). That planning application was supported by a Transport Assessment which considered the effect of the development proposals on the highway network local to the site. The Tesco store has since been constructed and opened in April 2016.
7.5 The S106 Deed of Variation in relation to the permitted Tesco store and office park allows for the construction of up to 45,000 square metres of the $B 1(a) / B 1(b)$ office space being delivered on the remainder of the site, as part of the previous outline planning permission for an office park.
7.6 The current planning application seeks outline planning permission, with all matters reserved except access, for the development of up to 60,000 square metres (GEA) of B1(a)/B1(b) office space. Vehicle access to the site would be via the two existing roundabout junctions on Lakeview Drive.
7.7 The current development proposals would supersede and replace the previous outline permission for an office park on the site. In comparison with the previous outline planning permission for an office park on the site, the current site area excludes the portion of the site, north of Lakeview Drive, which has since been developed for a Tesco store. However, the site area now includes a parcel of land along the frontage of the A41 Oxford Road, south of Lakeview Drive, which was previously not within the applicant's ownership and was not part of the previous outline planning permission for an office park.
7.8 Following submission of the planning application comments have been received from OCC in relation to both the Transport Assessment and Framework Travel Plan and, as such, updated versions of those documents have been prepared to address comments received.
7.9 This Updated Transport Assessment has been prepared in accordance with national and local guidance and has considered the highways and transport matters associated with the current development proposals and, in particular, the effect of the development proposals on the highway network local to the site.
7.10 This Updated Transport Assessment demonstrates that:

- The application site is accessible by foot, cycle and by public transport;
- The application site is allocated under Bicester Policy 4 of the Cherwell Local Plan for development of a high-quality office park;
- Outline planning permission was previously granted in 2010 for the construction of a 60,000-square metre office park comprising 53,000 square metres of $B 1(a) / B 1(b)$ office space and a 7,000-square metre C1 hotel, served by circa 1,837 car parking spaces (Planning Ref: 07/01106/OUT);
- The development proposals will be accessed from Lakeview Drive via two existing roundabout junctions;
- Car parking and cycle parking will be provided in accordance with local parking standards;
- The proposed highway works include the provision of a new southbound bus stop on Oxford Road adjacent to the application site and a widened foot/ cycleway on Oxford Road between Lakeview Drive and Pioneer Way;
- The effect of the development proposals on the local highway network has been assessed based on parameters agreed with OCC;
- Highway mitigation works have been identified at the junction between the A41 Oxford Road/ Lakeview drive and at the junction between Oxford Road/ Middleton Stoney Road;
- The proposed highway mitigation works have been subject to a Stage 1 Road Safety Audit which has not raised any material concerns with the proposed work and any comments have been addressed in the drawings provided and the Designers Response;
- The results of detailed junction capacity analysis demonstrate that, subject to highway mitigation works identified, the development proposals would not result in a material effect in the operation of the highway network local to the site. As such further mitigation measures or obligation towards further transport schemes is not considered necessary or justified in planning terms;
- The highway mitigation works proposed should only need to be implemented when the provision office space at the application site exceeds a threshold of 45,000 square metres as previously permitted at the application site; and,
- A Framework Workplace Travel Plan has been developed in order to promote sustainable travel choices amongst staff and visitors to the proposed development and is submitted under separate cover.
7.11 On the basis of the above it is concluded that the development proposals will not result in a material effect on the operation of the highway network local to the site. The development proposals are in accordance with national and local transport related planning policy and, as such, should not be resisted on highways or transportation grounds.


## motion

Figures


Legend:
Bicester Office Park,
Bicester


## Legend:

----Local Footpath

Site Location

1 Wretchwick Green

2 Kingsmere

3 Bicester Village

4 Bicester town centre

5 Graven Hill

6 Bicester Park and Ride

Bicester Office Park
Bicester
Figure 3.2 Local Footpaths
Not to Scale
motion


## Legend

-=-=-Local Signed Cycle Route $=-=-$ National Cycle Network R5

## Site Location

1 Wretchwick Green

2 Kingsmere

3 Bicester Village

4 Bicester town centre

5 Graven Hill

6 Bicester Park and Ride

Bicester Office Park
Bicester
Figure 3.3 Local Cycle Routes
Not to Scale
motion


## Legend

-= 8 - Middle Barton to Bicester (daily, Fridays only)

- 26 - Bicester to Kingsmere (every 30 minutes)
--'NS5 - Oxford to Gosford to Bicester (every 30 minutes)

S5 - Oxford to Gosford to Bicester (every 25 minutes)

X5 - Cambridge to Bedford to Oxford (every 30 minutes)

- Existing Bus Stops

O Proposed Bus Stops

## 1 Wretchwick Green

2 Kingsmere
3 Bicester Village

4 Bicester town centre

5 Graven Hill

6 Bicester Park and Ride
$\gtrsim$ Station
Bicester Office Park

Bicester Office Park

Figure 3.4-Local Bus Services Not to Scale
motion

motion






motion


motion
Bicseter Business Park





motion
Bicseter Business Park
Figure 5.14: 2026 Baseline plus Committed Developments plus 60,000 office Park Traffic Flows - PM Peak

## motion

## Appendix A

Oxfordshire County Council Pre-Application Response

## District: Cherwell

## Application No: 17/CH0005/PREAPP

Proposal: The construction of an office park providing up to 57,000 square metres of B1 office space.
Location: Bicester Office Park. Land To South And East Of The A41 Oxford Road, Bicester, Oxfordshire

## Transport

Oxfordshire County Council is a consultee of the local planning authority and provides advice on the likely transport and highways impact of development where necessary.

It should be noted that the advice below represents the informal opinion of an officer of the council only, which is given entirely without prejudice to the formal consideration of any planning application, which may be submitted. Nevertheless the comments are given in good faith and fairly reflect an opinion at the time of drafting given the information submitted.

## Key issues:

- Strategic contribution towards the South Eastern Perimeter Road


## Legal agreement required to secure:

If a planning application were to be submitted and approved a S278 would be required to deliver any highway improvements that it was decided would be needed to make the development acceptable e.g. new site access junction, footway improvements.

A new S106 agreement would be needed to secure the S278 works and also a financial contribution towards
(i) Public transport improvements and
(ii) Strategic contribution towards the delivery of the South East Link Roadrequired to mitigate the development's impact on the A41 junctions

Travel Plan monitoring fees shall be required

## Informatives:

Please note the Advance Payments Code (APC), Sections 219-225 of the Highways Act, is in force in the county to ensure financial security from the developer to off-set the frontage owners' liability for private street works, typically in the form of a cash deposit or bond. Should a developer wish for a street or estate to remain private then to secure exemption from the APC procedure a 'Private Road Agreement' must be
entered into with the County Council to protect the interests of prospective frontage owners. For guidance and information on road adoptions etc. please email the County's Road Agreements Team at roadagreements@oxfordshire.gov.uk

## Detailed comments:

The A41 from which the site is accessed is heavily trafficked and will be put under further pressure from Cherwell Local Plan growth allocations, including the allocation on this site (Bicester 4).

This was recognised by Bicester Village in their application for Phase 4 of their development, where they are now delivering major highway improvements at and between the Esso roundabout and Pingle Drive junctions, having also provided a Bicester Park and Ride facility.

The highway works which are currently underway on the A41 (and related to the expansion of Bicester Village) will deliver a new bus layby on the northbound side of the A41. The highway works which are related to the construction and use of the permitted Bicester Business Park would, once they are triggered (i.e. once construction begins), also provide a northbound and southbound bus layby. Clearly as the Bicester Village works are already underway, once construction of any permission granted for the business park begins, its corresponding remaining liability would be to provide the southbound layby (as the northbound will have by then been delivered).

## Scoping Note

Having had a chance to look at the Scoping Note dated 19 ${ }^{\text {th }}$ April 2017 for a Transport Assessment, I wish to make the following comments.

## Policy Consideration

Various Policies that should be considered relevant to this development are:

## National Policies

- National Planning Policy Framework (NPPF)
- National Planning Practice Guidance (NPPG)

Local Policy Context include

- Connecting Oxfordshire 2015-2031 (LTP4)
- The Cherwell Local Plan (Adopted July 2015) from which the Policy Bicester 4 requires;
- Layout that enables a high degree of integration and connectivity between new and existing development particularly the mixed use urban extension at South West Bicester to the west, the garden centre to the south, and, to the north, Bicester town centre and Bicester Village retail outlet
- Provision for safe pedestrian access from the A41 including facilitating the crossing of the A41 to the north and west, and the provision and upgrading of footpaths and cycleways that link to existing networks to improve connectivity generally and to develop
links between this site, nearby development sites and the town centre.
- Good accessibility to public transport services should be provided for, including the accommodation of new bus stops to link the development to the wider town.
- A Transport Assessment and Travel Plan to accompany development proposals.


## Area of Impact and Junction Modelling

The scoping note accompanying this pre-application enquiry proposes to consider the following junctions for assessment

- Oxford Road / Pingle Drive Roundabout
- Oxford Road / A41 signalised roundabout
- Site Access (Oxford Road / A41 Lakeview Drive signalised junction)
- Oxford Road (A41) / Kingsmere signalised junction.

As previously mentioned in our telephone conversation on $26^{\text {th }}$ April, in addition to the above junctions, the Transport Assessment will need to look at a wider study area to include;

- A41 / Vendee Drive / Oxford Road (A41) roundabout and
- Oxford Road / Middleton Stoney Road / Kings End roundabout
- Rodney House roundabout junction.

These junctions further afield are critical, likely to be impacted by the whole of Bicester 10 when it comes forward and Bicester 4 and the TA shall be expected to carry out capacity tests demonstrating the effect of the development on the highway network.

The scoping note under section 4.4 mentions that traffic surveys shall be undertaken during a weekday morning and evening peak period. The weekend peaks on the A41 approaching Bicester are very high. Owing to the adjacent land use particularly Bicester Village and Tesco superstore, in terms of the effect of the proposal on traffic at the Saturday and Sunday peak times, it would add to the already high volume of retail development traffic in the area. I would like to see further justification of not including a weekend assessment.

## Future Years

Paragraph 4.5 of the Scoping Note sets a future year assessment to the fifth year after submission of the Transport Assessment - which puts it down to 2022. In my view, I feel this period should be extended to cover 2026 in line with the Bicester Transport Model which includes 2024 interim year and also includes the committed development expected to come forward at that time. We would like this to be the forecast year rather than 2022.

Committed development - Use of the Bicester Transport Model 2026 would include all development expected to come forward by that time. Consideration also needs to be given to two pending planning applications close by to the site, which are both proposing highway mitigation works along the A41. These are;

- 16-02505-OUT - Bicester Gateway (Kingsmere Retail)
- 16-02586-OUT - Bicester Gateway (Bicester 10)

The model includes significant committed developments expected to come forward and including the growth trips. Should the model be used, TEMPRO shall not be required in this case.

We shall however like to see the network tested using the flows from the model.

## Trip Generation

The scoping note accompanying this pre-application enquiry proposes to use TRICS database to establish an estimate of the number of vehicles that the proposed development might generate when it is fully occupied.

I appreciate that the scoping note submitted attempts to estimate the likely number of trips generated that shall be generated by the development. However, the trip rates used appear rather low especially in the PM peak. I would further appreciate that a trip rates commensurate to the developments close by to be considered, such as ones used in planning ref: 16-02586-OUT.

Characteristics of business parks are likely to have very high levels of car use and very peaky demand for travel. The Oxford Business Park (Garsington Road) certainly displays these characteristics, which results in very long queues and delays when employees decide to leave at the same time (at 1705, for example). Arguably, similar characteristics could be expected on this site, especially when combined with the late Friday afternoon flow from the Tesco store. Will these characteristics be reflected in a TA - what mitigation can be provided - to spread the peak for example.

## Other scoping matters

Public Transport - The applicant will need to robustly assess public transport accessibility between the development site and the wider network. The original application included a requirement to provide a pair of bus stops on the A41 and an agreement to provide some S106 funding to provide a bus service into the site.

The bus stops have not been fully delivered, with a new bus stop having recently been installed on the western side of the A41, to the north of the Premier Inn hotel. I guess the bus stop on the eastern side of the A41 is tied up with the Bicester Business Park Legal Agreement. In any event, it is absolutely essential that this is provided.

That being said, the walking distance to these bus stops along the A41 from some of these workplace units could be around 750 metres. I would like to see how the applicant addresses the distance in the TA.

## South Eastern Perimeter Road (SEPR)

The Local Transport Plan 4 Bicester Area Strategy proposes a South East Perimeter Road in Bicester, which will ease congestion on the A41 and also mitigate the development's impact on the A41 junctions. It is partly funded, but currently requires contributions to fund the western section proposed, so contributions towards this are likely to be a consideration in terms of mitigating the Bicester Business Park
proposals. Other future developments in the area would also be expected to contribute.

The cumulative impact of development in Bicester will be severe if appropriate contributions are not secured from all development sites towards the strategic transport infrastructure required to mitigate the increased transport movements.

Strategic transport modelling demonstrates the benefits that the SEPR will bring to the A41 (Oxford Road):

- The A41 Oxford Road is a key corridor in Bicester where junctions along its length are impacted significantly as a result of the growth of Bicester, including Bicester 10. The Application Site is estimated to increase the proportion of peak hour traffic at the A41/ Vendee Drive junction by between $7 \%$ and 8\% in 2024.
- The SEPR has been identified as a key piece of strategic infrastructure that will bring direct relief to the A41 corridor, thereby facilitating improved operation of junctions directly impacted by Bicester 10.
- Modelling has demonstrated the benefits that the SEPR would bring to the A41. In the AM peak:
- Over 1000 vehicles (pcu's) that would otherwise use the A41 Oxford Rd northbound through Vendee Dve would route via SEPR (eastbound)
- Around 930 vehicles (pcu's) that would otherwise use A41 Boundary Way and turn left on A41 Oxford Rd southbound past Bic 10, would route via SEPR (westbound)
- Therefore, over 1930 vehicles (pcu's) would use the SEPR that would otherwise route along A41 past the Bicester 10 site.

It is acknowledged however, that the capacity released on the A41 by the SEPR will itself encourage some traffic that might otherwise choose NOT to use the A41, to divert along the corridor. When taking diverted traffic into account, the net reduction in traffic on the A41 in the vicinity of the Bicester 10 site would be around 1130 pcu's.

## Car parking

Sufficient car parking will need to be provided to ensure that there is no overspill onto surrounding roads or inappropriate use of the Park and Ride site. Designs and provision should take into account areas within the development that may be subject to inappropriate parking such as on green verge areas or turning heads. OCC requires $2.4 \mathrm{~m} \times 4.8 \mathrm{~m}$ parking bays and 6 m width of manoeuvrable space between parking rows. OCC parking standards for B1 Office developments also require 1 parking space per 30sqm GFA, to include about 6\% of DDA per development unit.

Consideration of the interaction of car parking with other sites in the area e.g. acting as an overspill car parking area for Bicester Village (rather than Bicester Village visitors using the $\mathrm{P} \& \mathrm{R}$ ) must also be made. A robust car parking management plan should be included in the Travel Plan.

## Cycle parking

The county's cycle parking standards sets out how developers should provide sufficient secure and covered cycle parking for staff and visitors. Cycle parking should be easy to locate and as close to the buildings as possible, not only to make it as attractive to potential users as possible but also to allow natural surveillance from the building itself.

## Drainage

A surface water drainage scheme for the site will need to be submitted with a planning application. This will be based on sustainable drainage principles and an assessment of the hydrological and hydro-geological context of the development, The scheme will need to include:

- Discharge Rates
- Discharge Volumes
- Maintenance and management of SUDS features (including details of who will be responsible maintaining the SUDS \& landowner details)
- Sizing of features - attenuation volume
- Infiltration tests to be undertaken in accordance with BRE365
- Detailed drainage layout with pipe numbers (to include direction of flow)
- SUDS (list the suds features mentioned within the FRA to ensure they are carried forward into the detailed drainage strategy)
- Network drainage calculations (to prove that the proposals will work)
- Phasing plans
- Flood Risk Assessment


## Travel Plan

A Travel Plan Statement meeting the requirements set out in the Oxfordshire County Council guidance document, Transport for New Developments; Transport
Assessments and Travel Plans will be required for this application. It would need to be produced and agreed prior to first occupation.

Additionally, a Travel Information Pack would need to be submitted to and approved by the Local Planning Authority prior to first occupation. The first occupants of each development unit shall be provided with a copy of the approved Travel Information Pack.

Officer's Name: Rashid Bbosa
Officer's Title: Transport Engineer
Date: 09 May 2017

## motion

## Appendix B

Oxfordshire County Council Planning Application Response

# COUNTY COUNCIL'S RESPONSE TO CONSULTATION ON THE FOLLOWING DEVELOPMENT PROPOSAL 

District: Cherwell<br>Application No: 17/02534/OUT<br>Proposal: OUTLINE - The construction of a business park of up to 60,000 sq.m (GEA) of flexible Class B1(a) office / Class B1(b) research \& development floorspace; parking for up to 2,000 cars; and associated highways, infrastructure and earthworks<br>Location: Land North Of Bicester Avenue Garden Centre Oxford Road Bicester

Response date: $\mathbf{2 7}^{\text {th }}$ February 2018

This report sets out the officer views of Oxfordshire County Council (OCC) on the above proposal. These are set out by individual service area/technical discipline and include details of any planning conditions or informatives that should be attached in the event that permission is granted and any obligations to be secured by way of a S106 agreement. Where considered appropriate, an overarching strategic commentary is also included. If the local County Council member has provided comments on the application these are provided as a separate attachment.

## Assessment Criteria Proposal overview and mix /population generation

OCC's response is based on a development as set out in the table below. The development is taken from the application form

| Commercial - use class | $\mathrm{m}^{2}$ |
| :--- | :---: |
| B1 | 58,200 |

## Strategic Comments

This application covers the majority of the Local Plan allocation site Bicester 4: Bicester Business Park. The application site covers a modified area to that consented for office use in 2010 (07/01106/OUT), in part due to the implementation of the Tesco site to the north (12/01193/F).

Whilst the principle of the development with $\mathrm{B} 1(\mathrm{a})$ office / B1(b) research \& development floorspace is supported, there are a number of issues with the current planning submission as outlined below.

Transport Development Control object for the following reasons:

- The Transport Assessment has not given adequate information about the traffic impact on the local network, in particular key committed development traffic is omitted;
- The proposed highway works are not considered safe and sufficient to mitigate the possible impact of the development;
- The drainage information submitted is insufficient.

There is also an archaeology objection because the site is located in an area of archaeological interest and the results of an archaeological evaluation are required prior to determination of this application.

Any new Section 106 or Deed of Variation agreed for this development site will need to maintain the remaining contributions in the existing S106 associated with permission 07/01106/OUT (as varied in November 2013) proportionately to the scale of new development.

Further details are provided in the officer responses below.

Officer's Name: David Flavin

Officer's Title: Senior Planning Officer
Date: $27^{\text {th }}$ February 2018

## General Information and Advice

## Recommendations for approval contrary to OCC objection:

IF within this response an OCC officer has raised an objection but the Local Planning Authority are still minded to recommend approval, OCC would be grateful for notification (via planningconsultations@oxfordshire.gov.uk) as to why material consideration outweigh OCC's objections, and given an opportunity to make further representations.

## Outline applications and contributions

The number and type of dwellings and/or the floor space may be set by the developer at the time of application, or if not stated in the application, a policy compliant mix will be used for assessment of the impact and mitigation in the form of s106 contributions. These are set out on the first page of this response.

In the case of outline applications, once the unit mix/floor space is confirmed by the developer a matrix (if appropriate) will be applied to assess any increase in contributions payable. The matrix will be based on an assumed policy compliant mix as if not agreed during the s106 negotiations.

Where unit mix is established prior to commencement of development, the matrix sum can be fixed based on the supplied mix (with scope for higher contribution if there is a revised reserved matters approval).

## Where a S106/Planning Obligation is required:

> Index Linked - in order to maintain the real value of s106 contributions, contributions will be index linked. Base values and the index to be applied are set out in the Schedules to this response.
$>$ Security of payment for deferred contributions - An approved bond will be required to secure payments where the payment of S106 contributions (in aggregate) have been agreed to be deferred to post implementation and the total County contributions for the development exceed £1m (after indexation).
> Administration and Monitoring Fee
An administration and monitoring fee will be required to cover the extra monitoring and administration associated with the S106 agreement. The final amount will be based on the OCC's scale of fees and will adjusted to take account of the number of obligations and the complexity of the S106 agreement.
> OCC Legal Fees The applicant will be required to pay OCC's legal fees in relation to legal agreements. Please note the fees apply whether an s106 agreement is completed or not.

## CIL Regulation 123

Due to pooling constraints for local authorities set out in Regulation 123 of the Community Infrastructure Levy Regulations 2010 (as amended), OCC may choose not to seek contributions set out in this response during the s106 drafting and negotiation.

That decision is taken either because:

- OCC considers that to do so it would breach the limit of 5 obligations to that infrastructure type or that infrastructure project or
- OCC considers that it is appropriate to reserve the ability to seek contributions to that infrastructure type or that infrastructure project in relation to the impacts of another proposal.

The district planning authority should however, take into account the whole impact of the proposed development on the county infrastructure, and the lack of mitigation in making its decision.

## Transport Schedule

## Recommendation:

Objection (for the following reasons):

- The Transport Assessment has not given adequate information about the traffic impact on the local network, in particular key committed development traffic is omitted.
- The proposed highway works are not considered safe and sufficient to mitigate the possible impact of the development
- The drainage information submitted is insufficient

If despite OCC's objection permission is proposed to be granted then OCC requires prior to the issuing of planning permission a S106 agreement including an obligation to enter into a S278 agreement to mitigate the impact of the development plus planning conditions as detailed below.

S106 Contributions

| Contribution | Amount £ | Price base | Index | Towards (details) |
| :--- | :--- | :--- | :--- | :--- |
| Highway <br> infrastructure | TBC | TBC | Baxter | The South East <br> Perimeter Road or <br> scheme of similar <br> benefit. |
| Strategic rail <br> contribution | TBC | TBC | TBC | Rail improvements <br> between Bicester and <br> Milton Keynes |
| Public transport <br> services | TBC | TBC | RPI-x | Provision of a bus <br> service linking the <br> development with <br> Bicester Town <br> Centre/station |
| Public transport <br> infrastructure (if <br> not dealt with <br> under S278/S38 <br> agreement) | TBC |  | Baxter | Provision of bus stop <br> infrastructure within <br> the site and on Oxford <br> Road. |
| Travel Plan <br> Monitoring | $£ 2,040$ | January <br> 2018 | RPI-x | Monitoring and review <br> of travel plan |
| Total | TBC |  |  |  |

## Comments:

## Highway Capacity Assessment

## Trip generation

The proposed trip generation is considered to be sufficiently robust.

## Committed Development

As part of the pre-application discussions, it was recommended that any assessment of the highway network be carried out as per the Bicester Transport Model (BTM) traffic flows.

The Transport Assessment presented as part of this application has however left out a vital committed development which did not form part of the BTM and neither was its proposal envisaged at the time pre-application scoping discussions were held.

An application for the development of a two-storey drive-through restaurant (class A3/A5) including car park has recently been permitted adjacent to the Tesco filling station (Planning Ref: 17/00889/F). Although relatively small in scale (to the developments around) it is expected to have a significant degree of impact on the operation of the local network. Also not included in either the BTM or this TA is the recently approved development (Kingsmere Retail) on land adjoining Pioneer Way and A41/Oxford Road (planning Ref: 16/02505/OUT). Both of these, by virtue of their proximity to the proposed development cannot be ignored in any traffic impact assessment. (Reason for objection)

Further to that, I am unconvinced by the applicant's approach of assessing the development impact on the local network. I would have expected a robust assessment to show the impact on the local network with and without the traffic from the complete proposed development.

The Transport Assessment should be revisited, and the following should be noted regarding the Bicester Transport Model 2026 scenario:

- Kingsmere Retail and the drive through restaurant mentioned above are NOT included (see above)
- Bicester 4 office development is NOT included
- Tesco IS included


## Trip distribution

The distribution of development traffic on the local road network has been done based on Travel to Work Census data, from the MSOA Cherwell 015 output area. Paragraph 5.17 of the TA gives the expected distribution of vehicle trips in reference to the travel to work census data. However, since the 2011 Census, housing development has taken place at Kingsmere, which could affect the distribution, which could have an impact on the distribution. It is noted that the TA does not include the census data tables. (Reason for objection)

## Trip assignment

The assumptions regarding trip assignment are not provided in the TA (Reason for objection)

## Junction modelling and mitigation

Section 6.0 of the TA presents highway capacity assessments undertaken to inform of the likely impacts of the development on the network, together with a proposed highway mitigation strategy. Junction analysis carried out utilising the industry standard modelling packages for each type of junction demonstrated that some shall operate within the designed capacity. However, the A41/Oxford Road junction and the Middleton Stoney Road/Oxford Road/Kings End junction showed that junctions would operate over and above the theoretical capacity in the 2026 baseline scenario.

The A41 Oxford Road/Lakeview Drive mitigation measures do not appear to fully alleviate the development impact. Oxford Road NB right AM and SB left/ahead in the AM and PM at the junction are left at or over capacity. Lakeview Drive left/right is brought over capacity in the PM. There is also a residual large impact on some of the movements. At the Vendee Drive/A41 Oxford Road junction, the A41 in the PM peak is at capacity and worsened further by the development. (Reason for objection) However, it should be noted that these capacity assessments may vary when the TA is revised.

## A41 Oxford Road

The site is accessed from Lakeview Drive via the signal controlled junction with the A41 Oxford Road. The A41/Oxford Road section between Vendee Drive and the Middleton Stoney Road junction comprises of sets of traffic signals which have been modelled using LINSIG. Model results/output files have been attached as Appendix F. However, corresponding .Isg files have not been submitted to enable us to thoroughly check the validity of the modelling work. Without the .Isg files, model parameters such as road geometry and input flows cannot be fully assessed. Until this information is submitted, the modelling results cannot be relied on. (Reason for objection)

The highway mitigation arrangement proposed in Drawing 170211-02 presents a number of design issues that I consider would increase safety risk on what is already a very busy section of A41 Oxford Road. Observed notably are;

- The scheme proposes to include an additional right turning lane from A41 Oxford Road into Lakeview Drive. Lakeview Drive currently has a single flowin lane from the northbound A41 traffic. Although the scheme attempts to create an additional flow-in lane at its mouth, lane continuity is unclear. The presence of the triangular island pedestrian refuge between the A41 Oxford Road (SB) and Lakeview Drive prevents a balance between the exit and entry lanes. The number of straight ahead entry and exit lanes for a traffic stream should be balanced to reduce conflict caused by traffic merging or diverging within the junction intervisibility zone. Where it is necessary to reduce the number of lanes on the exit arm this should be carried out beyond the junction intervisibility zone, over 100 metres for a single lane reduction, measured from the limit of the junction intervisibility zone, according to Figure 2/11 of Design Manual for Roads and Bridges (Vol 6 Sec 2, Part 3 TD/04). (Reason for objection)
- No vehicle tracking has been provided to demonstrate safe passage of the vehicles particularly on the turn in and out of Lakeview Drive. With the significant narrowing of carriageway lanes along the A41 Oxford Road and
bearing in mind that Lakeview Drive also serves as access to Tesco deliveries, the application must demonstrate by tracking analysis that gives consideration to long and articulate vehicles besides cars that they can reasonably use the junction. (Reason for objection)
- The current A41 Oxford Road layout requires some motorists to change lanes over very short distances. With the development proposing to add lanes on top of what is in existence, that leaves me concerned that this would likely lead to increased conflicts in the immediate vicinity of the development. In the event that the proposed highway works requiring carriageway widening along the A41/Oxford Road are agreed, these should be carried as per OCC specifications. We would require the surface course in the adjacent area / lane to be replaced with a stepped joint in the layers below as illustrated in drawing HSD 700/025 via https://www.oxfordshire.gov.uk/cms/content/highway-standard-details
- The mitigation layout plan is not scaled to enable a comprehensive review of the dimensions of the proposed highway. (Reason for objection)
- Triangular island pedestrian refuge - Pedestrian refuges and traffic islands help pedestrians by enabling them to deal with one lane or direction of traffic at a time. This appears to be significantly reduced, and I would like to be certain that this is deep enough to accommodate a reasonable number of pedestrians including bicycles and/or a wheelchair - noticing that this is a busy intersection likely to be used by platoons of pedestrians such as at peak times.
The central reservation - The proposed scheme also intends to significantly reduce the width of the central reservation along the southern arm of the A41 Oxford Road. In its existing layout the A41 Oxford Road ranges between $4.2 \mathrm{~m}-6 \mathrm{~m}$ in width. A further reduction in width shall likely make it impossible for erection of the associated street furniture such as signage, lamp posts and traffic signal posts. These structures are accommodated within highway land between/adjacent to carriageways and it is required that any placement of such posts should be clear by 0.45 m from any face of the kerb. The proposed layout does not appear to have given consideration for this. It goes without need to say that such a busy section of highway shall require significant signage and traffic signal heads to give information to motorists and control traffic respectively.

Such significant highway changes need to be accompanied by a Stage 1 Safety Audit as part of the application.

## Oxford Road / Middleton Stoney Road / Kings End

This is a mini-roundabout that has been modelled using ARCADY and model output/results for the existing junction operation show that Kings End approach operates over the theoretical capacity in the AM peak period while Middleton Stoney and Oxford Road approaches operate over the recommended RFC threshold in the PM peak period.

The application proposes to make improvements to this mini-roundabout as mitigation which would involve increasing the entry width from the Kings End approach. (Drawing No. 170211-04). The modelling predicts an improvement in the operation of the Kings End approach in the AM peak period in 2016. However, the same model shows a general deterioration in RFC values for all other approach arms in both peak periods. The Middleton Stoney Road and Oxford Road are seen to have rising RFC values, reading just below the recommended threshold.

## Public Transport

Discussions during the original Bicester Business Park application identified the need for a bus service to this site, as some parts of the site are more than 400 metres from bus stops on the A41 Oxford Road. The amount 'agreed' in discussion was subsumed into an overall sum for the site. The understanding was to allow for a new bus service to enter the site, which would also require a new bus stop on Lakeview Drive. This service would link Bicester North to Bicester Village along Pingle Drive, London Road and into the site.

It is likely that the bus would not need to run all day, but would be needed in the peaks and lunchtimes, as at Milton Park, a similar development in Oxfordshire. Most of the cost of a bus is in the need for a 'peak vehicle'. It's probable that this service would be operated in conjunction with another service, but we still need the 'peak bus'.

As part of the Bicester Gateway Retail Park planning consent it was proposed to include a southbound bus stop adjacent to the site along the A41 Oxford Road as part of the highway improvements. However, to allow for the possibility that that consent is not implemented either wholly or earlier than this proposed development, then we will require a commitment from this development to install the same bus stop with associated infrastructure.

## Pedestrian / Cycle routes

The design and access statement mentions that bicycle routes are linked into the scheme. Apart from the pedestrian and vehicular accesses, the application has not demonstrated a direct connectivity to any dedicated cycle routes. National Cycle Route 52 abuts the site along its boundary with the A41 Oxford Road. This cycle route provides a cycling infrastructure connecting south towards Wendlebury, Kidlington and Oxford. North of the application site, the route connects to Bicester Village and Bicester Town Centre.

Much as am convinced that the development is well placed for future employees and visitors to utilise this route to and from the site by foot and/or cycle, I am concerned by its width for a shared pedestrian/cycle route. On the merge to the A41/Oxford Road from Lakeview Drive, the shared infrastructure is seen to considerably narrow in width as it runs past the proposed bus stop layby. We would like to see a 3metre provision being extended further south right up to the pedestrian crossing that leads to Pioneer Way. The need for such an improvement is in part driven by the growth of the town and the need to link residential areas to employments. This is a pedestrian/cycle desire line into the wider Kingsmere residential development via Pioneer Way from the site which must be improved. (To be incorporated into s278)

We would like the development to provide a pedestrian/cycle only access onto the A41, along its western boundary. This connection should be informed by the pedestrian desire line that aims to enhance pedestrian connectivity and reduce walking distances. It is also thought that this would enhance multimodal travel for visitors and staff arriving by public transport from the bus stop. Rather than walking along Lakeview Drive pedestrians crossing the A41 Oxford Road from Kingsmere, and from areas south such as the Bicester Park and Ride and Wendlebury would directly access the development via this access.

## Parking Strategy

The parking strategy that the TA presents is informed by OCC maximum parking level standards. Although the application sets to provide the maximum provision for the scale of the proposed development, there needs to be a careful balance between meeting the demand for parking without unduly encouraging car use, particularly given the potential for sustainable travel to the site. I strongly recommend that the level of parking provision be supported by a parking accumulation study.

That being said, it is important that the development does not lead to overspill street parking. It is thus important that the Framework Travel Plan sets off with robust measures that promote multi modal travel choices.

Consideration of the interaction of car parking with other sites in the area e.g. acting as an overspill car parking area for Bicester Village (rather than Bicester Village visitors using the $\mathrm{P} \& \mathrm{R}$ ) has not been made. A robust car parking management plan should be included.

## Personal Injury Accident Data

The TA has presented Personal Injury Accident data of reported collisions in the immediate vicinity of the site obtained from Thames Valley Police (TVP). This data reportedly covers the period between 01/07/2012 and 01/07/2017 and is appended to the TA. Although am not questioning the presented dataset, I would like clarification on how the applicant managed to retrieve information such as how a particular accident was linked to the causation factor and its location from the TVP report. Unless there is more to that report than presented here, I am not convinced by how the applicant has reached this conclusion. Could this please be clarified.

Further review of the personal injury accident data held by OCC has revealed 5 injury collisions. These incidents occurred between Pioneer Way and Lakeview Drive junctions with A41 Oxford Road (excluding Bicester Avenue and Esso Roundabout).

Three out of five of these incidents involved vehicles making right manoeuvres either into Pioneer Way or Lakeview Drive. Although these recorded incidents are of the slight category I am concerned that any additional lanes created would increase the likelihood of conflicts during lane changing manoeuvres. Should you require more detailed information on the most recent Personal Injury Accident data, please contac79t our Road Safety Officer on Anthony.Kirkwood@Oxfordshire.gov.uk

## Transport Strategy

Policy Bicester 4 of the Cherwell Local Plan relating to the site requires:

- Layout that enables a high degree of integration and connectivity between new and existing development particularly the mixed use urban extension at South West Bicester to the west, the garden centre to the south, and, to the north, Bicester town centre and Bicester Village retail outlet.
- Provision for safe pedestrian access from the A41 including facilitating the crossing of the A41 to the north and west, and the provision and upgrading of footpaths and cycleways that link to existing networks to improve connectivity generally and to develop links between this site, nearby development sites and the town centre.
- Good accessibility to public transport services should be provided for, including the accommodation of new bus stops to link the development to the wider town.
- A Transport Assessment and Travel Plan to accompany development proposals.

As indicated at the pre-application stage, the A41 from which the site is accessed is heavily trafficked and will be put under further pressure from Cherwell Local Plan growth allocations, including the allocation on this site (Bicester 4).

This was recognised by Bicester Village in their application for Phase 4 of their development, where they have now delivered major highway improvements at and between the Esso roundabout and Pingle Drive junctions, having also provided a Bicester Park and Ride facility.

The highway improvements on the A41 related to the expansion of Bicester Village have delivered a new bus layby on the northbound side of the A41. The highway works which are related to the construction and use of the permitted Bicester Business Park would also have needed to provide a northbound and southbound bus layby; however, the northbound layby is now delivered and the southbound layby will now be delivered by 16/02505/OUT - Bicester Gateway (Kingsmere Retail).

Planning consent was granted in November 2013 for the construction of a Tesco food store of 8,135 square metres and petrol filing station on part of the consented office park site (Planning Ref: 12/01193/F). The S106 Deed of Variation in relation to the consented Tesco store and office park allows for the construction of up to 45,000 square metres of the B1(a)/B1(b) office space being delivered on the remainder of the site, as part of the previous outline planning consent for an office park.

The November 2013 deed of variation to the original Section 106 agreement (dated 26 October 2010 associated with planning permission 07/01106/OUT) set out appropriate contributions/mitigation schemes required in order to make the development acceptable. Any new Section 106 or Deed of Variation agreed for this development site will need to maintain the remaining contribution requirements proportionate to the scale of new development and amend how these are allocated against schemes where necessary, to fit the present context.

The cumulative impact of Local Plan growth development in Bicester will be severe if appropriate contributions are not secured from all development sites towards the strategic transport infrastructure required to mitigate the increase in transport movements.

The varied Section 106 was made prior to the current adopted Cherwell Local Plan, which includes increased growth and additional infrastructure requirements within the plan period, such as a South East Perimeter Road (SEPR). The SEPR is also now detailed in Oxfordshire County Council's Local Transport Plan 4, as a scheme to ease congestion on the A41, and will therefore directly contribute towards mitigating this development proposal's impact. The scheme is partly funded, but currently requires contributions to fund the western section proposed. This development will therefore be expected to contribute towards the SEPR or a scheme of similar benefit.

The varied Section 106 made provision to support rail service improvements, now partly implemented by East West Rail phase one. Oxfordshire County Council continue to support rail improvement schemes, making this sustainable form of travel more attractive and in turn reducing single occupancy car travel. The rail contribution carried forward in the new Section 106 or Deed of Variation must therefore be allocated against supporting East West Rail Phase 2."

Strategic transport modelling demonstrates the benefits that the SEPR will bring to the A41 /Oxford Road:

- The A41 Oxford Road is a key corridor in Bicester where junctions along its length are impacted significantly as a result of the growth of Bicester, including Bicester 4. The Application Site will increase the proportion of peak hour traffic through this corridor.
- The SEPR has been identified as a key piece of strategic infrastructure that will bring direct relief to the A41 corridor, thereby facilitating improved operation of junctions directly impacted by Bicester 4.
- Modelling has demonstrated the benefits that the SEPR would bring to the A41. In the AM peak:
- Over 1000 vehicles (pcu's) that would otherwise use the A41 Oxford Rd northbound through Vendee Drive would route via SEPR (eastbound)
- Around 930 vehicles (pcu's) that would otherwise use A41 Boundary Way and turn left on A41 Oxford Rd southbound past Bicester 4, would route via SEPR (westbound)
- Therefore, over 1930 vehicles (pcu's) would use the SEPR that would otherwise route along A41 past the Bicester 4 site.

It is acknowledged however, that the capacity released on the A41 by the SEPR will itself encourage some traffic that might otherwise choose NOT to use the A41, to divert along the corridor. When taking diverted traffic into account, the net reduction in traffic on the A41 in the vicinity of the Bicester 4 site would be around 1130 pcu's.

## Drainage

The drainage strategy is presented in Appendix F of the Flood Risk Assessment, which is itself contained in Appendix 13.1 of Volume 2 of the Environmental Statement. It is proposed to use SuDS to manage surface water runoff across the development.

OCC as Lead Local Flood Authority considers 'soft' SuDS (e.g. Ponds, Swales etc) preferable to 'hard' SuDS (e.g. Underground Storage Tanks). The images represented in the Design and Access statement (Bennets Associates - Dec 2017) on Page 9 would be representative of the type of the amenity value that can be added to the development by the use of these 'soft' SuDS.

OCC considered that the drainage proposals were not adequately described within the strategy document. For an outline application, the proposal needs to describe the attenuation storage volumes that are required to provide mitigation and achieve compliance with the proposed allowable discharge rates. Typically the applicant must show by way of a sketch, which describes the SuDS features and demonstrates that they fit within the red line application boundary. A supporting calculation needs to be provided and for initial sizing calculations in support of outline application the toolkit provided by the 'UK Suds' website is acceptable to OCC. These considerations were absent from the application. (Reason for objection)

The drainage strategy will need to comply with the Defra 'Non - Statutory Technical Standards' and good practice such as the ' Preliminary rainfall runoff management for developments ' ( Defra/EA R\&D Technical Report SC030219 Revision E), and 'The SuDS Manual' ( CIRIA C753).

In terms of the allowable discharge rates for the site, it will be required to consider the need to control and mitigate the additional runoff 'volumes' (Technical Standards S4 - S6) and 'rates' (Technical Standards S2 - S3). Therefore, QBAR greenfield rate for the site will be appropriate for the site or alternatively 'long term storage' should be provided.

The proposals to use permeable paving for parking spaces and rainwater harvesting for the site were very welcomed. Especially so, as the proposals will need to demonstrate a 'treatment train' approach is being achieved at the site, so that SuDS water quality is achieved.

No soakage testing results were provided with the application. Part infiltration in some areas of the site may be possible, therefore infiltration testing should be carried out at the site, which may form part of a condition.

A SuDS Management and Maintenance Plan will also be required for the site

## Travel Plan

A framework travel plan has been submitted for the Business Park which has been checked against our guidance.

It should be noted that at this stage the submitted travel plan does not include enough detail to satisfy our guidance or to be fully assessed. For this reason, comments are very general.

This framework travel plan will act as an umbrella plan for the site as a whole and will set the travel aspirations for the site. Future occupiers will either make a commitment to take on the objectives of this travel plan or if their business is over travel plan thresholds they will be develop their own travel plan using this framework travel plan as the basis for their plan. If their individual site is above travel plan thresholds they will also be expected to pay the appropriate monitoring fees.

- It has not been explained what the purpose of this framework travel plan is i.e. that it will act as an umbrella plan for the site and that it will be the basis nor any future travel plans that future occupiers develop. Or that this plan will be adopted by future occupiers who will be expected to work towards the overall goals and targets of this plan. Further to this it is not clear from the travel plan what is being planned for this site and the makeup of the site once the project is completed. No idea of the number of employees that are likely to be based at this site, clearly the 2,00 car parking spaces are going to be used to someone?
- Para 3.2 One of the main objectives of the travel plan should be to reduce Single Occupancy Vehicle (SOV) trips made to and from the site. It should also be noted that car share is one way of reducing SOV trips made to and from the site. Oxfordshire County Council recommends Oxfordshire Lift Share as the car share provider of choice https://liftshare.com/uk/community/oxfordshire.
- Section 4 travel plan coordinator, should note that it will be the TPC's responsibility to ensure that future occupiers are informed of the framework travel plan and their travel plan responsibilities, and to ensure that they work towards the aims and targets of this plan.
- If they have to develop their own travel plans it will be the TPC's responsibility to ensure that this happens within the required timescales and to inform them of the need to use the FTP as a basis for their own plans.
- Para 4.3 TPC contact details will need to be sent to the Travel Plan Team at Oxfordshire County Council.
- The FTP contains no measurable targets for the site. The FTP contains no information which can be used to set initial FTP targets such bas the 2011 Census travel to work data.
- A target will be required for each mode of travel in percentages and numbers for each year in which a survey will take place. We will be looking for a 5-10\% SOV reduction over the first five years of the FTP's operation.

Action plan requires further development. It should have a mixture of short, medium and longer term actions all with a completion date under headings such as measures to reduce the number of SOV journeys made to and from the site and measures to increase cycling and car sharing.

## S278 Highway Works:

An obligation to enter into a S278 Agreement will be required to secure mitigation/improvement works, including:

- Site accesses
- Pedestrian footway improvements along the A41/Oxford Road
- Bus stop adjacent to the development on the eastern side of the A41 Oxford Road subject to the event that the consented development which the bus stop forms a part of is not implemented
- Junction capacity improvements as appropriate


## Notes:

These highway works shall be secured by means of S106 with restriction not to implement development (or occasionally other trigger point) until S278 agreement has been entered into. The trigger by which time S278 works are to be completed shall also be included in the S106 agreement.

Identification of areas required to be dedicated as public highway and agreement of all relevant landowners will be necessary in order to enter into the S278 agreements.

## S38 Highway Works

An obligation to provide a spine road as part of the highway network or an on-site right of way may be required for the development. The S106 agreement will secure delivery via future completion of a S38 agreement.

## Planning Conditions:

In the event that permission is to be given, the following planning conditions should be attached:

## Site Access: Full Details

Prior to the commencement of the development hereby approved, full details of the means of access between the land and the highway including position, layout, and vision splays shall be submitted to and approved in writing by the Local Planning Authority. There shall be no obstruction of the vision splays above 0.6 m high. Thereafter and prior to the first occupation of any of the development, the means of access shall be constructed and retained in accordance with the approved details. Reason - In the interests of highway safety and to comply with Government guidance contained within the National Planning Policy Framework

## Cycle Parking

The development hereby approved shall not be commenced until a plan for the car parking spaces to serve the entire development has been submitted and approved in writing by the Local Planning Authority. All car parking shall be retained unobstructed except for the parking and manoeuvring of vehicles at all times thereafter, unless otherwise agreed in writing beforehand by the local planning authority.
Reason: To ensure appropriate levels of cycle parking are available at all times to serve the development, and to comply with Government guidance contained within the National Planning Policy Framework.

## Car Parking

The development hereby approved shall not be commenced until a plan for the car parking spaces to serve the entire development has been submitted and approved in writing by the Local Planning Authority. All car parking shall be retained unobstructed
except for the parking and manoeuvring of vehicles at all times thereafter, unless otherwise agreed in writing beforehand by the local planning authority.
Reason: To ensure appropriate levels of car parking are available at all times to serve the development, and to comply with Government guidance contained within the National Planning Policy Framework.

## Drainage

Development shall not begin until a surface water drainage scheme for the site, based on sustainable drainage principles and an assessment of the hydrological and hydro-geological context of the development, has been submitted to and approved in writing by the local planning authority. The scheme shall subsequently be implemented in accordance with the approved details before the development is completed. The scheme shall also include:

- Discharge Rates
- Discharge Volumes
- Maintenance and management of SUDS features (this maybe secured by a Section 106 Agreement)
- Sizing of features - attenuation volume
- Infiltration in accordance with BRE365
- Detailed drainage layout with pipe numbers
- SUDS - (list the suds features mentioned within the FRA to ensure they are carried forward into the detailed drainage strategy)
- Network drainage calculations
- Phasing

Reason - To ensure satisfactory drainage of the site in the interests of public health, to avoid flooding of adjacent land and property and to comply with Government guidance contained within the National Planning Policy Framework.

## Travel Plan

The submitted travel plan will be revised in line with comments received and resubmitted to the Local Planning Authority for approval before first occupation.

## Construction traffic management plan

Prior to commencement of the development hereby approved, a Construction Traffic Management Plan (CTMP) shall be submitted to and approved in writing by the Local Planning Authority. The CTMP shall include a commitment to deliveries only arriving at or leaving the site outside local peak traffic periods. Thereafter, the approved CTMP shall be implemented and operated in accordance with the approved details. Reason - In the interests of highway safety and the residential amenities of neighbouring occupiers.

Officer's Name: Rashid Bbosa
Officer's Title: Transport Engineer
Date: 23 February 2018

## Archaeology Schedule

## Recommendation:

Objection for the following reason/s:
The site is located in an area of archaeological interest and the results of an archaeological evaluation will need to be submitted along with this planning application prior to the determination of this application.

## Comments:

The site is located in an area of archaeological interest as shown by an archaeological evaluation over part of the site which recorded a range of archaeological deposits dating from the prehistoric to Roman periods. The site is located 650 m north of the site of the Roman Small Town of Alchester and is located along the line of the Roman Road heading north from this town. Iron Age and Roman settlement evidence has been recorded along the route of this road in the vicinity of this site including 300 m south and 260 m north east of the proposed site. A further Iron Age and Roman settlement has also been recorded 280m north of the site.

Prehistoric archaeological deposits have been recorded in the immediate area and two Bronze Age barrows are recorded 280m north east of the proposed site. The proposed site is also located immediately to the south and west of an area of Bronze Age settlement identified through archaeological excavation.

This excavation revealed a number of Bronze Age roundhouses either side of a bradded river channel. An oven was also recorded associated with one of the roundhouses along with a number of larger postholes or pits. Three cremation burials were also recorded on the site. A Roman channel was cut along the line of the braided channel. Bronze Age settlement sites such as this are relatively rare within the District and as such are of significance.

Only part of the proposed site was subject to an archaeological evaluation undertaken as part of a separate planning application. The area that was not investigated is immediately east of the line of the Roman road and immediately south of the area of Bronze Age settlement recorded by the excavation. It is therefore very likely that further aspects of this significant settlement could survive within this previously un-investigated area of the proposed development and archaeological deposits from the Roman period could survive along the line of the road as recorded elsewhere in the vicinity.

This is recognised in the submitted EIA which states in 10.64 that,
> 'It is likely these remains will extend somewhat beyond the trenching area and therefore the potential for further prehistoric and Roman finds or features is considered high.'

The EIA sets out proposed mitigation for the site. This mitigation only proposes to undertake a strip map and recording action within areas where the previous evaluation recorded archaeological deposits (10.73). The remaining area of the site, presumably including the portion of the site that was not subject to this evaluation, would only be subject to a topsoil watching brief.

As this includes the area that is likely to contain further aspects of the identified Bronze Age settlement as well as any previously unidentified Roman settlement along the line of the Roman road this proposed mitigation would not be appropriate.

A programme of archaeological evaluation will need to be undertaken on this uninvestigated part of the site ahead of the determination of any planning application for the site in order to identify whether or not archaeological deposits related to the Bronze Age settlement and Roman road survive and to provide the information required to assess the significance of any surviving archaeological deposits.

The EIA also states that we were asked whether or not any archaeological investigations could be conditioned on the $15^{\text {th }}$ August 2017 (10.9). The EIA states that a decision is awaited. This is not true and we responded to this email on the $18^{\text {th }}$ August 2017 to Nuala C. Woodley of AOC where we stated,
'I cannot agree that this can simply be undertaken as a condition on any planning application and I certainly do not agree with your proposal that the areas which have not been evaluated can be dealt with through a watching brief.'

In accordance with the National Planning Policy Framework (NPPF), we would therefore recommend that, prior to the determination of this application the applicant should therefore be responsible for the implementation of an archaeological field evaluation. This must be carried out by a professionally qualified archaeological organisation and should aim to define the character and extent of the archaeological remains within the application area, and thus indicate the weight which should be attached to their preservation.

This information can be used for identifying potential options for minimising or avoiding damage to the archaeology and on this basis, an informed and reasonable decision can be taken.

This evaluation must be undertaken in line with an agreed written scheme of investigation as set out in the CIfA standards and Guidance for Archaeological Field Evaluation (2014, para 3.1.11). We will need to produce a design brief which will set out the requirements for this evaluation.

## Planning Conditions:

In the event that permission is to be given, the following planning conditions should be attached:

Officer's Name: Richard Oram
Officer's Title: Planning Archaeologist
Date: $11^{\text {th }}$ January 2018

## Minerals \& Waste

## Recommendation:

No objection

## Key issues:

This site is within 400 m of a waste management facility safeguarded in the Oxfordshire Minerals and Waste Local Plan: Part 1 - Core Strategy (Bicester Sewage Treatment Works (STW)). Therefore, any potential effects of the proposed development that may directly or indirectly prevent or prejudice the operation of Bicester STW should be addressed.

## Legal agreement required to secure:

N/A

## Conditions:

N/A

## Informatives:

N/A

## Detailed comments:

Officer's Name: Elise Kinderman
Officer's Title: Principal Minerals and Waste Policy Officer
Date: 19 ${ }^{\text {th }}$ January 2018

## motion

Appendix C
Traffic Accident Data

Thames Valley Police

## Chief Constable Francis Habgood QPM

Ms K Lewis
motion
8 Duncannon Street,
LONDON
WC2N 4JF

Telephone: 101
Direct dial: 01865542051
Email: publicaccess@thamesvalley.pnn.police.uk

Our ref: HQ/PA/001870/17
Your ref:
7 July 2017
Dear Ms Lewis
I write in response to the above-referenced Freedom of Information Act (FOIA) request submitted on 5 July 2017. Thames Valley Police has now considered this request, which for clarity, has been repeated below:

| Request | $\underline{\text { Response }}$ |
| :--- | :--- |
| I am after the total number of slight, |  |
| serious and fatal accidents over the most |  |
| recent five year period to include |  |
| causation factors. The area I require this |  |
| for is as follows: | Slight - 40 <br> Serious - 5 <br> Fatal - 2 |
|  <br> Ride/Vendee Drive roundabout and the <br> Kings End/Middleton Stoney roundabout; <br> A41 between the Esso Roundabout and <br> Rodney House Roundabout; and, <br> Lakeview Drive. | Plare attached data sheet <br> for causation factors. The causation <br> factors listed are the initial opinion of <br> attending officers. These may be <br> disproven in following investigations. |

## Complaint Rights

If you are dissatisfied with the handling procedures or the decision made by Thames Valley Police, you can lodge a complaint with the force to have the decision reviewed within two months of the date of this response. Complaints should be made in writing to the FOI inbox; publicaccess@thamesvalley.pnn.police.uk.

If, after lodging a complaint with Thames Valley Police, you are still unhappy with the outcome, you may make application to the Information Commissioner at the Information Commissioner's Office, Wycliffe House, Water Lane, Wilmslow, Cheshire, SK9 5AF.


Thames Valley Police Chief Constable Francis Habgood QPM

If you require any further assistance, please do not hesitate to contact this office.
Yours sincerely

Darren Humphries
Public Access
Joint Information Management Unit
Number
of
casualties


| 501. Impaired by alcohol <br> 406. Failed to judge other persons path or sp <br> 410. Loss of control <br> 405. Failed to look properly <br> 602. Careless/Reckless/In a hurry <br> 405. Failed to look properly <br> 307. Travelling to fast for pons path or Sp <br> 306. Exceeding speed limit <br> 505. Illness or disability, mental or physical <br> 405. Failed to look properly <br> 405. Failed to look properly <br> 103. Slippery judge other persons path or sp <br> 103. Slippery road (due to weather) <br> 308. Following too close <br> 999. Other <br> 308. Following too close <br> 902 . Vehicle in course of crime <br> 407. Too close to cyclist, horse or pedestrian <br> 501. Impaired by alcohol <br> 405. Failed braking <br> 405. Failed to look properly <br> 606. Inexperience of driving on the left <br> 109. Animal or object in carriageway <br> 405. Failed to look properly <br> 407. Too close to cyclist, horse or pedestrian 405. Failed to look properly <br> 501. Impaired by alcohol <br> 405. Failed to look properly <br> 505. Illness or disability, mental or physical 203. Defective brakes <br> 508. Driver using <br> 602. Careless/Reckless/In a hurry <br> 503. Fatigue <br> 405. Failed to look properly <br> 405. Failed to look properly <br> 403. Poor turn or manoeuvre <br> 302. Disobeyed Give Way or Stop sign or ma |  |
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## motion

Appendix D
Parameters Plan


## motion

Appendix E
Highways Access Plan


Appendix F
Proposed Junction Mitigation





## motion

Appendix G



Appendix H
Stage 1 Road Safety Audit

## BICESTER OFFICE PARK

A41/ Lakeview Drive, Proposed Highway Arrangement

Stage 1 Road Safety Audit
Requested by Motion

March 2018

## Gateway TSP

| Project: | Bicester Office Park <br> A41/ Lakeview Drive, Proposed Highway Arrangement |
| :--- | :--- |
| Client: | Motion |
| Document: | Stage 1 Road Safety Audit |
| Gateway TSP ref: | WP/ SG/ 170211 RSA1 v1.1 |
| Issue date: | 4th April 2018 |
| Status: | Final v1.1 |
| Authorised by: | WP |

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## Gateway TSP

## Road Safety Engineering

84 North Street
Guildford
Surrey
GU1 4AU
01483679350
admin@gateway-tsp.co.uk
www.gateway-tsp.co.uk

## Gateway

## CONTENTS

1 Introduction ..... 1
2 Items Considered by this Road Safety Audit ..... 3
3 Collision Data ..... 4
4 Previous Road Safety Audit. ..... 5
5 Problems Identified by this Road Safety Audit ..... 6
6 Audit Team Statement .....  .7
Appendices
Appendix A: Location Plan(s)
Appendix B: Designer's Response

1

11 This report describes a Stage 1 Road Safety Audit (RSA) of proposed junction amendments at A41 junction with Lakeview Drive, Bicester.

12 The highway works considered by this Audit comprise kerb realignments and amendments to the existing signalised junction to provide additional traffic lanes on the A41 approaches to the junction, also providing an additional right turn lane into Lakeview Drive by reducing the width of the central median.

13 A41 Oxford Road is a dual carriageway with two lanes in each direction and a 40 mph posted speed limit with double yellow lines throughout. The carriageway is lit and there are shared use pedestrian/ cycle facilities on all arms. Lakeview Drive has a three lane approach to the junction, one left turn and two right turn, and a two lane exit from the junction, with a 30 mph posted speed limit.

14 It is understood that there are other planned improvement works consented as part of another development but not yet implemented. This will increase the number of traffic lanes on A41, which is replicated in these proposals.

15 This Road Safety Audit was carried out by Wendy Palmer and Steve Giles and consisted of a desktop study and a site visit, which was carried out on Thursday 29 ${ }^{\text {th }}$ March 2018, when the weather was raining and the road surface damp. Traffic flows were as expected for the time of day.

16 The terms of reference for this RSA are as described in the Design Manual for Roads and Bridges (DMRB) document HD19/15. The Audit Team is independent of the project design team and has not been involved in the design process in any other capacity. The audit considers only the potential road safety implications of the scheme and has not verified compliance of the design with any other criteria.

17 The Audit Team has not been made aware of any Departures from Standard. Whilst reference may be made to design standards, this report is not intended to provide a design check.

18 Recommendations are aimed at addressing the identified potential road safety problems. However, there may be other acceptable ways to overcome a problem, considering wider constraints and opportunities; the Auditors would be pleased to discuss such alternative solutions as appropriate. The recommendations contained herein do not absolve the Designer of his/her responsibilities.

## Gateway <br> TSP

| Document ref. | Rev. | Originator | Title |
| :--- | :--- | :--- | :--- |
| 170211-TK02 | - | Motion | Swept Path Analysis Right Turn Lane Access |
| $170211-$ TK03 | - | Motion | Swept Path Analysis Right Turn Lane Egress |
| $170211-07$ | A | Motion | Proposed Highway Arrangement |
| $170211-08$ | - | Motion | Proposed Highway Arrangement |
| $170211-09$ | - | Motion | Island Dimensions |

Additional/background information provided to the Audit Team

- None


## Gateway <br> TSP

3.1 Personal Injury Collision (PIC) information was requested from Crashmap (www.crashmap.co.uk) which indicated that there have been no PICs at the junction during the latest five year period.
4.1 The Audit Team is unaware of any previous road safety audits on these proposals.

## Gateway TSP

5

## PROBLEMS IDENTIFIED BY THIS ROAD SAFETY AUDIT

## General Matters

5.1 The Audit Team raises no concerns at this Stage 1 RSA in respect of general matters.

## Local Alignment

5.2 The Audit Team raises no concerns at this Stage 1 RSA in respect of local alignment.

## Junctions

5.3 The Audit Team raises no concerns at this Stage 1 RSA in respect of junctions.

## Non-motorised User Provision

5.4 The Audit Team raises no concerns at this Stage 1 RSA in respect of non-motorised user provision. However, full details should be prepared in detailed design, for the purposes of Stage 2 RSA.

## Road Signs, Carriageway Markings and Lighting

5.5 The Audit Team raises no concerns at this Stage 1 RSA in respect of road signs, carriageway markings and lighting. However, full details should be prepared in detailed design, for the purposes of Stage 2 RSA.

## 6 AUDIT TEAM STATEMENT

6.1 We certify that this Road Safety Audit has been carried out in accordance with DMRB document HD 19/ 15.

## Audit Team Leader

Wendy Palmer
MCIHT, MSoRSA, HE Cert Comp
Road Safety Engineer

Signed:


Date: $\quad 3^{\text {rd }}$ April 2018

## Audit Team Members)

Steve Giles
BEng (Hons), Eng, FIHE, MCIHT, MICE, CMILT, MSoRSA, HE Cert Comp
Director \& Senior Road Safety Consultant

Signed:


Date: $\quad 3^{\text {rd }}$ April 2018

> APPENDIX A
> Location Plan(s)


# APPENDIX B <br> Designer"s Response 

Project:
Client:
Document:
Gateway TSP ref
Status:
Issue date:

Bicester Office Park
A41/ Lakeview Drive, Proposed Highway Arrangement
Motion
Stage 1 Road Safety Audit
WP/ SG/ 170211 RSA1 v1.1
Final v1.1
4th April 2018

| Item <br> No. | Audit Team Recommendation | Designer's Response | Audit Team's Further Comments |
| :--- | :--- | :--- | :--- |
| $\mathbf{5 . 1}$ | $\mathrm{n} / \mathrm{a}$ |  |  |
| $\mathbf{5 . 2}$ | $\mathrm{n} / \mathrm{a}$ |  |  |
| $\mathbf{5 . 3}$ | $\mathrm{n} / \mathrm{a}$ |  |  |
| $\mathbf{5 . 4}$ | $\mathrm{n} / \mathrm{a}$ |  |  |
| $\mathbf{5 . 5}$ | $\mathrm{n} / \mathrm{a}$ |  |  |


| Project: | Bicester Office Park |
| :--- | :--- |
|  | A41/ Lakeview Drive, Proposed Highway Arrangement |
| Client: | Motion |
| Document: | Stage 1 Road Safety Audit |
| Gateway TSP ref: | WP/ SG/ 170211 RSA1 v1.1 |
| Status: | Final v1.1 |
| Issue date: | 4th April 2018 |

## Designer's Statement:

I confirm that I have considered the items that have arisen in the Stage 1 Road Safety Audit Report and my response to its recommendations are set out above.


Designer: David Lewis
Date: $\quad 4^{\text {th }}$ April 2018

## Audit Team Statement:

We agree/ do not agree [delete as appropriate] with the Designer's Response and our comments are provided above.


Audit Team Leader: Wendy Palmer
Date:
$4^{\text {th }}$ April 2018

Highway Authority/Project Sponsor/ Client Organisation Statement: I accept/ do not accept the Designer's Response (delete as appropriate)
[Name], on behalf of Highway Authority/ Project Sponsor/ Client Organisation (delete as appropriate)

Date:

## motion

Appendix I
Bicester Traffic Model Outputs



Appendix J
Census Data and Trip Distribution Calculations

| Anticipated Route | Outer Areas | Cherwell District | Total | Percentage |
| :--- | :---: | :---: | :---: | :---: |
| A41 South | 904 | 486 | 1,390 | $27 \%$ |
| Vendee Drive | 108 | 439 | 547 | $12 \%$ |
| Kingsmere | 0 | 195 | 195 | $3 \%$ |
| A41 East | 568 | 695 | 1,263 | $23 \%$ |
| Oxford Road North | 274 | 1513 | 1,787 | $35 \%$ |
| Total |  |  | 5,182 | $100 \%$ |


| Output Area | Location | Anticipated Route | People |
| :--- | :--- | :--- | :---: |
| E02005921 : Cherwell 001 | North Banbury | A41 South | 16 |
| E02005922 : Cherwell 002 | North Banbury | A41 South | 37 |
| E02005923 : Cherwell 003 | Central Banbury | A41 South | 22 |
| E02005924 : Cherwell 004 | North East Banbury | A41 South | 36 |
| E02005925 : Cherwell 005 | West Banbury | A41 South | 44 |
| E02005926 : Cherwell 006 | South West Banbury | A41 South | 14 |
| E02005927 : Cherwell 007 | South Banbury | A41 South | 31 |
| E02005928 : Cherwell 008 | South Banbury | A41 South | 61 |
| E02005929 : Cherwell 009 | South West Banbury | A41 South | 26 |
| E02005930 : Cherwell 010 | South East Banbury | Oxford Road (50\%) | Vendee Drive (50\%) |
|  |  | A41 East (50\%) | 61 |
| E02005931 : Cherwell 0111 | North Bicester | Oxford Road (50\%) | 61 |
| E02005932 : Cherwell 012 | North Central Bicester | Oxford Road | 172 |
| E02005933 : Cherwell 013 | North East Bicester | A41 East (50\%) | 172 |
|  |  | Oxford Road (50\%) | 460 |
| E02005934 : Cherwell 014 | West Central Bicester | Oxford Road (50\%) | 288 |
|  |  | Vendee Drive (50\%) | 287 |
| E02005935 : Cherwell 015 | South Central Bicester | A41 East (33\%) | Kingsmere (33\%) |
|  | Oxford Road (33\%) | 338 |  |
| E02005936 : Cherwell 016 | South Bicester | A41 East (25\%) | 195 |
| E02005937 : Cherwell 017 | North East Kidlington | A41 South (50\%) | 195 |
| E02005938 : Cherwell 018 | Central Kidlingon | A41 South | 195 |
| E02005939 : Cherwell 019 | South Kidlington | A41 South | 40 |
|  |  |  | 78 |
| Total |  | 40 |  |


| Place of Residence | Anticipated Route | People |
| :--- | :---: | :---: |
| Aylesbury Vale | A41 east | 546 |
| Dacorum | A41 east | 7 |
| Central Bedfordshire | A41 east | 10 |
| Luton | A41 east | 5 |
| Oxford | A41 south | 206 |
| West Oxfordshire | A41 south | 145 |
| Vale of White Horse | A41 south | 104 |
| Southampton | A41 south | 13 |
| Swindon | A41 south | 13 |
| West Berkshire | A41 south | 8 |
| Reading | A41 south | 7 |
| Bournemouth | A41 south | 6 |
| New Forest | A41 south | 3 |
| Portsmouth | A41 south | 3 |
| Wiltshire | A41 south | 3 |
| Eastleigh | A41 south | 2 |
| Hart | A41 south | 2 |
| Test Valley | A41 south | 2 |
| Poole | A41 south | 2 |
| East Hampshire | A41 south | 1 |
| Fareham | A41 south | 1 |
| Winchester | A41 south | 1 |
| Bristol, City of | A41 south | 1 |
| Christchurch | A41 south | 1 |
| Cornwall,Isles of Scilly | A41 south | 1 |
| East Devon | A41 south | 1 |
| South Oxfordshire | A41 south | 120 |
| Wycombe | A41 south | 46 |
| Stratford-on-Avon | A41 south | 19 |
| Birmingham | A41 south | 13 |
| Brighton and Hove | A41 south | 12 |
| Warwick | A41 south | 10 |
| Chiltern | A41 south | 9 |
| Wandsworth | A41 south | 7 |
| Windsor and Maidenhead south | A41 south | 4 |
| Ealing | A41 south | 7 |
| Camden | A41 south | 6 |
| Southwark | A41 south | 5 |
| Westminster,City of London | A41 south | 5 |
| South Bucks | A41 south | 5 |
| Rugby | A41 south | 5 |
| Hammersmith and Fulham | A41 south | 5 |
| Kensington and Chelsea | A41 south | 4 |
| Richmond upon Thames | A41 south | 4 |
| Wigan | A41 south | 4 |
| Bracknell Forest | 4 |  |
| Coventry | Kingston upon Hull, City of |  |


| Three Rivers | A41 south | 3 |
| :--- | :--- | :--- |
| Barking and Dagenham | A41 south | 3 |
| Brent | A41 south | 3 |
| Hounslow | A41 south | 3 |
| Basingstoke and Deane | A41 south | 3 |
| Elmbridge | A41 south | 3 |
| Cheltenham | A41 south | 3 |
| Solihull | A41 south | 3 |
| East Hertfordshire | A41 south | 2 |
| Charnwood | A41 south | 2 |
| Greenwich | A41 south | 2 |
| Haringey | A41 south | 2 |
| Hillingdon | A41 south | 2 |
| Merton | A41 south | 2 |
| Tower Hamlets | A41 south | 2 |
| Manchester | A41 south | 2 |
| Wokingham | A41 south | 2 |
| Bath and North East Somerset | A41 south | 2 |
| Herefordshire, County of | A41 south | 2 |
| Sandwell | A41 south | 2 |
| Stoke-on-Trent | A41 south | 2 |
| Wolverhampton | A41 south | 2 |
| North Hertfordshire | A41 south | 1 |
| St Albans | A41 south | 1 |
| Thurrock | A41 south | 1 |
| Watford | A41 south | 1 |
| Welwyn Hatfield | A41 south | 1 |
| Blaby | A41 south | 1 |
| Mansfield | Oxford Road | 13 |
| Nottingham | Oxford Road | 2 |
| Harrow | Oxford Road | 2 |
| Havering | A41 south | 1 |
| Lambeth | A41 south | 1 |
| Newham | A41 south | 1 |
| Redbridge | A41 south | 1 |
| Stockton-on-Tees | A41 south | 1 |
| Liverpool | A41 south | 1 |
| Oldham | A41 south | 1 |
| Salford | A41 south | 1 |
| South Ribble | A41 south | 1 |
| Ashford | A41 south | 1 |
| Reigate and Banstead | A41 south | 1 |
| Rushmoor | A41 south | 1 |
| Malvern Hills | A41 south | 1 |
| Redditch | A41 south | 1 |
| Shropshire | A41 south | 1 |
| Daventry | Rotherham | 1 |
| Sheffield | Aiddlesbrough |  |
|  |  |  |


| Bradford | Oxford Road | 1 |
| :--- | :---: | :---: |
| East Riding of Yorkshire | Oxford Road | 1 |
| Leeds | Oxford Road | 1 |
| Selby | Oxford Road | 1 |
| Leicester | Oxford Road | 6 |
| South Northamptonshire | Oxford Road | 186 |
| Northampton | Oxford Road | 42 |
| Wellingborough | Oxford Road | 5 |
| Bedford | Oxford Road | 4 |
| St Edmundsbury | Oxford Road | 2 |
| East Cambridgeshire | Oxford Road | 1 |
| Great Yarmouth | Oxford Road | 1 |
| Huntingdonshire | Oxford Road | 1 |
| Peterborough | Oxford Road | 1 |
| South Cambridgeshire | Oxford Road | 1 |
| Harborough | Oxford Road | 1 |
| Kettering | Oxford Road | 1 |
| Milton Keynes | Vendee Drive | 108 |
| Cherwell District |  |  |
| Total | N/A | 3,328 |

Appendix K
Model Output Files

## motion

Oxford Road / Middleton Stoney Road / Kings End


Filename: Middleton Stoney - Kings End - Oxford Road - 2018-06-21 (No Mitigation).j9
Path: N:\Projects\Imbic2 170211\Analysis\Modelling\Middleton Stoney
Report generation date: 03/07/2018 10:32:07

```
„2026 BTM, AM
"2026 BTM, PM
"2026 Baseline, AM
„2026 Baseline, PM
"2026 BTM + 60sqm, AM
"2026 BTM + 60sqm, PM
```

Summary of junction performance

|  | AM |  |  |  |  |  |  | PM |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Queue (Veh) | Delay (s) | RFC | LOS | Junction <br> Delay (s) | Junction LOS | Network Residual Capacity | Queue (Veh) | Delay <br> (s) | RFC | LOS | Junction Delay (s) | Junction LOS | Network Residual Capacity |
|  | 2026 BTM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Arm 1 | 1.8 | 7.55 | 0.65 | A | 21.55 | C | $-4 \%$ <br> [Arm 2] | 2.2 | 10.83 | 0.69 | B | 13.28 | B | $1 \%$ <br> [Arm 2] |
| Arm 2 | 12.5 | 60.87 | 0.94 | F |  |  |  | 6.3 | 29.47 | 0.87 | D |  |  |  |
| Arm 3 | 0.9 | 3.30 | 0.49 | A |  |  |  | 2.9 | 6.48 | 0.74 | A |  |  |  |
|  | 2026 Baseline |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Arm 1 | 2.9 | 10.45 | 0.74 | B | 289.54 | F | $-17 \%$ <br> [Arm 2] | 3.1 | 15.11 | 0.76 | C | 17.54 | C | $0 \%$ <br> [Arm 2] |
| Arm 2 | 209.6 | 925.89 | 1.18 | F |  |  |  | 7.2 | 33.23 | 0.88 | D |  |  |  |
| Arm 3 | 1.0 | 3.36 | 0.50 | A |  |  |  | 6.0 | 11.80 | 0.86 | B |  |  |  |
|  | 2026 BTM + 60sqm |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Arm 1 | 3.4 | 11.97 | 0.78 | B | 410.61 | F | $\begin{gathered} -20 \% \\ \text { [Arm 2] } \end{gathered}$ | 3.6 | 17.41 | 0.79 | C | 20.61 | C | $0 \text { \% }$ <br> [Arm 2] |
| Arm 2 | 290.9 | 1308.81 | 1.26 | F |  |  |  | 7.4 | 34.37 | 0.89 | D |  |  |  |
| Arm 3 | 1.0 | 3.39 | 0.50 | A |  |  |  | 8.5 | 16.20 | 0.90 | C |  |  |  |

There are warnings associated with one or more model runs - see the 'Data Errors and Warnings' tables for each Analysis or Demand Set.

[^0]
## File summary

File Description

| Title | Middleton Stoney - Kings End - Oxford Road |
| :--- | :--- |
| Location | Bicester |
| Site number |  |
| Date | $15 / 06 / 2017$ |
| Version |  |
| Status | (new file) |
| Identifier |  |
| Client |  |
| Jobnumber |  |
| Enumerator | MOTIONVklewis |
| Description |  |

## Units

| Distance units | Speed units | Traffic units input | Traffic units results | Flow units | Average delay units | Total delay units | Rate of delay units |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| m | kph | Veh | Veh | perHour | s | -Min | perMin |

## Analysis Options

| Vehicle <br> length (m) | Calculate Queue <br> Percentiles | Calculate detailed <br> queueing delay | Calculate residual <br> capacity | Residual capacity <br> criteria type | RFC <br> Threshold | Average Delay <br> threshold (s) | Queue threshold <br> (PCU) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 5.75 |  |  | $\checkmark$ | Delay | 0.85 | 36.00 | 20.00 |

## Demand Set Summary

| ID | Scenario name | Time Period name | Traffic profile type | Start time (HH:mm) | Finish time (HH:mm) | Time period length (min) | Time segment length (min) | Run automatically |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| D1 | 2026 BTM | AM | FLAT | 07:45 | 09:15 | 90 | 15 | $\checkmark$ |
| D2 | 2026 BTM | PM | FLAT | 17:00 | 18:30 | 90 | 15 | $\checkmark$ |
| D3 | 2026 Baseline | AM | FLAT | 07:45 | 09:15 | 90 | 15 | $\checkmark$ |
| D4 | 2026 Baseline | PM | FLAT | 17:00 | 18:30 | 90 | 15 | $\checkmark$ |
| D5 | 2026 BTM + 60sqm | AM | FLAT | 07:45 | 09:15 | 90 | 15 | $\checkmark$ |
| D6 | 2026 BTM + 60sqm | PM | FLAT | 17:00 | 18:30 | 90 | 15 | $\checkmark$ |

## Analysis Set Details

| ID | Include in report | Network flow scaling factor (\%) | Network capacity scaling factor (\%) |
| :---: | :---: | :---: | :---: |
| A1 | $\checkmark$ | 100.000 | 100.000 |

THE FUTURE

## 2026 BTM, AM

Data Errors and Warnings

| Severity | Area | Item | Description |
| :---: | :---: | :--- | :--- |
| Warning | Geometry | Arm 1 - Roundabout <br> Geometry | Effective flare length is over 30m, which is outside the normal range. Treat capacities with increasing caution. |

## Junction Network

## Junctions

| Junction | Name | Junction Type | Arm order | Junction Delay (s) | Junction LOS |
| :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | untitled | Standard Roundabout | $1,2,3$ | 21.55 | C |

## Junction Network Options

| Driving side | Lighting | Network residual capacity (\%) | First arm reaching threshold |
| :---: | :---: | :---: | :---: |
| Left | Normal/unknown | -4 | Arm 2 |

## Arms

## Arms

| Arm | Name | Description |
| :---: | :--- | :--- |
| $\mathbf{1}$ | Middleton Stoney |  |
| $\mathbf{2}$ | Kings End |  |
| $\mathbf{3}$ | Oxford Road |  |

Roundabout Geometry

| Arm | V - Approach road half- <br> width $(\mathbf{m})$ | E - Entry width <br> $(\mathbf{m})$ | I' - Effective flare <br> length $(\mathbf{m})$ | R - Entry radius <br> $(\mathbf{m})$ | D - Inscribed circle <br> diameter $(\mathbf{m})$ | PHI - Conflict (entry) <br> angle (deg) | Exit <br> only |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | 3.50 | 7.50 | 32.0 | 20.0 | 19.0 |  |  |
| $\mathbf{2}$ | 3.50 | 4.50 | 10.0 | 80.0 | 19.0 |  |  |
| $\mathbf{3}$ | 7.50 | 7.50 | 0.0 | 17.0 | 19.0 |  |  |

## Slope / Intercept / Capacity

Roundabout Slope and Intercept used in model

| Arm | Final slope | Final intercept (PCU/hr) |
| :---: | :---: | :---: |
| $\mathbf{1}$ | 0.699 | 1893 |
| $\mathbf{2}$ | 0.591 | 1315 |
| $\mathbf{3}$ | 0.749 | 2174 |

The slope and intercept shown above include any corrections and adjustments.

## Traffic Demand

## Demand Set Details

| ID | Scenario <br> name | Time Period <br> name | Traffic profile <br> type | Start time <br> (HH:mm) | Finish time <br> (HH:mm) | Time period length <br> (min) | Time segment length <br> (min) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| D1 | 2026 BTM | AM | FLAT | $07: 45$ | $09: 15$ | Run <br> automatically |  |


| Vehicle mix varies over turn | Vehicle mix varies over entry | Vehicle mix source | PCU Factor for a HV (PCU) |
| :---: | :---: | :---: | :---: |
| $\checkmark$ | $\checkmark$ | HV Percentages | 2.00 |

THE FUTURE

Demand overview (Traffic)

| Arm | Linked arm | Profile type | Use O-D data | Average Demand (Veh/hr) | Scaling Factor (\%) |
| :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ |  | FLAT | $\checkmark$ | 875 | 100.000 |
| $\mathbf{2}$ |  | FLAT | $\checkmark$ | 773 | 100.000 |
| $\mathbf{3}$ |  | FLAT | $\checkmark$ | 1030 | 100.000 |

## Origin-Destination Data

Demand (Veh/hr)

|  | To |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| From |  | $\mathbf{1}$ | $\mathbf{2}$ | $\mathbf{3}$ |
|  | $\mathbf{1}$ | 0 | 111 | 764 |
|  | $\mathbf{2}$ | 2 | 0 | 771 |
|  | $\mathbf{3}$ | 295 | 735 | 0 |

## Vehicle Mix

Heavy Vehicle Percentages

|  | To |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| From |  | $\mathbf{1}$ | $\mathbf{2}$ | $\mathbf{3}$ |
|  | $\mathbf{1}$ | 0 | 0 | 1 |
|  | $\mathbf{2}$ | 0 | 0 | 4 |
|  | $\mathbf{3}$ | $\mathbf{1}$ | $\mathbf{3}$ | 0 |

## Results

Results Summary for whole modelled period

| Arm | Max RFC | Max delay (s) | Max Queue (Veh) | Max LOS | Average Demand <br> (Veh/hr) | Total Junction <br> Arrivals (Veh) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | 0.65 | 7.55 | 1.8 | A | 875 | 1313 |
| $\mathbf{2}$ | 0.94 | 60.87 | 12.5 | F | 773 | 1159 |
| $\mathbf{3}$ | 0.49 | 3.30 | 0.9 | A | 1030 | 1545 |

## Main Results for each time segment

07:45-08:00

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 875 | 219 | 732 | 1354 | 0.646 | 868 | 296 | 0.0 | 1.8 | 7.310 | A |
| 2 | 773 | 193 | 758 | 829 | 0.932 | 740 | 842 | 0.0 | 8.3 | 32.981 | D |
| 3 | 1030 | 258 | 2 | 2121 | 0.486 | 1026 | 1496 | 0.0 | 0.9 | 3.277 | A |

08:00-08:15

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 875 | 219 | 735 | 1352 | 0.647 | 875 | 297 | 1.8 | 1.8 | 7.544 | A |
| 2 | 773 | 193 | 764 | 826 | 0.936 | 765 | 846 | 8.3 | 10.2 | 50.484 | F |
| 3 | 1030 | 258 | 2 | 2121 | 0.486 | 1030 | 1527 | 0.9 | 0.9 | 3.298 | A |

08:15-08:30

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 875 | 219 | 735 | 1352 | 0.647 | 875 | 297 | 1.8 | 1.8 | 7.552 | A |
| 2 | 773 | 193 | 764 | 826 | 0.936 | 769 | 846 | 10.2 | 11.1 | 55.309 | F |
| 3 | 1030 | 258 | 2 | 2121 | 0.486 | 1030 | 1531 | 0.9 | 0.9 | 3.298 | A |

08:30-08:45

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 875 | 219 | 735 | 1352 | 0.647 | 875 | 297 | 1.8 | 1.8 | 7.552 | A |
| 2 | 773 | 193 | 764 | 826 | 0.936 | 771 | 846 | 11.1 | 11.7 | 57.953 | F |
| 3 | 1030 | 258 | 2 | 2121 | 0.486 | 1030 | 1533 | 0.9 | 0.9 | 3.298 | A |

08:45-09:00

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 875 | 219 | 735 | 1352 | 0.647 | 875 | 297 | 1.8 | 1.8 | 7.552 | A |
| 2 | 773 | 193 | 764 | 826 | 0.936 | 771 | 846 | 11.7 | 12.2 | 59.664 | F |
| 3 | 1030 | 258 | 2 | 2121 | 0.486 | 1030 | 1533 | 0.9 | 0.9 | 3.298 | A |

09:00-09:15

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity <br> (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 875 | 219 | 735 | 1352 | 0.647 | 875 | 297 | 1.8 | 1.8 | 7.552 | A |
| 2 | 773 | 193 | 764 | 826 | 0.936 | 772 | 846 | 12.2 | 12.5 | 60.873 | F |
| 3 | 1030 | 258 | 2 | 2121 | 0.486 | 1030 | 1534 | 0.9 | 0.9 | 3.298 | A |

## 2026 BTM, PM

Data Errors and Warnings

| Severity | Area | Item | Description |
| :---: | :---: | :--- | :--- |
| Warning | Geometry | Arm 1 - Roundabout <br> Geometry | Effective flare length is over 30m, which is outside the normal range. Treat capacities with increasing caution. |

## Junction Network

## Junctions

| Junction | Name | Junction Type | Arm order | Junction Delay (s) | Junction LOS |
| :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | untitled | Standard Roundabout | $1,2,3$ | 13.28 | B |

## Junction Network Options

| Driving side | Lighting | Network residual capacity (\%) | First arm reaching threshold |
| :---: | :---: | :---: | :---: |
| Left | Normal/unknown | 1 | Arm 2 |

## Traffic Demand

## Demand Set Details

| ID | Scenario <br> name | Time Period <br> name | Traffic profile <br> type | Start time <br> (HH:mm) | Finish time <br> (HH:mm) | Time period length <br> ( $\mathbf{m i n}$ ) | Time segment length <br> (min) | Run <br> automatically |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| D2 | 2026 BTM | PM | FLAT | $17: 00$ | $18: 30$ | 90 | 15 |  |


| Vehicle mix varies over turn | Vehicle mix varies over entry | Vehicle mix source | PCU Factor for a HV (PCU) |
| :---: | :---: | :---: | :---: |
| $\checkmark$ | $\checkmark$ | HV Percentages | 2.00 |

Demand overview (Traffic)

| Arm | Linked arm | Profile type | Use O-D data | Average Demand (Veh/hr) | Scaling Factor (\%) |
| :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ |  | FLAT | $\checkmark$ | 747 | 100.000 |
| $\mathbf{2}$ |  | FLAT | $\checkmark$ | 792 | 100.000 |
| $\mathbf{3}$ |  | FLAT | $\checkmark$ | 1600 | 100.000 |

## Origin-Destination Data

Demand (Veh/hr)

|  | To |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| From |  | $\mathbf{1}$ | $\mathbf{2}$ | $\mathbf{3}$ |
|  | $\mathbf{1}$ | 0 | 73 | 674 |
|  | $\mathbf{2}$ | 4 | 0 | 788 |
|  | $\mathbf{3}$ | 462 | 1138 | 0 |

## Vehicle Mix

Heavy Vehicle Percentages

|  | To |  |  |  |
| :--- | :--- | :--- | :--- | :--- |
| From |  | $\mathbf{1}$ | $\mathbf{2}$ | $\mathbf{3}$ |
|  | $\mathbf{1}$ | 0 | 0 | $\mathbf{1}$ |
|  | $\mathbf{2}$ | 0 | 0 | 0 |
|  | $\mathbf{3}$ | 0 | $\mathbf{1}$ | 0 |

## Results

Results Summary for whole modelled period

| Arm | Max RFC | Max delay (s) | Max Queue (Veh) | Max LOS | Average Demand <br> (Veh/hr) | Total Junction <br> Arrivals (Veh) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | 0.69 | 10.83 | 2.2 | B | 747 | 1121 |
| $\mathbf{2}$ | 0.87 | 29.47 | 6.3 | $D$ | 792 | 1188 |
| $\mathbf{3}$ | 0.74 | 6.48 | 2.9 | A | 1600 | 2400 |

## Main Results for each time segment

17:00-17:15

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 747 | 187 | 1130 | 1085 | 0.689 | 738 | 463 | 0.0 | 2.1 | 10.157 | B |
| 2 | 792 | 198 | 666 | 917 | 0.864 | 771 | 1202 | 0.0 | 5.3 | 22.190 | C |
| 3 | 1600 | 400 | 4 | 2156 | 0.742 | 1589 | 1433 | 0.0 | 2.8 | 6.233 | A |

17:15-17:30

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 747 | 187 | 1138 | 1079 | 0.692 | 747 | 466 | 2.1 | 2.2 | 10.802 | B |
| 2 | 792 | 198 | 674 | 913 | 0.868 | 790 | 1211 | 5.3 | 5.8 | 28.207 | D |
| 3 | 1600 | 400 | 4 | 2156 | 0.742 | 1600 | 1459 | 2.8 | 2.8 | 6.472 | A |

17:30-17:45

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity <br> (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 747 | 187 | 1138 | 1079 | 0.692 | 747 | 466 | 2.2 | 2.2 | 10.820 | B |
| 2 | 792 | 198 | 674 | 913 | 0.868 | 791 | 1211 | 5.8 | 6.1 | 28.937 | D |
| 3 | 1600 | 400 | 4 | 2156 | 0.742 | 1600 | 1461 | 2.8 | 2.9 | 6.474 | A |

17:45-18:00

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity <br> (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 747 | 187 | 1138 | 1079 | 0.692 | 747 | 466 | 2.2 | 2.2 | 10.825 | B |
| 2 | 792 | 198 | 674 | 912 | 0.868 | 792 | 1211 | 6.1 | 6.2 | 29.222 | D |
| 3 | 1600 | 400 | 4 | 2156 | 0.742 | 1600 | 1462 | 2.9 | 2.9 | 6.477 | A |

18:00-18:15

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 747 | 187 | 1138 | 1079 | 0.692 | 747 | 466 | 2.2 | 2.2 | 10.827 | B |
| 2 | 792 | 198 | 674 | 912 | 0.868 | 792 | 1211 | 6.2 | 6.3 | 29.376 | D |
| 3 | 1600 | 400 | 4 | 2156 | 0.742 | 1600 | 1462 | 2.9 | 2.9 | 6.477 | A |

18:15-18:30

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 747 | 187 | 1138 | 1079 | 0.692 | 747 | 466 | 2.2 | 2.2 | 10.827 | B |
| 2 | 792 | 198 | 674 | 912 | 0.868 | 792 | 1211 | 6.3 | 6.3 | 29.470 | D |
| 3 | 1600 | 400 | 4 | 2156 | 0.742 | 1600 | 1462 | 2.9 | 2.9 | 6.477 | A |

## 2026 Baseline, AM

Data Errors and Warnings

| Severity | Area | Item | Description |
| :---: | :---: | :--- | :--- |
| Warning | Geometry | Arm 1 - Roundabout <br> Geometry | Effective flare length is over 30m, which is outside the normal range. Treat capacities with increasing caution. |

## Junction Network

## Junctions

| Junction | Name | Junction Type | Arm order | Junction Delay (s) | Junction LOS |
| :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | untitled | Standard Roundabout | $1,2,3$ | 289.54 | F |

## Junction Network Options

| Driving side | Lighting | Network residual capacity (\%) | First arm reaching threshold |
| :---: | :---: | :---: | :---: |
| Left | Normal/unknown | -17 | Arm 2 |

## Traffic Demand

## Demand Set Details

| ID | Scenario <br> name | Time Period <br> name | Traffic profile <br> type | Start time <br> $(\mathbf{H H}: \mathbf{m m})$ | Finish time <br> $(\mathbf{H H}: \mathbf{m m})$ | Time period length <br> $(\mathbf{m i n})$ | Time segment length <br> (min) | Run <br> automatically |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| D3 | 2026 Baseline | AM | FLAT | $07: 45$ | $09: 15$ | 90 | 15 |  |


| Vehicle mix varies over turn | Vehicle mix varies over entry | Vehicle mix source | PCU Factor for a HV (PCU) |
| :---: | :---: | :---: | :---: |
| $\checkmark$ | $\checkmark$ | HV Percentages | 2.00 |

Demand overview (Traffic)

| Arm | Linked arm | Profile type | Use O-D data | Average Demand (Veh/hr) | Scaling Factor (\%) |
| :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ |  | FLAT | $\checkmark$ | 1000 | 100.000 |
| $\mathbf{2}$ |  | FLAT | $\checkmark$ | 891 | 100.000 |
| $\mathbf{3}$ |  | FLAT | $\checkmark$ | 1051 | 100.000 |

## Origin-Destination Data

Demand (Veh/hr)

|  | To |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| From |  | $\mathbf{1}$ | $\mathbf{2}$ | $\mathbf{3}$ |
|  | $\mathbf{1}$ | 0 | 111 | 889 |
|  | $\mathbf{2}$ | 2 | 0 | 889 |
|  | $\mathbf{3}$ | 306 | 745 | 0 |

## Vehicle Mix

Heavy Vehicle Percentages

|  | To |  |  |  |
| :--- | :--- | :--- | :--- | :--- |
| From |  | $\mathbf{1}$ | $\mathbf{2}$ | $\mathbf{3}$ |
|  | $\mathbf{1}$ | 0 | 0 | $\mathbf{1}$ |
|  | $\mathbf{2}$ | 0 | 0 | 4 |
|  | $\mathbf{3}$ | $\mathbf{1}$ | $\mathbf{3}$ | 0 |

## Results

## Results Summary for whole modelled period

| Arm | Max RFC | Max delay (s) | Max Queue (Veh) | Max LOS | Average Demand <br> (Veh/hr) | Total Junction <br> Arrivals (Veh) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | 0.74 | 10.45 | 2.9 | B | 1000 | 1500 |
| $\mathbf{2}$ | 1.18 | 925.89 | 209.6 | F | 891 | 1337 |
| $\mathbf{3}$ | 0.50 | 3.36 | 1.0 | A | 1051 | 1576 |

## Main Results for each time segment

07:45-08:00

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 1000 | 250 | 742 | 1346 | 0.743 | 989 | 307 | 0.0 | 2.8 | 9.796 | A |
| 2 | 891 | 223 | 879 | 760 | 1.173 | 740 | 852 | 0.0 | 37.7 | 103.328 | F |
| 3 | 1051 | 263 | 2 | 2121 | 0.495 | 1047 | 1618 | 0.0 | 1.0 | 3.338 | A |

08:00-08:15

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 1000 | 250 | 745 | 1344 | 0.744 | 1000 | 308 | 2.8 | 2.8 | 10.427 | B |
| 2 | 891 | 223 | 889 | 754 | 1.181 | 753 | 856 | 37.7 | 72.3 | 274.449 | F |
| 3 | 1051 | 263 | 2 | 2121 | 0.495 | 1051 | 1640 | 1.0 | 1.0 | 3.362 | A |

08:15-08:30

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 1000 | 250 | 745 | 1344 | 0.744 | 1000 | 308 | 2.8 | 2.9 | 10.442 | B |
| 2 | 891 | 223 | 889 | 754 | 1.182 | 753 | 856 | 72.3 | 106.7 | 436.691 | F |
| 3 | 1051 | 263 | 2 | 2121 | 0.495 | 1051 | 1641 | 1.0 | 1.0 | 3.362 | A |

08:30-08:45

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 1000 | 250 | 745 | 1344 | 0.744 | 1000 | 308 | 2.9 | 2.9 | 10.448 | B |
| 2 | 891 | 223 | 889 | 754 | 1.182 | 754 | 856 | 106.7 | 141.0 | 599.523 | F |
| 3 | 1051 | 263 | 2 | 2121 | 0.495 | 1051 | 1641 | 1.0 | 1.0 | 3.362 | A |

08:45-09:00

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 1000 | 250 | 745 | 1344 | 0.744 | 1000 | 308 | 2.9 | 2.9 | 10.450 | B |
| 2 | 891 | 223 | 889 | 754 | 1.182 | 754 | 856 | 141.0 | 175.3 | 762.634 | F |
| 3 | 1051 | 263 | 2 | 2121 | 0.495 | 1051 | 1641 | 1.0 | 1.0 | 3.362 | A |

09:00-09:15

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity <br> (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 1000 | 250 | 745 | 1344 | 0.744 | 1000 | 308 | 2.9 | 2.9 | 10.452 | B |
| 2 | 891 | 223 | 889 | 754 | 1.182 | 754 | 856 | 175.3 | 209.6 | 925.886 | F |
| 3 | 1051 | 263 | 2 | 2121 | 0.495 | 1051 | 1641 | 1.0 | 1.0 | 3.362 | A |

## 2026 Baseline, PM

Data Errors and Warnings

| Severity | Area | Item | Description |
| :---: | :---: | :--- | :--- |
| Warning | Geometry | Arm 1 - Roundabout <br> Geometry | Effective flare length is over 30m, which is outside the normal range. Treat capacities with increasing caution. |

## Junction Network

## Junctions

| Junction | Name | Junction Type | Arm order | Junction Delay (s) | Junction LOS |
| :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | untitled | Standard Roundabout | $1,2,3$ | 17.54 | C |

## Junction Network Options

| Driving side | Lighting | Network residual capacity (\%) | First arm reaching threshold |
| :---: | :---: | :---: | :---: |
| Left | Normal/unknown | 0 | Arm 2 |

## Traffic Demand

## Demand Set Details

| ID | Scenario <br> name | Time Period <br> name | Traffic profile <br> type | Start time <br> $(\mathbf{H H}: \mathbf{m m})$ | Finish time <br> $(\mathbf{H H}: \mathbf{m m})$ | Time period length <br> $(\mathbf{m i n})$ | Time segment length <br> (min) | Run <br> automatically |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| D4 | 2026 Baseline | PM | FLAT | $17: 00$ | $18: 30$ | 90 | 15 |  |


| Vehicle mix varies over turn | Vehicle mix varies over entry | Vehicle mix source | PCU Factor for a HV (PCU) |
| :---: | :---: | :---: | :---: |
| $\checkmark$ | $\checkmark$ | HV Percentages | 2.00 |

Demand overview (Traffic)

| Arm | Linked arm | Profile type | Use O-D data | Average Demand (Veh/hr) | Scaling Factor (\%) |
| :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ |  | FLAT | $\checkmark$ | 756 | 100.000 |
| $\mathbf{2}$ |  | FLAT | $\checkmark$ | 801 | 100.000 |
| $\mathbf{3}$ |  | FLAT | $\checkmark$ | 1852 | 100.000 |

## Origin-Destination Data

Demand (Veh/hr)

|  | To |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| From |  | $\mathbf{1}$ | $\mathbf{2}$ | $\mathbf{3}$ |
|  | $\mathbf{1}$ | 0 | 73 | 683 |
|  | $\mathbf{2}$ | $\mathbf{4}$ | 0 | 797 |
|  | $\mathbf{3}$ | 592 | 1260 | 0 |

## Vehicle Mix

Heavy Vehicle Percentages

|  | To |  |  |  |
| :--- | :--- | :--- | :--- | :--- |
| From |  | $\mathbf{1}$ | $\mathbf{2}$ | $\mathbf{3}$ |
|  | $\mathbf{1}$ | 0 | 0 | $\mathbf{1}$ |
|  | $\mathbf{2}$ | 0 | 0 | 0 |
|  | $\mathbf{3}$ | 0 | $\mathbf{1}$ | 0 |

## Results

## Results Summary for whole modelled period

| Arm | Max RFC | Max delay (s) | Max Queue (Veh) | Max LOS | Average Demand <br> (Veh/hr) | Total Junction <br> Arrivals (Veh) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | 0.76 | 15.11 | 3.1 | C | 756 | 1134 |
| $\mathbf{2}$ | 0.88 | 33.23 | 7.2 | D | 801 | 1202 |
| $\mathbf{3}$ | 0.86 | 11.80 | 6.0 | $B$ | 1852 | 2778 |

## Main Results for each time segment

17:00-17:15

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 756 | 189 | 1245 | 1005 | 0.753 | 745 | 589 | 0.0 | 2.9 | 13.318 | B |
| 2 | 801 | 200 | 673 | 913 | 0.877 | 778 | 1317 | 0.0 | 5.8 | 23.643 | C |
| 3 | 1852 | 463 | 4 | 2156 | 0.859 | 1830 | 1447 | 0.0 | 5.6 | 10.396 | B |

17:15-17:30

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 756 | 189 | 1259 | 994 | 0.760 | 755 | 596 | 2.9 | 3.0 | 14.966 | B |
| 2 | 801 | 200 | 682 | 907 | 0.883 | 798 | 1332 | 5.8 | 6.5 | 31.198 | D |
| 3 | 1852 | 463 | 4 | 2156 | 0.859 | 1851 | 1476 | 5.6 | 5.8 | 11.702 | B |

17:30-17:45

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 756 | 189 | 1260 | 994 | 0.761 | 756 | 596 | 3.0 | 3.1 | 15.063 | C |
| 2 | 801 | 200 | 683 | 907 | 0.883 | 800 | 1333 | 6.5 | 6.8 | 32.345 | D |
| 3 | 1852 | 463 | 4 | 2156 | 0.859 | 1852 | 1479 | 5.8 | 5.9 | 11.760 | B |

17:45-18:00

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 756 | 189 | 1260 | 994 | 0.761 | 756 | 596 | 3.1 | 3.1 | 15.089 | C |
| 2 | 801 | 200 | 683 | 907 | 0.883 | 800 | 1333 | 6.8 | 7.0 | 32.812 | D |
| 3 | 1852 | 463 | 4 | 2156 | 0.859 | 1852 | 1479 | 5.9 | 5.9 | 11.780 | B |

18:00-18:15

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 756 | 189 | 1260 | 994 | 0.761 | 756 | 596 | 3.1 | 3.1 | 15.100 | C |
| 2 | 801 | 200 | 683 | 907 | 0.883 | 801 | 1333 | 7.0 | 7.1 | 33.066 | D |
| 3 | 1852 | 463 | 4 | 2156 | 0.859 | 1852 | 1480 | 5.9 | 6.0 | 11.792 | B |

18:15-18:30

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 756 | 189 | 1260 | 994 | 0.761 | 756 | 596 | 3.1 | 3.1 | 15.107 | C |
| 2 | 801 | 200 | 683 | 907 | 0.883 | 801 | 1333 | 7.1 | 7.2 | 33.226 | D |
| 3 | 1852 | 463 | 4 | 2156 | 0.859 | 1852 | 1480 | 6.0 | 6.0 | 11.799 | B |

## 2026 BTM + 60sqm, AM

Data Errors and Warnings

| Severity | Area | Item | Description |
| :---: | :---: | :--- | :--- |
| Warning | Geometry | Arm 1 - Roundabout <br> Geometry | Effective flare length is over 30m, which is outside the normal range. Treat capacities with increasing caution. |

## Junction Network

## Junctions

| Junction | Name | Junction Type | Arm order | Junction Delay (s) | Junction LOS |
| :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | untitled | Standard Roundabout | $1,2,3$ | 410.61 | F |

## Junction Network Options

| Driving side | Lighting | Network residual capacity (\%) | First arm reaching threshold |
| :---: | :---: | :---: | :---: |
| Left | Normal/unknown | -20 | Arm 2 |

## Traffic Demand

## Demand Set Details

| ID | Scenario name | Time Period <br> name | Traffic profile <br> type | Start time <br> $(\mathbf{H H}: \mathbf{m m})$ | Finish time <br> $(\mathbf{H H}: \mathbf{m m})$ | Time period length <br> $(\mathbf{m i n})$ | Time segment length <br> $(\mathbf{m i n})$ | Run <br> automatically |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| D5 | 2026 BTM +60 sqm | AM | FLAT | $07: 45$ | $09: 15$ | 90 | 15 |  |


| Vehicle mix varies over turn | Vehicle mix varies over entry | Vehicle mix source | PCU Factor for a HV (PCU) |
| :---: | :---: | :---: | :---: |
| $\checkmark$ | $\checkmark$ | HV Percentages | 2.00 |

Demand overview (Traffic)

| Arm | Linked arm | Profile type | Use O-D data | Average Demand (Veh/hr) | Scaling Factor (\%) |
| :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ |  | FLAT | $\checkmark$ | 1041 | 100.000 |
| $\mathbf{2}$ |  | FLAT | $\checkmark$ | 930 | 100.000 |
| $\mathbf{3}$ |  | FLAT | $\checkmark$ | 1059 | 100.000 |

## Origin-Destination Data

Demand (Veh/hr)

|  | To |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| From |  | $\mathbf{1}$ | $\mathbf{2}$ | $\mathbf{3}$ |
|  | $\mathbf{1}$ | 0 | 111 | 930 |
|  | $\mathbf{2}$ | 2 | 0 | 928 |
|  | $\mathbf{3}$ | 310 | 749 | 0 |

## Vehicle Mix

Heavy Vehicle Percentages

|  | To |  |  |  |
| :--- | :--- | :--- | :--- | :--- |
| From |  | $\mathbf{1}$ | $\mathbf{2}$ | $\mathbf{3}$ |
|  | $\mathbf{1}$ | 0 | 0 | $\mathbf{1}$ |
|  | $\mathbf{2}$ | 0 | 0 | 3 |
|  | $\mathbf{3}$ | $\mathbf{1}$ | $\mathbf{3}$ | 0 |

## Results

## Results Summary for whole modelled period

| Arm | Max RFC | Max delay (s) | Max Queue (Veh) | Max LOS | Average Demand <br> (Veh/hr) | Total Junction <br> Arrivals (Veh) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | 0.78 | 11.97 | 3.4 | B | 1041 | 1562 |
| $\mathbf{2}$ | 1.26 | 1308.81 | 290.9 | F | 930 | 1395 |
| $\mathbf{3}$ | 0.50 | 3.39 | 1.0 | A | 1059 | 1588 |

## Main Results for each time segment

07:45-08:00

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 1041 | 260 | 746 | 1343 | 0.775 | 1028 | 310 | 0.0 | 3.3 | 11.004 | B |
| 2 | 930 | 232 | 918 | 744 | 1.250 | 730 | 856 | 0.0 | 50.1 | 133.677 | F |
| 3 | 1059 | 265 | 2 | 2122 | 0.499 | 1055 | 1646 | 0.0 | 1.0 | 3.363 | A |

08:00-08:15

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 1041 | 260 | 749 | 1341 | 0.776 | 1041 | 312 | 3.3 | 3.4 | 11.926 | B |
| 2 | 930 | 232 | 930 | 738 | 1.261 | 737 | 860 | 50.1 | 98.3 | 372.433 | F |
| 3 | 1059 | 265 | 2 | 2122 | 0.499 | 1059 | 1665 | 1.0 | 1.0 | 3.387 | A |

08:15-08:30

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 1041 | 260 | 749 | 1341 | 0.776 | 1041 | 312 | 3.4 | 3.4 | 11.957 | B |
| 2 | 930 | 232 | 930 | 738 | 1.261 | 737 | 860 | 98.3 | 146.5 | 605.689 | F |
| 3 | 1059 | 265 | 2 | 2122 | 0.499 | 1059 | 1666 | 1.0 | 1.0 | 3.387 | A |

08:30-08:45

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 1041 | 260 | 749 | 1341 | 0.776 | 1041 | 312 | 3.4 | 3.4 | 11.966 | B |
| 2 | 930 | 232 | 930 | 738 | 1.261 | 737 | 860 | 146.5 | 194.6 | 839.795 | F |
| 3 | 1059 | 265 | 2 | 2122 | 0.499 | 1059 | 1666 | 1.0 | 1.0 | 3.387 | A |

08:45-09:00

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 1041 | 260 | 749 | 1341 | 0.776 | 1041 | 312 | 3.4 | 3.4 | 11.971 | B |
| 2 | 930 | 232 | 930 | 738 | 1.261 | 737 | 860 | 194.6 | 242.7 | 1074.224 | F |
| 3 | 1059 | 265 | 2 | 2122 | 0.499 | 1059 | 1666 | 1.0 | 1.0 | 3.387 | A |

09:00-09:15

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 1041 | 260 | 749 | 1341 | 0.776 | 1041 | 312 | 3.4 | 3.4 | 11.973 | B |
| 2 | 930 | 232 | 930 | 738 | 1.261 | 737 | 860 | 242.7 | 290.9 | 1308.814 | F |
| 3 | 1059 | 265 | 2 | 2122 | 0.499 | 1059 | 1666 | 1.0 | 1.0 | 3.387 | A |

## 2026 BTM + 60sqm, PM

Data Errors and Warnings

| Severity | Area | Item | Description |
| :---: | :--- | :--- | :--- |
| Warning | Geometry | Arm 1 - Roundabout <br> Geometry | Effective flare length is over 30m, which is outside the normal range. Treat capacities with increasing caution. |

## Junction Network

## Junctions

| Junction | Name | Junction Type | Arm order | Junction Delay (s) | Junction LOS |
| :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | untitled | Standard Roundabout | $1,2,3$ | 20.61 | C |

## Junction Network Options

| Driving side | Lighting | Network residual capacity (\%) | First arm reaching threshold |
| :---: | :---: | :---: | :---: |
| Left | Normal/unknown | 0 | Arm 2 |

## Traffic Demand

## Demand Set Details

| ID | Scenario name | Time Period <br> name | Traffic profile <br> type | Start time <br> $(H H: m m)$ | Finish time <br> $(H H: m m)$ | Time period length <br> $(\mathbf{m i n})$ | Time segment length <br> $(\mathbf{m i n})$ | Run <br> automatically |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| D6 | 2026 BTM +60 sqm | PM | FLAT | $17: 00$ | $18: 30$ | 90 | 15 |  |


| Vehicle mix varies over turn | Vehicle mix varies over entry | Vehicle mix source | PCU Factor for a HV (PCU) |
| :---: | :---: | :---: | :---: |
| $\checkmark$ | $\checkmark$ | HV Percentages | 2.00 |

Demand overview (Traffic)

| Arm | Linked arm | Profile type | Use O-D data | Average Demand (Veh/hr) | Scaling Factor (\%) |
| :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ |  | FLAT | $\checkmark$ | 759 | 100.000 |
| $\mathbf{2}$ |  | FLAT | $\checkmark$ | 803 | 100.000 |
| $\mathbf{3}$ |  | FLAT | $\checkmark$ | 1936 | 100.000 |

## Origin-Destination Data

Demand (Veh/hr)

|  | To |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| From |  | $\mathbf{1}$ | $\mathbf{2}$ | $\mathbf{3}$ |
|  | $\mathbf{1}$ | 0 | 73 | 686 |
|  | $\mathbf{2}$ | $\mathbf{4}$ | 0 | 799 |
|  | $\mathbf{3}$ | 635 | 1301 | 0 |

## Vehicle Mix

Heavy Vehicle Percentages

|  | To |  |  |  |
| :--- | :--- | :--- | :--- | :--- |
| From |  | $\mathbf{1}$ | $\mathbf{2}$ | $\mathbf{3}$ |
|  | $\mathbf{1}$ | 0 | 0 | $\mathbf{1}$ |
|  | $\mathbf{2}$ | 0 | 0 | 0 |
|  | $\mathbf{3}$ | 0 | $\mathbf{1}$ | 0 |

## Results

## Results Summary for whole modelled period

| Arm | Max RFC | Max delay (s) | Max Queue (Veh) | Max LOS | Average Demand <br> (Veh/hr) | Total Junction <br> Arrivals (Veh) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | 0.79 | 17.41 | 3.6 | C | 759 | 1138 |
| $\mathbf{2}$ | 0.89 | 34.37 | 7.4 | $D$ | 803 | 1205 |
| $\mathbf{3}$ | 0.90 | 16.20 | 8.5 | $C$ | 1936 | 2904 |

## Main Results for each time segment

17:00-17:15

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 759 | 190 | 1281 | 980 | 0.775 | 746 | 629 | 0.0 | 3.2 | 14.721 | B |
| 2 | 803 | 201 | 674 | 912 | 0.880 | 780 | 1352 | 0.0 | 5.9 | 24.016 | C |
| 3 | 1936 | 484 | 4 | 2157 | 0.898 | 1906 | 1450 | 0.0 | 7.6 | 13.077 | B |

17:15-17:30

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 759 | 190 | 1300 | 966 | 0.786 | 758 | 638 | 3.2 | 3.5 | 17.115 | C |
| 2 | 803 | 201 | 685 | 906 | 0.886 | 800 | 1373 | 5.9 | 6.7 | 32.032 | D |
| 3 | 1936 | 484 | 4 | 2157 | 0.898 | 1934 | 1481 | 7.6 | 8.1 | 15.816 | C |

17:30-17:45

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 759 | 190 | 1300 | 966 | 0.786 | 759 | 639 | 3.5 | 3.5 | 17.310 | C |
| 2 | 803 | 201 | 686 | 905 | 0.887 | 802 | 1373 | 6.7 | 7.0 | 33.341 | D |
| 3 | 1936 | 484 | 4 | 2157 | 0.898 | 1935 | 1483 | 8.1 | 8.3 | 16.034 | C |

17:45-18:00

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 759 | 190 | 1301 | 965 | 0.786 | 759 | 639 | 3.5 | 3.6 | 17.366 | C |
| 2 | 803 | 201 | 686 | 905 | 0.887 | 802 | 1374 | 7.0 | 7.2 | 33.883 | D |
| 3 | 1936 | 484 | 4 | 2157 | 0.898 | 1936 | 1484 | 8.3 | 8.4 | 16.120 | C |

18:00-18:15

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 759 | 190 | 1301 | 965 | 0.786 | 759 | 639 | 3.6 | 3.6 | 17.392 | C |
| 2 | 803 | 201 | 686 | 905 | 0.887 | 803 | 1374 | 7.2 | 7.3 | 34.179 | D |
| 3 | 1936 | 484 | 4 | 2157 | 0.898 | 1936 | 1484 | 8.4 | 8.5 | 16.168 | C |

18:15-18:30

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 759 | 190 | 1301 | 965 | 0.786 | 759 | 639 | 3.6 | 3.6 | 17.408 | C |
| 2 | 803 | 201 | 686 | 905 | 0.887 | 803 | 1374 | 7.3 | 7.4 | 34.366 | D |
| 3 | 1936 | 484 | 4 | 2157 | 0.898 | 1936 | 1485 | 8.5 | 8.5 | 16.196 | C |

## Junctions 9

| Junctions 9 |
| :---: |
| ARCADY 9 - Roundabout Module |
| Version: 9.0.2.5947 <br> © Copyright TRL Limited, 2017 |
| For sales and distribution information, program advice and maintenance, contact TRL: +44 (0)1344770558 software@trl.co.uk www.trlsoftware.co.uk |
| The users of this computer program for the solution of an engineering problem are in no way relieved of their responsibility for the correctness of the solution |

Filename: Middleton Stoney - Kings End - Oxford Road - 2018-06-21 (With Mitgation).j9
Path: N:\Projects\Imbic2 170211\Analysis\Modelling\Middleton Stoney
Report generation date: 25/06/2018 14:32:59

```
"2026 BTM + 60sqm, AM
"2026 BTM + 60sqm, PM
```


## Summary of junction performance

|  | AM |  |  |  |  |  |  | PM |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Queue <br> (Veh) | Delay (s) | RFC | LOS | Junction Delay (s) | Junction LOS | Network Residual Capacity | Queue (Veh) | Delay <br> (s) | RFC | LOS | Junction <br> Delay (s) | Junction LOS | Network Residual Capacity |
|  | 2026 BTM + 60sqm |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Arm 1 | 3.4 | 11.97 | 0.78 | B | 16.90 | C | -1\% | 3.6 | 17.41 | 0.79 | C | 14.75 | B | 6 \% |
| Arm 2 | 9.4 | 37.61 | 0.91 | E |  |  | [Arm 2] | 1.9 | 8.69 | 0.66 | A |  |  |  |
| Arm 3 | 1.0 | 3.39 | 0.50 | A |  |  |  | 8.5 | 16.20 | 0.90 | C |  |  | [Arm 1] |

There are warnings associated with one or more model runs - see the 'Data Errors and Warnings' tables for each Analysis or Demand Set.
Values shown are the highest values encountered over all time segments. Delay is the maximum value of average delay per arriving vehicle. Junction LOS and Junction Delay are demand-weighted averages. Network Residual Capacity indicates the amount by which network flow could be increased before a user-definable threshold (see Analysis Options) is met.

File summary
File Description

| Title | Middleton Stoney - Kings End - Oxford Road |
| :--- | :--- |
| Location | Bicester |
| Site number |  |
| Date | $15 / 06 / 2017$ |
| Version |  |
| Status | (new file) |
| Identifier |  |
| Client |  |
| Jobnumber |  |
| Enumerator | MOTIONTklewis |
| Description |  |

Units

| Distance units | Speed units | Traffic units input | Traffic units results | Flow units | Average delay units | Total delay units | Rate of delay units |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| m | kph | Veh | Veh | perHour | S | -Min | perMin |

## Analysis Options

| Vehicle <br> length $(\mathbf{m})$ | Calculate Queue <br> Percentiles | Calculate detailed <br> queueing delay | Calculate residual <br> capacity | Residual capacity <br> criteria type | RFC <br> Threshold | Average Delay <br> threshold (s) | Queue threshold <br> (PCU) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 5.75 |  |  | $\checkmark$ | Delay | 0.85 | 36.00 | 20.00 |

## Demand Set Summary

| ID | Scenario name | Time Period name | Traffic profile type | Start time (HH:mm) | Finish time (HH:mm) | Time period length (min) | Time segment length (min) | Run automatically |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| D1 | 2026 BTM + 60sqm | AM | FLAT | 07:45 | 09:15 | 90 | 15 | $\checkmark$ |
| D2 | 2026 BTM + 60sqm | PM | FLAT | 17:00 | 18:30 | 90 | 15 | $\checkmark$ |

## Analysis Set Details

| ID | Include in report | Network flow scaling factor (\%) | Network capacity scaling factor (\%) |
| :---: | :---: | :---: | :---: |
| A1 | $\checkmark$ | 100.000 | 100.000 |

THE FUTURE

## 2026 BTM + 60sqm, AM

Data Errors and Warnings

| Severity | Area | Item | Description |
| :---: | :--- | :--- | :--- |
| Warning | Geometry | Arm 1 - Roundabout <br> Geometry | Effective flare length is over 30m, which is outside the normal range. Treat capacities with increasing caution. |

## Junction Network

## Junctions

| Junction | Name | Junction Type | Arm order | Junction Delay (s) | Junction LOS |
| :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | untitled | Standard Roundabout | $1,2,3$ | 16.90 | C |

## Junction Network Options

| Driving side | Lighting | Network residual capacity (\%) | First arm reaching threshold |
| :---: | :---: | :---: | :---: |
| Left | Normal/unknown | -1 | Arm 2 |

## Arms

## Arms

| Arm | Name | Description |
| :---: | :--- | :--- |
| $\mathbf{1}$ | Middleton Stoney |  |
| $\mathbf{2}$ | Kings End |  |
| $\mathbf{3}$ | Oxford Road |  |

Roundabout Geometry

| Arm | V - Approach road half- <br> width $(\mathbf{m})$ | E - Entry width <br> $(\mathbf{m})$ | I' - Effective flare <br> length $(\mathbf{m})$ | R - Entry radius <br> $(\mathbf{m})$ | D - Inscribed circle <br> diameter $(\mathbf{m})$ | PHI - Conflict (entry) <br> angle (deg) | Exit <br> only |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | 3.50 | 7.50 | 32.0 | 20.0 | 19.0 |  |  |
| $\mathbf{2}$ | 3.50 | 7.50 | 12.0 | 80.0 | 19.0 |  |  |
| $\mathbf{3}$ | 7.50 | 7.50 | 0.0 | 17.0 | 19.0 |  |  |

## Slope / Intercept / Capacity

Roundabout Slope and Intercept used in model

| Arm | Final slope | Final intercept (PCU/hr) |
| :---: | :---: | :---: |
| $\mathbf{1}$ | 0.699 | 1893 |
| $\mathbf{2}$ | 0.667 | 1679 |
| $\mathbf{3}$ | 0.749 | 2174 |

The slope and intercept shown above include any corrections and adjustments.

## Traffic Demand

## Demand Set Details

| ID | Scenario name | Time Period <br> name | Traffic profile <br> type | Start time <br> (HH:mm) | Finish time <br> (HH:mm) | Time period length <br> (min) | Time segment length <br> (min) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| D1 | 2026 BTM +60 sqm | AM | FLAT | $07: 45$ | Run <br> automatically |  |  |


| Vehicle mix varies over turn | Vehicle mix varies over entry | Vehicle mix source | PCU Factor for a HV (PCU) |
| :---: | :---: | :---: | :---: |
| $\checkmark$ | $\checkmark$ | HV Percentages | 2.00 |

THE FUTURE

Demand overview (Traffic)

| Arm | Linked arm | Profile type | Use O-D data | Average Demand (Veh/hr) | Scaling Factor (\%) |
| :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ |  | FLAT | $\checkmark$ | 1041 | 100.000 |
| $\mathbf{2}$ |  | FLAT | $\checkmark$ | 930 | 100.000 |
| $\mathbf{3}$ |  | FLAT | $\checkmark$ | 1059 | 100.000 |

## Origin-Destination Data

Demand (Veh/hr)

|  | To |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| From |  | $\mathbf{1}$ | $\mathbf{2}$ | $\mathbf{3}$ |
|  | $\mathbf{1}$ | 0 | 111 | 930 |
|  | $\mathbf{2}$ | 2 | 0 | 928 |
|  | $\mathbf{3}$ | 310 | 749 | 0 |

## Vehicle Mix

Heavy Vehicle Percentages

|  | To |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| From |  | $\mathbf{1}$ | $\mathbf{2}$ | $\mathbf{3}$ |
|  | $\mathbf{1}$ | 0 | 0 | 1 |
|  | $\mathbf{2}$ | 0 | 0 | 3 |
|  | $\mathbf{3}$ | $\mathbf{1}$ | $\mathbf{3}$ | 0 |

## Results

Results Summary for whole modelled period

| Arm | Max RFC | Max delay (s) | Max Queue (Veh) | Max LOS | Average Demand <br> (Veh/hr) | Total Junction <br> Arrivals (Veh) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | 0.78 | 11.97 | 3.4 | B | 1041 | 1562 |
| $\mathbf{2}$ | 0.91 | 37.61 | 9.4 | E | 930 | 1395 |
| $\mathbf{3}$ | 0.50 | 3.39 | 1.0 | A | 1059 | 1588 |

## Main Results for each time segment

07:45-08:00

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity <br> (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 1041 | 260 | 746 | 1343 | 0.775 | 1028 | 311 | 0.0 | 3.3 | 11.003 | B |
| 2 | 930 | 232 | 918 | 1030 | 0.903 | 902 | 856 | 0.0 | 7.0 | 24.395 | C |
| 3 | 1059 | 265 | 2 | 2121 | 0.499 | 1055 | 1818 | 0.0 | 1.0 | 3.364 | A |

08:00-08:15

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 1041 | 260 | 749 | 1341 | 0.776 | 1041 | 312 | 3.3 | 3.4 | 11.926 | B |
| 2 | 930 | 232 | 930 | 1022 | 0.910 | 925 | 860 | 7.0 | 8.2 | 34.121 | D |
| 3 | 1059 | 265 | 2 | 2121 | 0.499 | 1059 | 1853 | 1.0 | 1.0 | 3.387 | A |

08:15-08:30

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 1041 | 260 | 749 | 1341 | 0.776 | 1041 | 312 | 3.4 | 3.4 | 11.957 | B |
| 2 | 930 | 232 | 930 | 1022 | 0.910 | 928 | 860 | 8.2 | 8.8 | 35.995 | E |
| 3 | 1059 | 265 | 2 | 2121 | 0.499 | 1059 | 1856 | 1.0 | 1.0 | 3.387 | A |

08:30-08:45

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity <br> (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 1041 | 260 | 749 | 1341 | 0.776 | 1041 | 312 | 3.4 | 3.4 | 11.966 | B |
| 2 | 930 | 232 | 930 | 1022 | 0.910 | 929 | 860 | 8.8 | 9.0 | 36.829 | E |
| 3 | 1059 | 265 | 2 | 2121 | 0.499 | 1059 | 1857 | 1.0 | 1.0 | 3.387 | A |

08:45-09:00

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity <br> (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 1041 | 260 | 749 | 1341 | 0.776 | 1041 | 312 | 3.4 | 3.4 | 11.971 | B |
| 2 | 930 | 232 | 930 | 1022 | 0.910 | 929 | 860 | 9.0 | 9.2 | 37.302 | E |
| 3 | 1059 | 265 | 2 | 2121 | 0.499 | 1059 | 1857 | 1.0 | 1.0 | 3.388 | A |

09:00-09:15

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity <br> (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 1041 | 260 | 749 | 1341 | 0.776 | 1041 | 312 | 3.4 | 3.4 | 11.973 | B |
| 2 | 930 | 232 | 930 | 1022 | 0.910 | 929 | 860 | 9.2 | 9.4 | 37.611 | E |
| 3 | 1059 | 265 | 2 | 2121 | 0.499 | 1059 | 1857 | 1.0 | 1.0 | 3.388 | A |

## 2026 BTM + 60sqm, PM

Data Errors and Warnings

| Severity | Area | Item | Description |
| :---: | :--- | :--- | :--- |
| Warning | Geometry | Arm 1 - Roundabout <br> Geometry | Effective flare length is over 30m, which is outside the normal range. Treat capacities with increasing caution. |

## Junction Network

## Junctions

| Junction | Name | Junction Type | Arm order | Junction Delay (s) | Junction LOS |
| :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | untitled | Standard Roundabout | $1,2,3$ | 14.75 | $B$ |

## Junction Network Options

| Driving side | Lighting | Network residual capacity (\%) | First arm reaching threshold |
| :---: | :---: | :---: | :---: |
| Left | Normal/unknown | 6 | Arm 1 |

## Traffic Demand

## Demand Set Details

| ID | Scenario name | Time Period <br> name | Traffic profile <br> type | Start time <br> (HH:mm) | Finish time <br> (HH:mm) | Time period length <br> $(\mathbf{m i n})$ | Time segment length <br> (min) | Run <br> automatically |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| D2 | 2026 BTM +60 sqm | PM | FLAT | $17: 00$ | $18: 30$ | 90 | 15 |  |


| Vehicle mix varies over turn | Vehicle mix varies over entry | Vehicle mix source | PCU Factor for a HV (PCU) |
| :---: | :---: | :---: | :---: |
| $\checkmark$ | $\checkmark$ | HV Percentages | 2.00 |

Demand overview (Traffic)

| Arm | Linked arm | Profile type | Use O-D data | Average Demand (Veh/hr) | Scaling Factor (\%) |
| :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ |  | FLAT | $\checkmark$ | 759 | 100.000 |
| $\mathbf{2}$ |  | FLAT | $\checkmark$ | 803 | 100.000 |
| $\mathbf{3}$ |  | FLAT | $\checkmark$ | 1936 | 100.000 |

## Origin-Destination Data

Demand (Veh/hr)

|  | To |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| From |  | $\mathbf{1}$ | $\mathbf{2}$ | $\mathbf{3}$ |
|  | $\mathbf{1}$ | 0 | 73 | 686 |
|  | $\mathbf{2}$ | $\mathbf{4}$ | 0 | 799 |
|  | $\mathbf{3}$ | 635 | 1301 | 0 |

## Vehicle Mix

Heavy Vehicle Percentages

|  | To |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| From |  | $\mathbf{1}$ | $\mathbf{2}$ | $\mathbf{3}$ |
|  | $\mathbf{1}$ | 0 | 0 | $\mathbf{1}$ |
|  | $\mathbf{2}$ | 0 | 0 | 0 |
|  | $\mathbf{3}$ | 0 | $\mathbf{1}$ | 0 |

## Results

## Results Summary for whole modelled period

| Arm | Max RFC | Max delay (s) | Max Queue (Veh) | Max LOS | Average Demand <br> (Veh/hr) | Total Junction <br> Arrivals (Veh) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | 0.79 | 17.41 | 3.6 | C | 759 | 1138 |
| $\mathbf{2}$ | 0.66 | 8.69 | 1.9 | A | 803 | 1205 |
| $\mathbf{3}$ | 0.90 | 16.20 | 8.5 | C | 1936 | 2904 |

## Main Results for each time segment

17:00-17:15

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 759 | 190 | 1281 | 980 | 0.775 | 746 | 629 | 0.0 | 3.2 | 14.721 | B |
| 2 | 803 | 201 | 674 | 1225 | 0.656 | 796 | 1352 | 0.0 | 1.9 | 8.253 | A |
| 3 | 1936 | 484 | 4 | 2157 | 0.898 | 1906 | 1466 | 0.0 | 7.6 | 13.078 | B |

17:15-17:30

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 759 | 190 | 1300 | 966 | 0.786 | 758 | 638 | 3.2 | 3.5 | 17.115 | C |
| 2 | 803 | 201 | 685 | 1218 | 0.659 | 803 | 1373 | 1.9 | 1.9 | 8.669 | A |
| 3 | 1936 | 484 | 4 | 2157 | 0.898 | 1934 | 1484 | 7.6 | 8.1 | 15.817 | C |

17:30-17:45

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 759 | 190 | 1300 | 966 | 0.786 | 759 | 639 | 3.5 | 3.5 | 17.310 | C |
| 2 | 803 | 201 | 686 | 1217 | 0.660 | 803 | 1373 | 1.9 | 1.9 | 8.686 | A |
| 3 | 1936 | 484 | 4 | 2157 | 0.898 | 1935 | 1485 | 8.1 | 8.3 | 16.034 | C |

17:45-18:00

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 759 | 190 | 1301 | 965 | 0.786 | 759 | 639 | 3.5 | 3.6 | 17.366 | C |
| 2 | 803 | 201 | 686 | 1217 | 0.660 | 803 | 1374 | 1.9 | 1.9 | 8.690 | A |
| 3 | 1936 | 484 | 4 | 2157 | 0.898 | 1936 | 1485 | 8.3 | 8.4 | 16.120 | C |

18:00-18:15

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 759 | 190 | 1301 | 965 | 0.786 | 759 | 639 | 3.6 | 3.6 | 17.392 | C |
| 2 | 803 | 201 | 686 | 1217 | 0.660 | 803 | 1374 | 1.9 | 1.9 | 8.692 | A |
| 3 | 1936 | 484 | 4 | 2157 | 0.898 | 1936 | 1485 | 8.4 | 8.5 | 16.168 | C |

18:15-18:30

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 759 | 190 | 1301 | 965 | 0.786 | 759 | 639 | 3.6 | 3.6 | 17.408 | C |
| 2 | 803 | 201 | 686 | 1217 | 0.660 | 803 | 1374 | 1.9 | 1.9 | 8.693 | A |
| 3 | 1936 | 484 | 4 | 2157 | 0.898 | 1936 | 1485 | 8.5 | 8.5 | 16.196 | C |

A41 Highway Network

Basic Results Summary Basic Results Summary

## User and Project Details

| Project: | Bicester Office Park |
| :--- | :--- |
| Title: |  |
| Location: | Scenic Land Developments Ltd |
| Client: |  |
| Additional detail: | Oxford Road Model (inc BG Improvements) - 2018-04-30 Base (inc BV4).lsg3x |
| File name: |  |
| Author: | Motion |
| Company: <br> Address: |  |

Scenario 1: '2026 PM BTM + Committed' (FG11: '2026 PM BTM + Committed', Plan 1: 'Control Plan') Network Layout Diagram


Basic Results Summary
Network Results

| Item | Lane Description | Lane <br> Type | Full Phase | Arrow Phase | Num Greens | Total Green (s) | Arrow Green (s) | Demand <br> Flow <br> (pcu) | Sat Flow (pcu/Hr) | Capacity (pcu) | $\begin{aligned} & \text { Deg } \\ & \text { Sat (\%) } \end{aligned}$ | Turners In Gaps (pcu) | Turners When Unopposed (pcu) | Turners In Intergreen (pcu) | Total Delay (pcuHr) | Av. <br> Delay <br> Per PCU <br> (s/pcu) | Mean Max Queue (pcu) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network | - | - | - |  | - | - | - | - | - | - | 100.3\% | 71 | 27 | 0 | 177.4 | - | - |
| J1: Pingle Drive / Oxford | - | - | - |  | - | - | - | - | - | - | 58.7\% | 0 | 0 | 0 | 19.8 | - | - |
| 1/1 | Oxford Road (nb) Ahead | U | C1:A |  | 2 | 188 | - | 702 | 1915 | 1516 | 46.3\% | - | - | - | 0.8 | 4.0 | 3.6 |
| 1/2 | Oxford Road (nb) Ahead | U | C1:A |  | 2 | 188 | - | 778 | 2055 | 1627 | 47.8\% | - | - | - | 0.8 | 3.9 | 4.1 |
| 1/3 | Oxford Road (nb) Right | U | C1:F |  | 2 | 74 | - | 118 | 2042 | 647 | 18.2\% | - | - | - | 1.0 | 31.6 | 3.3 |
| 1/4 | Oxford Road (nb) Right | U | C1:F |  | 2 | 74 | - | 134 | 2042 | 647 | 20.7\% | - | - | - | 1.0 | 27.2 | 3.6 |
| 2/2+2/1 | Pingle Drive Left | U | C1:E |  | 2 | 88 | - | 465 | 2005:1870 | 525+317 | $\begin{gathered} 55.2 \text { : } \\ 55.2 \% \end{gathered}$ | - | - | - | 4.1 | 31.9 | 9.0 |
| 2/3 | Pingle Drive Right | U | C1:D |  | 2 | 24 | - | 131 | 2067 | 224 | 58.5\% | - | - | - | 2.5 | 70.1 | 5.0 |
| 3/1 | Oxford Road (sb) Left | U | C1:C |  | 2 | 150 | - | 42 | 1908 | 1208 | 3.5\% | - | - | - | 0.1 | 9.8 | 0.5 |
| 3/2 | Oxford Road (sb) Ahead | U | C1:B |  | 2 | 128 | - | 656 | 2105 | 1140 | 57.5\% | - | - | - | 4.0 | 22.0 | 15.6 |
| 3/3 | Oxford Road (sb) Ahead | U | C1:B |  | 2 | 128 | - | 669 | 2105 | 1140 | 58.7\% | - | - | - | 4.1 | 22.3 | 16.1 |
| 4/1 | Oxford Road (nb) | U | - |  | - | - | - | 702 | 1940 | 1940 | 36.2\% | - | - | - | 0.3 | 1.5 | 0.3 |
| 4/2 | Oxford Road (nb) | U | - |  | - | - | - | 909 | 2080 | 2080 | 43.7\% | - | - | - | 0.4 | 1.5 | 0.4 |
| 5/1 | Pingle Drive | U | - |  | - | - | - | 160 | 1965 | 1965 | 8.1\% | - | - | - | 0.0 | 1.0 | 0.0 |
| 5/2 | Pingle Drive | U | - |  | - | - | - | 134 | 2105 | 2105 | 6.4\% | - | - | - | 0.0 | 0.9 | 0.0 |
| 6/1 | Right Turn Lane Right | U | C1:G |  | 2 | 68 | - | 118 | 1980 | 578 | 20.4\% | - | - | - | 0.2 | 6.2 | 0.2 |
| 6/2 | Right Turn Lane Right | U | C1:G |  | 2 | 68 | - | 134 | 1980 | 578 | 23.2\% | - | - | - | 0.3 | 6.9 | 0.3 |
| Ped Link: P1 | Unnamed Ped Link | - | C1:H |  | 2 | 56 | - | 0 | - | 0 | 0.0\% | - | - | - | - | - | - |


| Ped Link: P2 | Unnamed Ped Link | - | C1:I | 2 | 136 | - | 0 | - | 40800 | 0.0\% | - | - | - | 0.0 | 0.0 | 0.0 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Ped Link: P3 | Unnamed Ped Link | - | C1:J | 2 | 116 | - | 0 | - | 0 | 0.0\% | - | - |  | - | - | - |
| J2: Esso Roundabout | - | - | - | - | - | - | - | - | - | 100.3\% | 71 | 27 | 0 | 92.0 | - | - |
| 1/1 | Oxford Road Left Ahead | U | C2:A | 2 | 173 | - | 605 | 2005 | 1462 | 41.4\% | - | - |  | 0.5 | 3.1 | 2.4 |
| 1/2 | Oxford Road Ahead | U | C2:A | 2 | 173 | - | 601 | 2155 | 1571 | 38.2\% | - | - | - | 0.5 | 3.1 | 2.4 |
| 1/3 | Oxford Road Right | U | C2:F | 2 | 88 | - | 532 | 1973 | 740 | 71.9\% | - | - | - | 4.4 | 29.9 | 17.3 |
| 1/4 | Oxford Road Right | U | C2:F | 2 | 88 | - | 537 | 1973 | 740 | 72.6\% | - | - | - | 4.1 | 27.3 | 17.3 |
| 2/1 | Central Link Right | U | C2:G | 2 | 76 | - | 532 | 2029 | 659 | 80.7\% | - | - |  | 2.5 | 17.2 | 18.1 |
| 2/2 | Central Link Right | U | C2:G | 2 | 76 | - | 537 | 2024 | 658 | 81.6\% | - | - |  | 2.7 | 18.3 | 18.5 |
| 3/1 | Ped Crossing Ahead | U | C2:J | 2 | 204 | - | 685 | 1965 | 1687 | 40.6\% | - | - | - | 0.4 | 1.9 | 11.3 |
| 3/2 | Ped Crossing Ahead | U | C2:J | 2 | 204 | - | 1105 | 2105 | 1807 | 61.1\% | - | - | - | 0.8 | 2.6 | 1.0 |
| 4/1 | Services Entry Left Ahead | 0 | - | - | - | - | 98 | 2075 | 480 | 20.4\% | 71 | 27 | 0 | 0.1 | 5.3 | 0.5 |
| 6/1 | Oxford Road <br> (sb) Left | U | C2:B | 2 | 81 | - | 699 | 2015 | 697 | 100.3\% | - | - | - | 19.8 | 102.0 | 37.6 |
| 6/2 | Oxford Road <br> (sb) Ahead | U | C2:B | 2 | 81 | - | 549 | 2105 | 728 | 75.4\% | - | - |  | 5.7 | 37.3 | 17.9 |
| 6/3 | Oxford Road <br> (sb) Ahead | U | C2:B | 2 | 81 | - | 542 | 2105 | 728 | 74.5\% | - | - |  | 4.8 | 31.9 | 16.2 |
| 7/1 | Internal (eb) Ahead | U | C2:C | 2 | 45 | - | 22 | 2015 | 395 | 5.6\% | - | - | - | 0.3 | 42.8 | 0.7 |
| 7/2 | Internal (eb) Right | U | C2:C | 2 | 45 | - | 31 | 1889 | 370 | 8.4\% | - | - | - | 0.4 | 44.3 | 0.9 |
| 8/1 | Right Ahead | U | C2:E | 2 | 81 | - | 566 | 2105 | 728 | 77.7\% | - | - | - | 2.8 | 18.0 | 4.1 |
| 8/2 | Right Ahead | U | C2:E | 2 | 81 | - | 556 | 2105 | 728 | 76.4\% | - | - | - | 2.8 | 18.2 | 8.6 |
| 9/1 | Ahead Right | U | C2:H | 2 | 45 | - | 319 | 2012 | 394 | 81.0\% | - | - | - | 5.1 | 57.6 | 12.3 |


| 9/2 | Right | U | C2:H |  | 2 | 45 | - | 315 | 1973 | 386 | 81.5\% | - | - | - | 4.7 | 53.4 | 12.1 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 10/1 | Ahead | U | - |  | - | - | - | 723 | 2015 | 2015 | 35.9\% | - | - | - | 0.3 | 1.4 | 0.3 |
| 10/2 | Ahead | U | - |  | - | - | - | 953 | 2155 | 2155 | 44.2\% | - | - | - | 0.4 | 1.5 | 5.9 |
| 10/3 | Ahead Right | U | - |  | - | - | - | 11 | 2079 | 2079 | 0.5\% | - | - | - | 0.0 | 0.9 | 0.0 |
| 11/2+11/1 | A41 entry Ahead Left | U | C2:D |  | 2 | 131 | - | 1266 | 2105:1965 | 1060+210 | $\begin{aligned} & 99.7: \\ & 99.7 \% \end{aligned}$ | - | - | - | 25.4 | 72.2 | 55.8 |
| 11/3 | A41 entry Ahead | U | C2:D |  | 2 | 131 | - | 589 | 2105 | 1167 | 50.5\% | - | - | - | 3.2 | 19.7 | 12.6 |
| 12/1 | A41 exit | U | - |  | - | - | - | 685 | 1965 | 1965 | 34.8\% | - | - | - | 0.3 | 1.4 | 0.3 |
| Ped Link: P1 | Unnamed Ped Link | - | C2:K |  | 2 | 8 | - | 0 | - | 0 | 0.0\% | - | - | - | - | - | - |
| Ped Link: P2 | Unnamed Ped Link | - | C2:I |  | 2 | 81 | - | 0 | - | 0 | 0.0\% | - | - | - | - | - | - |
| J3: Oxford <br> Road/ Lakeview Drive | - | - | - |  | - | - | - | - | - | - | 85.8\% | 0 | 0 | 0 | 33.5 | - | - |
| 1/1 | Oxford Road (nb) Ahead | U | C3:A |  | 2 | 164 | - | 638 | 2155 | 1491 | 42.8\% | - | - | - | 0.7 | 4.1 | 5.8 |
| 1/2 | Oxford Road (nb) Ahead | U | C3:A |  | 2 | 164 | - | 713 | 2155 | 1491 | 47.8\% | - | - | - | 0.9 | 4.6 | 5.4 |
| 1/3+1/4 | Oxford Road (nb) Ahead Right | U | $\begin{aligned} & \text { C3:A } \\ & \text { C3:E } \end{aligned}$ |  | 2 | 164:46 | - | 1019 | 2105:2155 | 1235+366 | $\begin{gathered} 63.7: \\ 63.7 \% \end{gathered}$ | - | - | - | 5.0 | 17.6 | 36.7 |
| 2/1 | Left | U | C3:C |  | 2 | 167 | - | 386 | 1923 | 1354 | 28.5\% | - | - | - | 0.9 | 8.2 | 5.4 |
| 2/2 | Ahead | U | C3:B |  | 2 | 135 | - | 949 | 2105 | 1202 | 79.0\% | - | - | - | 7.4 | 28.2 | 28.3 |
| 2/3 | Ahead | U | C3:B |  | 2 | 135 | - | 1008 | 2105 | 1202 | 83.9\% | - | - | - | 6.4 | 22.9 | 21.9 |
| 3/2+3/1 | Lakeview Drive Right Left | U | C3:D | C3:F | 2 | 22:74 | 52 | 521 | 2080:1940 | 0+607 | $\begin{gathered} 0.0: \\ 85.8 \% \end{gathered}$ | - | - | - | 8.9 | 61.3 | 21.1 |
| 3/3 | Lakeview Drive Right | U | C3:D |  | 2 | 22 | - | 138 | 2005 | 200 | 68.8\% | - | - | - | 3.1 | 80.2 | 5.7 |
| 4/1 | Lakeview Drive | U | - |  | - | - | - | 394 | 1965 | 1965 | 20.1\% | - | - | - | 0.1 | 1.1 | 0.1 |
| 4/2 | Lakeview Drive | U | - |  | - | - | - | 225 | 1965 | 1965 | 11.5\% | - | - | - | 0.1 | 1.1 | 4.6 |



Basic Results Summary
Scenario 3: '2026 AM BTM + Committed' (FG10: '2026 AM BTM + Committed', Plan 1: 'Control Plan')
Network Layout Diagram


Basic Results Summary
Network Results

| Item | Lane Description | Lane <br> Type | Full Phase | Arrow Phase | Num Greens | Total Green (s) | Arrow Green (s) | Demand <br> Flow <br> (pcu) | Sat Flow (pcu/Hr) | Capacity (pcu) | Deg Sat <br> (\%) | Turners In Gaps (pcu) | Turners When Unopposed (pcu) | Turners In Intergreen (pcu) | Total Delay (pcuHr) | Av. <br> Delay <br> Per PCU <br> (s/pcu) | Mean Max Queue (pcu) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network | - | - | - |  | - | - | - | - | - | - | 91.2\% | 121 | 6 | 0 | 100.0 | - | - |
| J1: Pingle Drive / Oxford | - | - | - |  | - | - | - | - | - | - | 43.0\% | 0 | 0 | 0 | 8.9 | - | - |
| 1/1 | Oxford Road (nb) Ahead | U | C1:A |  | 2 | 196 | - | 411 | 1915 | 1580 | 26.0\% | - | - | - | 0.3 | 3.0 | 2.0 |
| 1/2 | Oxford Road (nb) Ahead | U | C1:A |  | 2 | 196 | - | 549 | 2055 | 1695 | 32.4\% | - | - | - | 0.4 | 2.6 | 2.0 |
| 1/3 | Oxford Road (nb) Right | U | C1:F |  | 2 | 49 | - | 61 | 2042 | 434 | 14.1\% | - | - | - | 0.7 | 41.2 | 1.9 |
| 1/4 | Oxford Road (nb) Right | U | C1:F |  | 2 | 49 | - | 71 | 2042 | 434 | 16.4\% | - | - | - | 0.8 | 40.1 | 2.1 |
| 2/2+2/1 | Pingle Drive Left | U | C1:E |  | 2 | 55 | - | 81 | 2005:1870 | $363+202$ | $\begin{gathered} 14.3: \\ 14.3 \% \end{gathered}$ | - | - | - | 0.9 | 39.4 | 1.4 |
| 2/3 | Pingle Drive Right | U | C1:D |  | 2 | 16 | - | 50 | 2067 | 155 | 32.3\% | - | - | - | 1.0 | 69.7 | 1.8 |
| 3/1 | Oxford Road (sb) Left | U | C1:C |  | 2 | 175 | - | 175 | 1908 | 1407 | 12.4\% | - | - | - | 0.3 | 6.0 | 1.9 |
| 3/2 | Oxford Road (sb) Ahead | U | C1:B |  | 2 | 161 | - | 614 | 2105 | 1430 | 42.9\% | - | - | - | 1.9 | 11.0 | 10.1 |
| 3/3 | Oxford Road (sb) Ahead | U | C1:B |  | 2 | 161 | - | 615 | 2105 | 1430 | 43.0\% | - | - | - | 1.9 | 11.0 | 10.1 |
| 4/1 | Oxford Road (nb) | U | - |  | - | - | - | 411 | 1940 | 1940 | 21.2\% | - | - | - | 0.1 | 1.2 | 0.1 |
| 4/2 | Oxford Road (nb) | U | - |  | - | - | - | 599 | 2080 | 2080 | 28.8\% | - | - | - | 0.2 | 1.2 | 0.2 |
| 5/1 | Pingle Drive | U | - |  | - | - | - | 236 | 1965 | 1965 | 12.0\% | - | - | - | 0.1 | 1.0 | 0.1 |
| 5/2 | Pingle Drive | U | - |  | - | - | - | 71 | 2105 | 2105 | 3.4\% | - | - | - | 0.0 | 0.9 | 0.0 |
| 6/1 | Right Turn Lane Right | U | C1:G |  | 2 | 43 | - | 61 | 1980 | 371 | 16.4\% | - | - | - | 0.1 | 8.3 | 0.2 |
| 6/2 | Right Turn Lane Right | U | C1:G |  | 2 | 43 | - | 71 | 1980 | 371 | 19.1\% | - | - | - | 0.2 | 9.1 | 0.2 |
| Ped Link: P1 | Unnamed Ped Link | - | C1:H |  | 2 | 31 | - | 0 | - | 0 | 0.0\% | - | - | - | - | - | - |


| Ped Link: P2 | Unnamed Ped Link | - | C1:I | 2 | 161 | - | 0 | - | 48300 | 0.0\% | - | - | - | 0.0 | 0.0 | 0.0 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Ped Link: P3 | Unnamed Ped Link | - | C1:J | 2 | 149 | - | 0 | - | 0 | 0.0\% | - | - | - | - | - | - |
| J2: Esso Roundabout | - | - | - | - | - | - | - | - | - | 91.2\% | 121 | 6 | 0 | 55.9 | - | - |
| 1/1 | Oxford Road <br> Left Ahead | U | C2:A | 2 | 178 | - | 356 | 2006 | 1504 | 23.7\% | - | - | - | 0.3 | 2.7 | 1.4 |
| 1/2 | Oxford Road Ahead | U | C2:A | 2 | 178 | - | 315 | 2155 | 1616 | 19.5\% | - | - | - | 0.2 | 2.8 | 1.6 |
| 1/3 | Oxford Road Right | U | C2:F | 2 | 106 | - | 553 | 1973 | 888 | 62.3\% | - | - | - | 3.5 | 22.5 | 17.3 |
| 1/4 | Oxford Road Right | U | C2:F | 2 | 106 | - | 545 | 1973 | 888 | 61.4\% | - | - | - | 3.3 | 21.8 | 16.9 |
| 2/1 | Central Link Right | U | C2:G | 2 | 94 | - | 553 | 2029 | 812 | 68.1\% | - | - | - | 1.6 | 10.3 | 17.6 |
| 2/2 | Central Link Right | U | C2:G | 2 | 94 | - | 545 | 2024 | 810 | 67.3\% | - | - | - | 1.5 | 10.2 | 17.5 |
| 3/1 | Ped Crossing Ahead | U | C2:J | 2 | 204 | - | 647 | 1965 | 1687 | 38.4\% | - | - | - | 0.3 | 1.9 | 11.8 |
| 3/2 | Ped Crossing Ahead | U | C2:J | 2 | 204 | - | 1029 | 2105 | 1807 | 57.0\% | - | - | - | 0.7 | 2.4 | 0.9 |
| 4/1 | Services <br> Entry Left <br> Ahead | 0 | - | - | - | - | 127 | 2058 | 520 | 24.4\% | 121 | 6 | 0 | 0.2 | 4.7 | 0.6 |
| 6/1 | Oxford Road <br> (sb) Left | U | C2:B | 2 | 68 | - | 533 | 2015 | 588 | 90.7\% | - | - | - | 9.2 | 62.2 | 23.0 |
| 6/2 | Oxford Road <br> (sb) Ahead | U | C2:B | 2 | 68 | - | 410 | 2105 | 614 | 66.8\% | - | - | - | 4.5 | 39.2 | 14.2 |
| 6/3 | Oxford Road <br> (sb) Ahead | U | C2:B | 2 | 68 | - | 367 | 2105 | 614 | 59.8\% | - | - | - | 3.7 | 36.3 | 12.3 |
| 7/1 | Internal (eb) Ahead | U | C2:C | 2 | 40 | - | 45 | 2015 | 353 | 12.8\% | - | - | - | 0.6 | 47.0 | 1.4 |
| 7/2 | Internal (eb) Right | U | C2:C | 2 | 40 | - | 55 | 1889 | 331 | 16.6\% | - | - | - | 0.7 | 48.5 | 1.7 |
| 8/1 | Right Ahead | U | C2:E | 2 | 68 | - | 420 | 2105 | 614 | 68.4\% | - | - | - | 2.1 | 18.1 | 3.2 |
| 8/2 | Right Ahead | U | C2:E | 2 | 68 | - | 412 | 2105 | 614 | 67.1\% | - | - | - | 2.0 | 17.6 | 3.8 |
| 9/1 | Ahead Right | U | C2:H | 2 | 40 | - | 249 | 2014 | 352 | 70.6\% | - | - | - | 4.1 | 58.8 | 9.2 |


| 9/2 | Right | U | C2:H |  | 2 | 40 | - | 248 | 1973 | 345 | 71.8\% | - | - | - | 3.6 | 52.9 | 9.2 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 10/1 | Ahead | U | - |  | - | - | - | 432 | 2015 | 2015 | 21.4\% | - | - | - | 0.1 | 1.1 | 0.1 |
| 10/2 | Ahead | U | - |  | - | - | - | 614 | 2155 | 2155 | 28.5\% | - | - | - | 0.2 | 1.3 | 7.4 |
| 10/3 | Ahead Right | U | - |  | - | - | - | 19 | 2068 | 2068 | 0.9\% | - | - | - | 0.0 | 0.9 | 0.0 |
| 11/2+11/1 | A41 entry Ahead Left | U | C2:D |  | 2 | 144 | - | 1232 | 2105:1965 | 1198+152 | $\begin{gathered} 91.2 \text { : } \\ 91.2 \% \end{gathered}$ | - | - | - | 11.4 | 33.3 | 42.1 |
| 11/3 | A41 entry Ahead | U | C2:D |  | 2 | 144 | - | 449 | 2105 | 1281 | 35.1\% | - | - | - | 1.7 | 14.0 | 8.5 |
| 12/1 | A41 exit | U | - |  | - | - | - | 647 | 1965 | 1965 | 32.9\% | - | - | - | 0.2 | 1.4 | 0.2 |
| Ped Link: P1 | Unnamed Ped Link | - | C2:K |  | 2 | 8 | - | 0 | - | 0 | 0.0\% | - | - | - | - | - | - |
| Ped Link: P2 | Unnamed Ped Link | - | C2:I |  | 2 | 68 | - | 0 | - | 0 | 0.0\% | - | - | - | - | - | - |
| J3: Oxford Road/ Lakeview Drive | - | - | - |  | - | - | - | - | - | - | 70.4\% | 0 | 0 | 0 | 20.2 | - | - |
| 1/1 | Oxford Road (nb) Ahead | U | C3:A |  | 2 | 168 | - | 456 | 2155 | 1526 | 29.9\% | - | - | - | 0.3 | 2.7 | 1.5 |
| 1/2 | Oxford Road (nb) Ahead | U | C3:A |  | 2 | 168 | - | 590 | 2155 | 1526 | 38.7\% | - | - | - | 0.6 | 3.6 | 3.0 |
| 1/3+1/4 | Oxford Road (nb) Ahead Right | U | $\begin{aligned} & \text { C3:A } \\ & \text { C3:E } \end{aligned}$ |  | 2 | 168:37 | - | 839 | 2105:2155 | $845+346$ | $\begin{gathered} 70.4: \\ 70.4 \% \end{gathered}$ | - | - | - | 5.1 | 22.0 | 30.1 |
| 2/1 | Left | U | C3:C |  | 2 | 176 | - | 237 | 1923 | 1426 | 16.6\% | - | - | - | 0.4 | 6.6 | 2.6 |
| 2/2 | Ahead | U | C3:B |  | 2 | 147 | - | 862 | 2105 | 1307 | 66.0\% | - | - | - | 4.6 | 19.3 | 21.2 |
| 2/3 | Ahead | U | C3:B |  | 2 | 147 | - | 917 | 2105 | 1307 | 70.2\% | - | - | - | 4.1 | 16.2 | 19.5 |
| 3/2+3/1 | Lakeview Drive Right Left | U | C3:D | C3:F | 2 | 19:62 | 43 | 259 | 2005:1940 | $113+471$ | $\begin{gathered} 44.4: \\ 44.4 \% \end{gathered}$ | - | - | - | 3.3 | 45.4 | 7.0 |
| 3/3 | Lakeview Drive Right | U | C3:D |  | 2 | 19 | - | 78 | 2005 | 175 | 44.5\% | - | - | - | 1.5 | 71.0 | 3.1 |
| 4/1 | Lakeview Drive | U | - |  | - | - | - | 254 | 1965 | 1965 | 12.9\% | - | - | - | 0.1 | 1.1 | 0.1 |
| 4/2 | Lakeview Drive | U | - |  | - | - | - | 227 | 1965 | 1965 | 11.6\% | - | - | - | 0.1 | 1.0 | 3.4 |



Basic Results Summary Basic Results Summary

## User and Project Details

| Project: | Bicester Office Park |
| :--- | :--- |
| Title: |  |
| Location: | Scenic Land Developments Ltd |
| Client: |  |
| Additional detail: | Oxford Road Model (inc BG Improvements) - 2018-04-30 Base (inc BV4).lsg3x |
| File name: |  |
| Author: | Motion |
| Company: <br> Address: |  |

Scenario 2: '2026 PM BTM + Committed + 45k' (FG13: '2026 PM BTM + Committed + 45k', Plan 1: 'Control Plan') Network Layout Diagram


Basic Results Summary
Network Results

| Item | Lane Description | Lane Type | Full Phase | Arrow Phase | Num Greens | Total Green (s) | Arrow Green (s) | Demand Flow (pcu) | Sat Flow (pcu/Hr) | Capacity (pcu) | Deg Sat (\%) | Turners In Gaps (pcu) | Turners <br> When Unopposed (pcu) | Turners In Intergreen (pcu) | Total Delay (pcuHr) | Av. <br> Delay <br> Per PCU <br> (s/pcu) | Mean <br> Max Queue (pcu) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network | - | - | - |  | - | - | - | - | - | - | 100.8\% | 79 | 19 | 0 | 273.7 | - | - |
| J1: Pingle Drive / Oxford | - | - | - |  | - | - | - | - | - | - | 61.8\% | 0 | 0 | 0 | 19.8 | - | - |
| 1/1 | Oxford Road (nb) Ahead | U | C1:A |  | 2 | 188 | - | 825 | 1915 | 1516 | 54.4\% | - | - | - | 0.9 | 4.0 | 3.4 |
| 1/2 | Oxford Road (nb) Ahead | U | C1:A |  | 2 | 188 | - | 907 | 2055 | 1627 | 55.8\% | - | - | - | 1.0 | 3.9 | 4.2 |
| 1/3 | Oxford Road (nb) Right | U | C1:F |  | 2 | 63 | - | 121 | 2042 | 553 | 21.9\% | - | - | - | 1.1 | 33.5 | 3.2 |
| 1/4 | Oxford Road (nb) Right | U | C1:F |  | 2 | 63 | - | 131 | 2042 | 553 | 23.7\% | - | - | - | 1.0 | 27.7 | 3.1 |
| 2/2+2/1 | Pingle Drive Left | U | C1:E |  | 2 | 77 | - | 465 | 2005:1870 | 469+283 | $\begin{gathered} 61.8: \\ 61.8 \% \end{gathered}$ | - | - | - | 4.9 | 37.6 | 10.1 |
| 2/3 | Pingle Drive Right | U | C1:D |  | 2 | 24 | - | 131 | 2067 | 224 | 58.5\% | - | - | - | 2.6 | 70.2 | 5.1 |
| 3/1 | Oxford Road (sb) Left | U | C1:C |  | 2 | 161 | - | 42 | 1908 | 1296 | 3.2\% | - | - | - | 0.1 | 7.9 | 0.5 |
| 3/2 | Oxford Road (sb) Ahead | U | C1:B |  | 2 | 139 | - | 668 | 2105 | 1237 | 54.0\% | - | - | - | 3.4 | 18.1 | 14.1 |
| 3/3 | Oxford Road (sb) Ahead | U | C1:B |  | 2 | 139 | - | 675 | 2105 | 1237 | 54.6\% | - | - | - | 3.4 | 18.2 | 14.3 |
| 4/1 | Oxford Road (nb) | U | - |  | - | - | - | 825 | 1940 | 1940 | 42.5\% | - | - | - | 0.4 | 1.6 | 0.4 |
| 4/2 | Oxford Road (nb) | U | - |  | - | - | - | 1038 | 2080 | 2080 | 49.9\% | - | - | - | 0.5 | 1.7 | 0.5 |
| 5/1 | Pingle Drive | U | - |  | - | - | - | 163 | 1965 | 1965 | 8.3\% | - | - | - | 0.0 | 1.0 | 0.0 |
| 5/2 | Pingle Drive | U | - |  | - | - | - | 131 | 2105 | 2105 | 6.2\% | - | - | - | 0.0 | 0.9 | 0.0 |
| 6/1 | Right Turn Lane Right | U | C1:G |  | 2 | 57 | - | 121 | 1980 | 487 | 24.9\% | - | - | - | 0.3 | 7.9 | 0.3 |
| 6/2 | Right Turn Lane Right | U | C1:G |  | 2 | 57 | - | 131 | 1980 | 487 | 26.9\% | - | - | - | 0.3 | 9.0 | 0.4 |
| Ped Link: P1 | Unnamed Ped Link | - | C1:H |  | 2 | 45 | - | 0 | - | 0 | 0.0\% | - | - | - | - | - | - |


| Ped Link: P2 | Unnamed Ped Link | - | C1:I | 2 | 147 | - | 0 | - | 44100 | 0.0\% | - | - | - | 0.0 | 0.0 | 0.0 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Ped Link: P3 | Unnamed Ped Link | - | C1:J | 2 | 127 | - | 0 | - | 0 | 0.0\% | - | - | - | - | - | - |
| J2: Esso Roundabout | - | - | - | - | - | - | - | - | - | 100.3\% | 79 | 19 | 0 | 103.5 | - | - |
| 1/1 | Oxford Road Left Ahead | U | C2:A | 2 | 174 | - | 746 | 2007 | 1472 | 50.7\% | - | - | - | 1.0 | 4.8 | 5.8 |
| 1/2 | Oxford Road Ahead | U | C2:A | 2 | 174 | - | 851 | 2155 | 1580 | 53.8\% | - | - | - | 1.1 | 4.6 | 6.2 |
| 1/3 | Oxford Road Right | U | C2:F | 2 | 89 | - | 604 | 1973 | 748 | 80.7\% | - | - | - | 4.8 | 28.4 | 23.2 |
| 1/4 | Oxford Road Right | U | C2:F | 2 | 89 | - | 492 | 1973 | 748 | 65.8\% | - | - | - | 6.3 | 46.0 | 17.2 |
| 2/1 | Central Link Right | U | C2:G | 2 | 77 | - | 604 | 2029 | 668 | 90.4\% | - | - | - | 4.6 | 27.3 | 22.0 |
| 2/2 | Central Link Right | U | C2:G | 2 | 77 | - | 492 | 2024 | 666 | 73.8\% | - | - | - | 1.6 | 11.7 | 16.2 |
| 3/1 | Ped Crossing Ahead | U | C2:J | 2 | 204 | - | 762 | 1965 | 1687 | 45.2\% | - | - | - | 0.5 | 2.5 | 15.8 |
| 3/2 | Ped Crossing Ahead | U | C2:J | 2 | 204 | - | 1194 | 2105 | 1807 | 66.0\% | - | - | - | 1.0 | 3.1 | 2.8 |
| 4/1 | Services Entry Left Ahead | 0 | - | - | - | - | 98 | 2075 | 358 | 27.4\% | 79 | 19 | 0 | 0.2 | 8.1 | 0.9 |
| 6/1 | Oxford Road <br> (sb) Left | U | C2:B | 2 | 81 | - | 699 | 2015 | 697 | 100.3\% | - | - | - | 20.0 | 102.8 | 37.8 |
| 6/2 | Oxford Road <br> (sb) Ahead | U | C2:B | 2 | 81 | - | 588 | 2105 | 728 | 80.8\% | - | - | - | 6.3 | 38.4 | 19.7 |
| 6/3 | Oxford Road <br> (sb) Ahead | U | C2:B | 2 | 81 | - | 521 | 2105 | 728 | 71.6\% | - | - | - | 4.9 | 33.9 | 14.8 |
| 7/1 | Internal (eb) Ahead | U | C2:C | 2 | 44 | - | 161 | 2015 | 386 | 41.7\% | - | - | - | 2.0 | 44.2 | 6.4 |
| 7/2 | Internal (eb) Right | U | C2:C | 2 | 44 | - | 31 | 1889 | 362 | 8.6\% | - | - | - | 0.4 | 44.4 | 0.9 |
| 8/1 | Right Ahead | U | C2:E | 2 | 81 | - | 604 | 2105 | 728 | 83.0\% | - | - | - | 3.8 | 22.8 | 5.2 |
| 8/2 | Right Ahead | U | C2:E | 2 | 81 | - | 536 | 2105 | 728 | 73.6\% | - | - | - | 2.5 | 16.9 | 3.6 |
| 9/1 | Ahead Right | U | C2:H | 2 | 44 | - | 319 | 2012 | 386 | 82.7\% | - | - | - | 5.4 | 60.7 | 12.7 |


| 9/2 | Right | U | C2:H |  | 2 | 44 | - | 315 | 1973 | 378 | 83.3\% | - | - | - | 5.0 | 56.7 | 12.6 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 10/1 | Ahead | U | - |  | - | - | - | 857 | 2015 | 2015 | 42.5\% | - | - | - | 0.4 | 1.6 | 0.4 |
| 10/2 | Ahead | U | - |  | - | - | - | 1060 | 2155 | 2155 | 49.2\% | - | - | - | 0.5 | 1.7 | 7.7 |
| 10/3 | Ahead Right | U | - |  | - | - | - | 161 | 2033 | 2033 | 7.9\% | - | - | - | 0.0 | 1.0 | 0.0 |
| 11/2+11/1 | A41 entry Ahead Left | U | C2:D |  | 2 | 131 | - | 1278 | 2105:1965 | 1055+221 | $\begin{gathered} 100.2: \\ 100.2 \% \end{gathered}$ | - | - | - | 27.8 | 78.4 | 59.3 |
| 11/3 | A41 entry Ahead | U | C2:D |  | 2 | 131 | - | 589 | 2105 | 1167 | 50.5\% | - | - | - | 3.2 | 19.7 | 12.6 |
| 12/1 | A41 exit | U | - |  | - | - | - | 762 | 1965 | 1965 | 38.8\% | - | - | - | 0.3 | 1.5 | 1.4 |
| Ped Link: P1 | Unnamed Ped Link | - | C2:K |  | 2 | 8 | - | 0 | - | 0 | 0.0\% | - | - | - | - | - | - |
| Ped Link: P2 | Unnamed Ped Link | - | C2:I |  | 2 | 81 | - | 0 | - | 0 | 0.0\% | - | - | - | - | - | - |
| J3: Oxford <br> Road/ Lakeview Drive | - | - | - |  | - | - | - | - | - | - | 100.8\% | 0 | 0 | 0 | 111.9 | - | - |
| 1/1 | Oxford Road (nb) Ahead | U | C3:A |  | 2 | 124 | - | 932 | 2155 | 1131 | 82.4\% | - | - | - | 5.3 | 20.5 | 33.4 |
| 1/2 | Oxford Road (nb) Ahead | U | C3:A |  | 2 | 124 | - | 943 | 2155 | 1131 | 83.3\% | - | - | - | 5.9 | 22.3 | 34.2 |
| 1/3+1/4 | Oxford Road (nb) Ahead Right | U | $\begin{aligned} & \text { C3:A } \\ & \text { C3:E } \end{aligned}$ |  | 2 | 124:28 | - | 517 | 2105:2155 | 277+269 | $\begin{gathered} 94.7: \\ 94.7 \% \end{gathered}$ | - | - | - | 10.1 | 70.0 | 24.9 |
| 2/1 | Left | U | C3:C |  | 2 | 185 | - | 416 | 1923 | 1498 | 27.7\% | - | - | - | 0.8 | 7.2 | 4.4 |
| 2/2 | Ahead | U | C3:B |  | 2 | 109 | - | 976 | 2105 | 974 | 100.1\% | - | - | - | 24.6 | 90.9 | 49.1 |
| 2/3 | Ahead | U | C3:B |  | 2 | 109 | - | 981 | 2105 | 974 | 100.7\% | - | - | - | 24.1 | 88.6 | 51.3 |
| 3/2+3/1 | Lakeview Drive Right Left | U | C3:D | C3:F | 2 | 66:100 | 34 | 824 | 2080:1940 | 0+817 | $\begin{gathered} 0.0: \\ 100.8 \% \end{gathered}$ | - | - | - | 25.2 | 109.9 | 45.6 |
| 3/3 | Lakeview Drive Right | U | C3:D |  | 2 | 66 | - | 556 | 2005 | 568 | 97.9\% | - | - | - | 15.7 | 101.9 | 27.4 |
| 4/1 | Lakeview Drive | U | - |  | - | - | - | 436 | 1965 | 1965 | 22.2\% | - | - | - | 0.1 | 1.2 | 0.1 |
| 4/2 | Lakeview Drive | U | - |  | - | - | - | 235 | 1965 | 1965 | 12.0\% | - | - | - | 0.1 | 1.0 | 3.9 |



Basic Results Summary
Scenario 4: '2026 AM BTM + Committed + 45k' (FG12: '2026 AM BTM + Committed + 45k', Plan 1: 'Control Plan') Network Layout Diagram


Basic Results Summary
Network Results

| Item | Lane Description | Lane Type | Full Phase | Arrow Phase | Num Greens | Total Green (s) | Arrow Green (s) | Demand <br> Flow <br> (pcu) | Sat Flow (pcu/Hr) | Capacity (pcu) | Deg Sat <br> (\%) | Turners In Gaps (pcu) | Turners When Unopposed (pcu) | Turners In Intergreen (pcu) | Total Delay (pcuHr) | Av. <br> Delay <br> Per PCU <br> (s/pcu) | Mean Max Queue (pcu) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network | - | - | - |  | - | - | - | - | - | - | 96.2\% | 114 | 13 | 0 | 158.1 | - | - |
| J1: Pingle Drive / Oxford | - | - | - |  | - | - | - | - | - | - | 48.8\% | 0 | 0 | 0 | 9.7 | - | - |
| 1/1 | Oxford Road (nb) Ahead | U | C1:A |  | 2 | 196 | - | 440 | 1915 | 1580 | 27.9\% | - | - | - | 0.4 | 3.6 | 2.7 |
| 1/2 | Oxford Road (nb) Ahead | U | C1:A |  | 2 | 196 | - | 542 | 2055 | 1695 | 32.0\% | - | - | - | 0.5 | 3.0 | 2.8 |
| 1/3 | Oxford Road (nb) Right | U | C1:F |  | 2 | 40 | - | 61 | 2042 | 357 | 17.1\% | - | - | - | 0.9 | 52.0 | 1.9 |
| 1/4 | Oxford Road (nb) Right | U | C1:F |  | 2 | 40 | - | 71 | 2042 | 357 | 19.9\% | - | - | - | 0.9 | 46.0 | 2.1 |
| 2/2+2/1 | Pingle Drive Left | U | C1:E |  | 2 | 46 | - | 81 | 2005:1870 | 316+176 | $\begin{aligned} & 16.5: \\ & 16.5 \% \end{aligned}$ | - | - | - | 1.0 | 43.8 | 1.6 |
| 2/3 | Pingle Drive Right | U | C1:D |  | 2 | 16 | - | 50 | 2067 | 155 | 32.3\% | - | - | - | 1.0 | 69.8 | 1.9 |
| 3/1 | Oxford Road (sb) Left | U | C1:C |  | 2 | 184 | - | 175 | 1908 | 1479 | 11.8\% | - | - | - | 0.2 | 4.7 | 1.5 |
| 3/2 | Oxford Road (sb) Ahead | U | C1:B |  | 2 | 170 | - | 736 | 2105 | 1509 | 48.8\% | - | - | - | 2.0 | 9.7 | 11.3 |
| 3/3 | Oxford Road (sb) Ahead | U | C1:B |  | 2 | 170 | - | 735 | 2105 | 1509 | 48.7\% | - | - | - | 2.0 | 9.7 | 11.3 |
| 4/1 | Oxford Road (nb) | U | - |  | - | - | - | 440 | 1940 | 1940 | 22.7\% | - | - | - | 0.1 | 1.2 | 0.1 |
| 4/2 | Oxford Road (nb) | U | - |  | - | - | - | 592 | 2080 | 2080 | 28.5\% | - | - | - | 0.2 | 1.2 | 0.2 |
| 5/1 | Pingle Drive | U | - |  | - | - | - | 236 | 1965 | 1965 | 12.0\% | - | - | - | 0.1 | 1.0 | 0.1 |
| 5/2 | Pingle Drive | U | - |  | - | - | - | 71 | 2105 | 2105 | 3.4\% | - | - | - | 0.0 | 0.9 | 0.0 |
| 6/1 | Right Turn Lane Right | U | C1:G |  | 2 | 34 | - | 61 | 1980 | 297 | 20.5\% | - | - | - | 0.2 | 11.7 | 0.2 |
| 6/2 | Right Turn Lane Right | U | C1:G |  | 2 | 34 | - | 71 | 1980 | 297 | 23.9\% | - | - | - | 0.3 | 13.8 | 0.3 |
| Ped Link: P1 | Unnamed Ped Link | - | C1:H |  | 2 | 22 | - | 0 | - | 0 | 0.0\% | - | - | - | - | - | - |


| Ped Link: P2 | Unnamed Ped Link | - | C1:I | 2 | 170 | - | 0 | - | 51000 | 0.0\% | - | - | - | 0.0 | 0.0 | 0.0 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Ped Link: P3 | Unnamed Ped Link | - | C1:J | 2 | 158 | - | 0 | - | 0 | 0.0\% | - | - | - | - | - | - |
| J2: Esso Roundabout | - | - | - | - | - | - | - | - | - | 96.2\% | 114 | 13 | 0 | 92.9 | - | - |
| 1/1 | Oxford Road Left Ahead | U | C2:A | 2 | 169 | - | 441 | 2008 | 1431 | 30.8\% | - | - | - | 0.6 | 4.8 | 3.4 |
| 1/2 | Oxford Road Ahead | U | C2:A | 2 | 169 | - | 589 | 2155 | 1535 | 38.4\% | - | - | - | 0.9 | 5.2 | 5.0 |
| 1/3 | Oxford Road Right | U | C2:F | 2 | 101 | - | 686 | 1973 | 847 | 81.0\% | - | - | - | 4.5 | 23.4 | 22.7 |
| 1/4 | Oxford Road Right | U | C2:F | 2 | 101 | - | 89 | 1973 | 847 | 10.5\% | - | - | - | 0.6 | 22.3 | 3.2 |
| 2/1 | Central Link Right | U | C2:G | 2 | 89 | - | 686 | 2029 | 769 | 89.2\% | - | - | - | 4.7 | 24.5 | 25.9 |
| 2/2 | Central Link Right | U | C2:G | 2 | 89 | - | 89 | 2024 | 767 | 11.6\% | - | - | - | 0.1 | 3.4 | 2.5 |
| 3/1 | Ped Crossing Ahead | U | C2:J | 2 | 204 | - | 936 | 1965 | 1687 | 55.5\% | - | - | - | 0.7 | 2.5 | 17.2 |
| 3/2 | Ped Crossing Ahead | U | C2:J | 2 | 204 | - | 754 | 2105 | 1807 | 41.7\% | - | - | - | 1.1 | 5.0 | 8.0 |
| 4/1 | Services <br> Entry Left Ahead | 0 | - | - | - | - | 127 | 2058 | 467 | 27.2\% | 114 | 13 | 0 | 0.2 | 5.5 | 0.6 |
| 6/1 | Oxford Road <br> (sb) Left | U | C2:B | 2 | 64 | - | 533 | 2015 | 554 | 96.2\% | - | - | - | 12.8 | 86.3 | 26.2 |
| 6/2 | Oxford Road <br> (sb) Ahead | U | C2:B | 2 | 64 | - | 539 | 2105 | 579 | 93.1\% | - | - | - | 10.7 | 71.3 | 24.2 |
| 6/3 | Oxford Road <br> (sb) Ahead | U | C2:B | 2 | 64 | - | 480 | 2105 | 579 | 82.9\% | - | - | - | 6.9 | 51.6 | 18.4 |
| 7/1 | Internal (eb) Ahead | U | C2:C | 2 | 49 | - | 382 | 2015 | 428 | 89.2\% | - | - | - | 7.9 | 74.5 | 16.6 |
| 7/2 | Internal (eb) Right | U | C2:C | 2 | 49 | - | 55 | 1889 | 401 | 13.7\% | - | - | - | 0.7 | 43.9 | 1.6 |
| 8/1 | Right Ahead | U | C2:E | 2 | 64 | - | 537 | 2105 | 579 | 92.8\% | - | - | - | 8.7 | 58.2 | 11.7 |
| 8/2 | Right Ahead | U | C2:E | 2 | 64 | - | 537 | 2105 | 579 | 92.8\% | - | - | - | 7.7 | 51.6 | 12.9 |
| 9/1 | Ahead Right | U | C2:H | 2 | 49 | - | 248 | 2014 | 428 | 57.9\% | - | - | - | 3.4 | 49.0 | 8.6 |


| 9/2 | Right | U | C2:H |  | 2 | 49 | - | 249 | 1973 | 419 | 59.4\% | - | - | - | 2.9 | 42.4 | 8.5 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 10/1 | Ahead | U | - |  | - | - | - | 517 | 2015 | 2015 | 25.7\% | - | - | - | 0.2 | 1.2 | 0.2 |
| 10/2 | Ahead | U | - |  | - | - | - | 552 | 2155 | 2155 | 25.6\% | - | - | - | 0.2 | 1.3 | 7.0 |
| 10/3 | Ahead Right | U | - |  | - | - | - | 355 | 2022 | 2022 | 17.6\% | - | - | - | 0.1 | 1.1 | 0.1 |
| 11/2+11/1 | A41 entry Ahead Left | U | C2:D |  | 2 | 148 | - | 1391 | 2105:1965 | $1141+311$ | $\begin{aligned} & 95.8: \\ & 95.8 \% \end{aligned}$ | - | - | - | 15.7 | 40.6 | 47.6 |
| 11/3 | A41 entry Ahead | U | C2:D |  | 2 | 148 | - | 449 | 2105 | 1316 | 34.1\% | - | - | - | 1.6 | 12.8 | 7.5 |
| 12/1 | A41 exit | U | - |  | - | - | - | 936 | 1965 | 1965 | 47.6\% | - | - | - | 0.5 | 1.7 | 2.1 |
| Ped Link: P1 | Unnamed Ped Link | - | C2:K |  | 2 | 8 | - | 0 | - | 0 | 0.0\% | - | - | - | - | - | - |
| Ped Link: P2 | Unnamed Ped Link | - | C2:I |  | 2 | 64 | - | 0 | - | 0 | 0.0\% | - | - | - | - | - | - |
| J3: Oxford Road/ Lakeview Drive | - | - | - |  | - | - | - | - | - | - | 88.8\% | 0 | 0 | 0 | 38.3 | - | - |
| 1/1 | Oxford Road (nb) Ahead | U | C3:A |  | 2 | 156 | - | 774 | 2155 | 1419 | 54.6\% | - | - | - | 2.0 | 9.1 | 9.1 |
| 1/2 | Oxford Road (nb) Ahead | U | C3:A |  | 2 | 156 | - | 867 | 2155 | 1419 | 61.1\% | - | - | - | 2.5 | 10.5 | 25.4 |
| 1/3+1/4 | Oxford Road (nb) Ahead Right | U | $\begin{aligned} & \text { C3:A } \\ & \text { C3:E } \end{aligned}$ |  | 2 | 156:65 | - | 534 | 2105:2155 | 0+602 | $\begin{gathered} 0.0: \\ 88.8 \% \end{gathered}$ | - | - | - | 10.1 | 68.4 | 24.9 |
| 2/1 | Left | U | C3:C |  | 2 | 148 | - | 638 | 1923 | 1202 | 53.1\% | - | - | - | 1.3 | 7.2 | 5.7 |
| 2/2 | Ahead | U | C3:B |  | 2 | 123 | - | 869 | 2105 | 1096 | 79.3\% | - | - | - | 9.4 | 38.8 | 24.5 |
| 2/3 | Ahead | U | C3:B |  | 2 | 123 | - | 910 | 2105 | 1096 | 83.0\% | - | - | - | 6.5 | 25.6 | 30.8 |
| 3/2+3/1 | Lakeview Drive Right Left | U | C3:D | C3:F | 2 | 15:86 | 71 | 292 | 2005:1940 | 142+629 | $\begin{gathered} 39.4: \\ 37.5 \% \end{gathered}$ | - | - | - | 2.9 | 36.3 | 6.4 |
| 3/3 | Lakeview Drive Right | U | C3:D |  | 2 | 15 | - | 108 | 2005 | 142 | 76.0\% | - | - | - | 3.1 | 103.7 | 5.2 |
| 4/1 | Lakeview Drive | U | - |  | - | - | - | 673 | 1965 | 1965 | 34.2\% | - | - | - | 0.3 | 1.4 | 0.3 |
| 4/2 | Lakeview Drive | U | - |  | - | - | - | 499 | 1965 | 1965 | 25.4\% | - | - | - | 0.2 | 1.4 | 13.4 |



Basic Results Summary Basic Results Summary

## User and Project Details

| Project: | Bicester Office Park |
| :--- | :--- |
| Title: |  |
| Location: | Scenic Land Developments Ltd |
| Client: |  |
| Additional detail: | Oxford Road Model (inc BG Improvements) - 2018-04-30 Base (inc BV4).lsg3x |
| File name: |  |
| Author: | Motion |
| Company: <br> Address: |  |

Basic Results Summary
Scenario 5: '2026 PM BTM + Committed + Proposed' (FG19: '2026 PM BTM + Committed + Proposed', Plan 1: 'Control Plan')
Network Layout Diagram


Basic Results Summary
Network Results

| Item | Lane Description | Lane Type | Full Phase | Arrow Phase | Num Greens | Total Green (s) | Arrow Green (s) | Demand Flow (pcu) | Sat Flow (pcu/Hr) | Capacity (pcu) | Deg Sat (\%) | Turners In Gaps (pcu) | Turners <br> When Unopposed (pcu) | Turners In Intergreen (pcu) | Total Delay (pcuHr) | Av. <br> Delay <br> Per PCU <br> (s/pcu) | Mean <br> Max Queue (pcu) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network | - | - | - |  | - | - | - | - | - | - | 107.6\% | 62 | 36 | 0 | 413.4 | - | - |
| J1: Pingle Drive / Oxford | - | - | - |  | - | - | - | - | - | - | 66.9\% | 0 | 0 | 0 | 20.5 | - | - |
| 1/1 | Oxford Road (nb) Ahead | U | C1:A |  | 2 | 191 | - | 871 | 1915 | 1540 | 56.1\% | - | - | - | 1.0 | 4.2 | 7.6 |
| 1/2 | Oxford Road (nb) Ahead | U | C1:A |  | 2 | 191 | - | 945 | 2055 | 1653 | 56.7\% | - | - | - | 1.1 | 4.3 | 10.5 |
| 1/3 | Oxford Road (nb) Right | U | C1:F |  | 2 | 59 | - | 121 | 2042 | 519 | 23.3\% | - | - | - | 1.2 | 37.2 | 3.2 |
| 1/4 | Oxford Road (nb) Right | U | C1:F |  | 2 | 59 | - | 131 | 2042 | 519 | 25.2\% | - | - | - | 1.1 | 31.0 | 3.3 |
| 2/2+2/1 | Pingle Drive Left | U | C1:E |  | 2 | 70 | - | 465 | 2005:1870 | 434+262 | $\begin{gathered} 66.9: \\ 66.9 \% \end{gathered}$ | - | - | - | 5.4 | 42.2 | 11.2 |
| 2/3 | Pingle Drive Right | U | C1:D |  | 2 | 21 | - | 131 | 2067 | 198 | 66.1\% | - | - | - | 2.9 | 78.5 | 5.3 |
| 3/1 | Oxford Road (sb) Left | U | C1:C |  | 2 | 165 | - | 42 | 1908 | 1328 | 3.2\% | - | - | - | 0.1 | 7.1 | 0.5 |
| 3/2 | Oxford Road (sb) Ahead | U | C1:B |  | 2 | 146 | - | 675 | 2105 | 1298 | 52.0\% | - | - | - | 3.0 | 16.0 | 14.2 |
| 3/3 | Oxford Road (sb) Ahead | U | C1:B |  | 2 | 146 | - | 674 | 2105 | 1298 | 51.9\% | - | - | - | 3.0 | 15.9 | 14.2 |
| 4/1 | Oxford Road (nb) | U | - |  | - | - | - | 871 | 1940 | 1940 | 44.5\% | - | - | - | 0.4 | 1.7 | 0.4 |
| 4/2 | Oxford Road (nb) | U | - |  | - | - | - | 1076 | 2080 | 2080 | 51.4\% | - | - | - | 0.5 | 1.8 | 0.5 |
| 5/1 | Pingle Drive | U | - |  | - | - | - | 163 | 1965 | 1965 | 8.3\% | - | - | - | 0.0 | 1.0 | 0.0 |
| 5/2 | Pingle Drive | U | - |  | - | - | - | 131 | 2105 | 2105 | 6.2\% | - | - | - | 0.0 | 0.9 | 0.0 |
| 6/1 | Right Turn Lane Right | U | C1:G |  | 2 | 53 | - | 121 | 1980 | 454 | 26.6\% | - | - | - | 0.3 | 9.3 | 0.4 |
| 6/2 | Right Turn Lane Right | U | C1:G |  | 2 | 53 | - | 131 | 1980 | 454 | 28.8\% | - | - | - | 0.4 | 10.3 | 0.5 |
| Ped Link: P1 | Unnamed Ped Link | - | C1:H |  | 2 | 41 | - | 0 | - | 0 | 0.0\% | - | - | - | - | - | - |


| Basic Results Summary |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Ped Link: P2 | Unnamed Ped Link | - | C1:I | 2 | 151 | - | 0 | - | 45300 | 0.0\% | - | - | - | 0.0 | 0.0 | 0.0 |
| Ped Link: P3 | Unnamed Ped Link | - | C1:J | 2 | 134 | - | 0 | - | 0 | 0.0\% | - | - | - | - | - | - |
| J2: Esso Roundabout | - | - | - | - | - | - | - | - | - | 102.5\% | 62 | 36 | 0 | 120.9 | - | - |
| 1/1 | Oxford Road Left Ahead | U | C2:A | 2 | 177 | - | 834 | 2008 | 1498 | 55.2\% | - | - | - | 1.3 | 5.6 | 7.6 |
| 1/2 | Oxford Road Ahead | U | C2:A | 2 | 177 | - | 997 | 2155 | 1607 | 61.5\% | - | - | - | 1.5 | 5.5 | 7.6 |
| 1/3 | Oxford Road Right | U | C2:F | 2 | 89 | - | 642 | 1973 | 748 | 85.8\% | - | - | - | 5.4 | 30.4 | 23.8 |
| 1/4 | Oxford Road Right | U | C2:F | 2 | 89 | - | 359 | 1973 | 748 | 46.7\% | - | - | - | 5.1 | 52.7 | 11.9 |
| 2/1 | Central Link Right | U | C2:G | 2 | 77 | - | 642 | 2029 | 668 | 96.1\% | - | - | - | 8.5 | 47.9 | 27.8 |
| 2/2 | Central Link Right | U | C2:G | 2 | 77 | - | 359 | 2024 | 666 | 52.4\% | - | - | - | 0.6 | 6.5 | 11.3 |
| 3/1 | Ped Crossing Ahead | U | C2:J | 2 | 204 | - | 839 | 1965 | 1687 | 49.7\% | - | - | - | 1.1 | 4.9 | 16.4 |
| 3/2 | Ped Crossing Ahead | U | C2:J | 2 | 204 | - | 1172 | 2105 | 1807 | 64.3\% | - | - | - | 1.2 | 3.7 | 5.0 |
| 4/1 | Services Entry Left Ahead | 0 | - | - | - | - | 98 | 2075 | 370 | 26.5\% | 62 | 36 | 0 | 0.2 | 7.6 | 0.9 |
| 6/1 | Oxford Road <br> (sb) Left | U | C2:B | 2 | 84 | - | 699 | 2015 | 722 | 96.8\% | - | - | - | 14.3 | 73.6 | 32.4 |
| 6/2 | Oxford Road <br> (sb) Ahead | U | C2:B | 2 | 84 | - | 603 | 2105 | 754 | 79.9\% | - | - | - | 6.2 | 37.1 | 20.8 |
| 6/3 | Oxford Road <br> (sb) Ahead | U | C2:B | 2 | 84 | - | 512 | 2105 | 754 | 67.9\% | - | - | - | 4.5 | 31.7 | 15.1 |
| 7/1 | Internal (eb) Ahead | U | C2:C | 2 | 41 | - | 311 | 2015 | 361 | 86.1\% | - | - | - | 6.3 | 72.6 | 14.4 |
| 7/2 | Internal (eb) Right | U | C2:C | 2 | 41 | - | 31 | 1889 | 338 | 9.2\% | - | - | - | 0.4 | 46.0 | 0.9 |
| 8/1 | Right Ahead | U | C2:E | 2 | 84 | - | 620 | 2105 | 754 | 82.2\% | - | - | - | 3.7 | 21.7 | 5.5 |
| 8/2 | Right Ahead | U | C2:E | 2 | 84 | - | 526 | 2105 | 754 | 69.7\% | - | - | - | 2.3 | 15.5 | 3.7 |
| 9/1 | Ahead Right | U | C2:H | 2 | 41 | - | 319 | 2012 | 360 | 88.5\% | - | - | - | 6.5 | 73.3 | 13.8 |




Basic Results Summary
Scenario 6: '2026 AM BTM + Committed + Proposed' (FG18: '2026 AM BTM + Committed + Proposed', Plan 1:
'Control Plan')
Network Layout Diagram


Basic Results Summary
Network Results

| Item | Lane Description | Lane Type | Full Phase | Arrow Phase | Num Greens | Total Green (s) | Arrow Green (s) | Demand <br> Flow <br> (pcu) | Sat Flow (pcu/Hr) | Capacity (pcu) | $\begin{aligned} & \text { Deg } \\ & \text { Sat } \\ & \text { (\%) } \end{aligned}$ | Turners In Gaps (pcu) | Turners When Unopposed (pcu) | Turners In Intergreen (pcu) | Total Delay (pcuHr) | Av. <br> Delay <br> Per PCU <br> (s/pcu) | Mean Max Queue (pcu) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network | - | - | - |  | - | - | - | - | - | - | 98.9\% | 117 | 10 | 0 | 191.3 | - | - |
| J1: Pingle Drive / Oxford | - | - | - |  | - | - | - | - | - | - | 50.5\% | 0 | 0 | 0 | 9.7 | - | - |
| 1/1 | Oxford Road (nb) Ahead | U | C1:A |  | 2 | 196 | - | 459 | 1915 | 1580 | 29.1\% | - | - | - | 0.3 | 2.6 | 1.6 |
| 1/2 | Oxford Road (nb) Ahead | U | C1:A |  | 2 | 196 | - | 530 | 2055 | 1695 | 31.3\% | - | - | - | 0.4 | 2.6 | 2.0 |
| 1/3 | Oxford Road (nb) Right | U | C1:F |  | 2 | 36 | - | 64 | 2042 | 323 | 19.8\% | - | - | - | 1.0 | 56.4 | 2.1 |
| 1/4 | Oxford Road (nb) Right | U | C1:F |  | 2 | 36 | - | 68 | 2042 | 323 | 21.0\% | - | - | - | 0.9 | 47.3 | 2.2 |
| 2/2+2/1 | Pingle Drive Left | U | C1:E |  | 2 | 42 | - | 81 | 2005:1870 | 295+165 | $\begin{gathered} 17.6: \\ 17.6 \% \end{gathered}$ | - | - | - | 1.0 | 45.8 | 1.6 |
| 2/3 | Pingle Drive Right | U | C1:D |  | 2 | 16 | - | 50 | 2067 | 155 | 32.3\% | - | - | - | 1.0 | 69.7 | 1.8 |
| 3/1 | Oxford Road <br> (sb) Left | U | C1:C |  | 2 | 188 | - | 175 | 1908 | 1510 | 11.6\% | - | - | - | 0.2 | 4.2 | 1.4 |
| 3/2 | Oxford Road (sb) Ahead | U | C1:B |  | 2 | 174 | - | 773 | 2105 | 1544 | 50.1\% | - | - | - | 2.0 | 9.1 | 11.7 |
| 3/3 | Oxford Road (sb) Ahead | U | C1:B |  | 2 | 174 | - | 779 | 2105 | 1544 | 50.5\% | - | - | - | 2.0 | 9.1 | 11.8 |
| 4/1 | Oxford Road (nb) | U | - |  | - | - | - | 459 | 1940 | 1940 | 23.7\% | - | - | - | 0.2 | 1.2 | 0.2 |
| 4/2 | Oxford Road (nb) | U | - |  | - | - | - | 580 | 2080 | 2080 | 27.9\% | - | - | - | 0.2 | 1.2 | 0.2 |
| 5/1 | Pingle Drive | U | - |  | - | - | - | 239 | 1965 | 1965 | 12.2\% | - | - | - | 0.1 | 1.0 | 0.1 |
| 5/2 | Pingle Drive | U | - |  | - | - | - | 68 | 2105 | 2105 | 3.2\% | - | - | - | 0.0 | 0.9 | 0.0 |
| 6/1 | Right Turn Lane Right | U | C1:G |  | 2 | 30 | - | 64 | 1980 | 264 | 24.2\% | - | - | - | 0.2 | 12.6 | 0.2 |
| 6/2 | Right Turn Lane Right | U | C1:G |  | 2 | 30 | - | 68 | 1980 | 264 | 25.8\% | - | - | - | 0.3 | 13.7 | 0.3 |
| Ped Link: P1 | Unnamed Ped Link | - | C1:H |  | 2 | 18 | - | 0 | - | 0 | 0.0\% | - | - | - | - | - | - |





Basic Results Summary
Basic Results Summary

## User and Project Details

| Project: | Bicester Office Park |
| :--- | :--- |
| Title: |  |
| Location: | Scenic Land Developments Ltd |
| Client: |  |
| Additional detail: | Oxford Road Model (inc BG Improvements) - 2018-04-30 with Mitigation (inc <br> BV4).lsg3x |
| File name: |  |
| Author: | Motion |
| Company: |  |
| Address: |  |

Scenario 1: '2026 PM BTM + Committed + 60k' (FG17: '2026 PM BTM + Committed + 60k', Plan 1: 'Control Plan') Network Layout Diagram


Basic Results Summary
Network Results

| Item | Lane Description | Lane Type | Full Phase | Arrow Phase | Num Greens | Total Green (s) | Arrow Green (s) | Demand Flow (pcu) | Sat Flow (pcu/Hr) | Capacity (pcu) | Deg Sat <br> (\%) | Turners In Gaps (pcu) | Turners <br> When Unopposed (pcu) | Turners In Intergreen (pcu) | Total Delay (pcuHr) | Av. <br> Delay <br> Per PCU <br> (s/pcu) | Mean <br> Max Queue (pcu) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network | - | - | - |  | - | - | - | - | - | - | 97.7\% | 79 | 19 | 0 | 225.9 | - | - |
| J1: Pingle Drive / Oxford | - | - | - |  | - | - | - | - | - | - | 67.5\% | 0 | 0 | 0 | 22.4 | - | - |
| 1/1 | Oxford Road (nb) Ahead | U | C1:A |  | 2 | 190 | - | 869 | 1915 | 1532 | 56.7\% | - | - | - | 1.1 | 4.4 | 4.1 |
| 1/2 | Oxford Road (nb) Ahead | U | C1:A |  | 2 | 190 | - | 947 | 2055 | 1644 | 57.6\% | - | - | - | 1.1 | 4.3 | 6.5 |
| 1/3 | Oxford Road (nb) Right | U | C1:F |  | 2 | 71 | - | 139 | 2042 | 621 | 22.4\% | - | - | - | 1.0 | 25.8 | 2.8 |
| 1/4 | Oxford Road (nb) Right | U | C1:F |  | 2 | 71 | - | 145 | 2042 | 621 | 23.3\% | - | - | - | 1.0 | 25.4 | 2.8 |
| 2/2+2/1 | Pingle Drive Left | U | C1:E |  | 2 | 83 | - | 535 | 2005:1870 | 511+281 | $\begin{aligned} & 67.5: \\ & 67.5 \% \end{aligned}$ | - | - | - | 5.6 | 37.4 | 13.0 |
| 2/3 | Pingle Drive Right | U | C1:D |  | 2 | 22 | - | 139 | 2067 | 207 | 67.2\% | - | - | - | 3.0 | 78.1 | 5.7 |
| 3/1 | Oxford Road (sb) Left | U | C1:C |  | 2 | 153 | - | 73 | 1908 | 1232 | 5.9\% | - | - | - | 0.2 | 9.4 | 0.9 |
| 3/2 | Oxford Road (sb) Ahead | U | C1:B |  | 2 | 133 | - | 675 | 2105 | 1184 | 57.0\% | - | - | - | 3.8 | 20.4 | 15.5 |
| 3/3 | Oxford Road (sb) Ahead | U | C1:B |  | 2 | 133 | - | 674 | 2105 | 1184 | 56.9\% | - | - | - | 3.8 | 20.4 | 15.4 |
| 4/1 | Oxford Road (nb) | U | - |  | - | - | - | 869 | 1940 | 1940 | 44.8\% | - | - | - | 0.4 | 1.7 | 0.4 |
| 4/2 | Oxford Road (nb) | U | - |  | - | - | - | 1086 | 2080 | 2080 | 52.2\% | - | - | - | 0.5 | 1.8 | 0.5 |
| 5/1 | Pingle Drive | U | - |  | - | - | - | 212 | 1965 | 1965 | 10.8\% | - | - | - | 0.1 | 1.0 | 0.1 |
| 5/2 | Pingle Drive | U | - |  | - | - | - | 145 | 2105 | 2105 | 6.9\% | - | - | - | 0.0 | 0.9 | 0.0 |
| 6/1 | Right Turn Lane Right | U | C1:G |  | 2 | 65 | - | 139 | 1980 | 553 | 25.1\% | - | - | - | 0.3 | 8.6 | 0.4 |
| 6/2 | Right Turn Lane Right | U | C1:G |  | 2 | 65 | - | 145 | 1980 | 553 | 26.2\% | - | - | - | 0.4 | 9.2 | 0.5 |
| Ped Link: P1 | Unnamed Ped Link | - | C1:H |  | 2 | 53 | - | 0 | - | 0 | 0.0\% | - | - | - | - | - | - |


| Ped Link: P2 | Unnamed Ped Link | - | C1:I | 2 | 139 | - | 0 | - | 41700 | 0.0\% | - | - | - | 0.0 | 0.0 | 0.0 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Ped Link: P3 | Unnamed Ped Link | - | C1:J | 2 | 121 | - | 0 | - | 0 | 0.0\% | - | - | - | - | - | - |
| J2: Esso Roundabout | - | - | - | - | - | - | - | - | - | 94.5\% | 79 | 19 | 0 | 82.9 | - | - |
| 1/1 | Oxford Road Left Ahead | U | C2:A | 2 | 177 | - | 757 | 2007 | 1497 | 50.6\% | - | - | - | 1.1 | 5.2 | 7.8 |
| 1/2 | Oxford Road Ahead | U | C2:A | 2 | 177 | - | 961 | 2155 | 1607 | 59.8\% | - | - | - | 1.3 | 4.8 | 16.9 |
| 1/3 | Oxford Road Right | U | C2:F | 2 | 85 | - | 569 | 1973 | 715 | 79.6\% | - | - | - | 5.1 | 32.4 | 20.3 |
| 1/4 | Oxford Road Right | U | C2:F | 2 | 85 | - | 570 | 1973 | 715 | 79.7\% | - | - | - | 5.4 | 34.4 | 20.3 |
| 2/1 | Central Link Right | U | C2:G | 2 | 73 | - | 569 | 2029 | 634 | 89.7\% | - | - | - | 4.2 | 26.6 | 21.6 |
| 2/2 | Central Link Right | U | C2:G | 2 | 73 | - | 570 | 2024 | 632 | 90.1\% | - | - | - | 4.4 | 27.5 | 22.1 |
| 3/1 | Ped Crossing Ahead | U | C2:J | 2 | 204 | - | 814 | 1965 | 1687 | 48.3\% | - | - | - | 0.5 | 2.3 | 13.2 |
| 3/2 | Ped Crossing Ahead | U | C2:J | 2 | 204 | - | 1212 | 2105 | 1807 | 67.1\% | - | - | - | 1.1 | 3.3 | 4.0 |
| 4/1 | Services <br> Entry Left <br> Ahead | 0 | - | - | - | - | 98 | 2075 | 329 | 29.8\% | 79 | 19 | 0 | 0.3 | 11.3 | 1.2 |
| 6/1 | Oxford Road (sb) Left | U | C2:B | 2 | 88 | - | 714 | 2015 | 756 | 94.5\% | - | - | - | 11.8 | 59.7 | 30.3 |
| 6/2 | Oxford Road <br> (sb) Ahead | U | C2:B | 2 | 88 | - | 625 | 2105 | 789 | 79.2\% | - | - | - | 6.2 | 35.6 | 21.2 |
| 6/3 | Oxford Road (sb) Ahead | U | C2:B | 2 | 88 | - | 545 | 2105 | 789 | 69.0\% | - | - | - | 4.4 | 29.4 | 14.7 |
| 7/1 | Internal (eb) Ahead | U | C2:C | 2 | 41 | - | 173 | 2015 | 361 | 47.9\% | - | - | - | 2.4 | 50.7 | 6.4 |
| 7/2 | Internal (eb) Right | U | C2:C | 2 | 41 | - | 31 | 1889 | 338 | 9.2\% | - | - | - | 0.4 | 43.5 | 0.9 |
| 8/1 | Right Ahead | U | C2:E | 2 | 88 | - | 631 | 2105 | 789 | 79.9\% | - | - | - | 3.2 | 18.0 | 4.3 |
| 8/2 | Right Ahead | U | C2:E | 2 | 88 | - | 570 | 2105 | 789 | 72.2\% | - | - | - | 2.4 | 15.1 | 7.8 |
| 9/1 | Ahead Right | U | C2:H | 2 | 41 | - | 323 | 2011 | 360 | 89.6\% | - | - | - | 6.7 | 75.0 | 14.4 |


| 9/2 | Right | U | C2:H |  | 2 | 41 | - | 318 | 1973 | 353 | 90.0\% | - | - | - | 6.3 | 71.3 | 14.3 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 10/1 | Ahead | U | - |  | - | - | - | 886 | 2015 | 2015 | 44.0\% | - | - | - | 0.4 | 1.6 | 0.4 |
| 10/2 | Ahead | U | - |  | - | - | - | 1121 | 2155 | 2155 | 52.0\% | - | - | - | 0.5 | 1.7 | 0.5 |
| 10/3 | Ahead Right | U | - |  | - | - | - | 199 | 2047 | 2047 | 9.7\% | - | - | - | 0.1 | 1.0 | 0.1 |
| 11/2+11/1 | A41 entry Ahead Left | U | C2:D |  | 2 | 124 | - | 1282 | 2105:1965 | 846+618 | $\begin{aligned} & 87.6: \\ & 87.6 \% \end{aligned}$ | - | - | - | 10.6 | 29.6 | 24.2 |
| 11/3 | A41 entry Ahead | U | C2:D |  | 2 | 124 | - | 596 | 2105 | 1105 | 53.9\% | - | - | - | 3.7 | 22.4 | 13.7 |
| 12/1 | A41 exit | U | - |  | - | - | - | 814 | 1965 | 1965 | 41.4\% | - | - | - | 0.4 | 1.6 | 0.4 |
| Ped Link: P1 | Unnamed Ped Link | - | C2:K |  | 2 | 8 | - | 0 | - | 0 | 0.0\% | - | - | - | - | - | - |
| Ped Link: P2 | Unnamed Ped Link | - | C2:I |  | 2 | 88 | - | 0 | - | 0 | 0.0\% | - | - | - | - | - | - |
| J3: Oxford Road/ Lakeview Drive | - | - | - |  | - | - | - | - | - | - | 97.7\% | 0 | 0 | 0 | 79.1 | - | - |
| 1/1 | Oxford Road (nb) Ahead | U | C3:A |  | 2 | 101 | - | 721 | 2155 | 925 | 78.0\% | - | - | - | 4.5 | 22.6 | 24.1 |
| 1/2 | Oxford Road (nb) Ahead | U | C3:A |  | 2 | 101 | - | 729 | 2155 | 925 | 78.8\% | - | - | - | 4.5 | 22.3 | 24.3 |
| 1/3 | Oxford Road (nb) Ahead | U | C3:A |  | 2 | 101 | - | 712 | 2105 | 903 | 78.8\% | - | - | - | 4.6 | 23.4 | 23.8 |
| 1/4+1/5 | Oxford Road (nb) Right | U | C3:E |  | 2 | 19 | - | 263 | 1914:1914 | 161+167 | $\begin{array}{\|l\|} \hline 80.0: \\ 80.0 \% \end{array}$ | - | - | - | 4.9 | 67.4 | 6.4 |
| 2/2+2/1 | Left Ahead | U | $\begin{aligned} & \text { C3:B } \\ & \text { C3:C } \end{aligned}$ |  | 2 | 94:195 | - | 933 | 2105:1923 | 567+476 | $\begin{array}{\|l\|} 89.5: \\ 89.5 \% \end{array}$ | - | - | - | 10.1 | 39.1 | 27.7 |
| 2/3 | Ahead | U | C3:B |  | 2 | 94 | - | 749 | 2105 | 842 | 89.0\% | - | - | - | 13.2 | 63.6 | 28.1 |
| $2 / 4$ | Ahead | U | C3:B |  | 2 | 94 | - | 756 | 2105 | 842 | 89.8\% | - | - | - | 7.2 | 34.3 | 26.4 |
| 3/2+3/1 | Lakeview Drive Right Left | U | C3:D | C3:F | 2 | 91:116 | 25 | 925 | 2080:1940 | 0+947 | $\begin{gathered} 0.0: \\ 97.7 \% \end{gathered}$ | - | - | - | 18.6 | 72.3 | 42.3 |
| 3/3 | Lakeview Drive Right | U | C3:D |  | 2 | 91 | - | 695 | 2005 | 777 | 89.5\% | - | - | - | 11.1 | 57.7 | 27.4 |
| 4/1 | Lakeview Drive | U | - |  | - | - | - | 555 | 1965 | 1965 | 28.2\% | - | - | - | 0.2 | 1.3 | 1.3 |



Basic Results Summary
Scenario 2: '2026 AM BTM + Committed + 60k' (FG16: '2026 AM BTM + Committed + 60k', Plan 1: 'Control Plan') Network Layout Diagram


Basic Results Summary
Network Results

| Item | Lane Description | Lane Type | Full Phase | Arrow Phase | Num Greens | Total Green (s) | Arrow Green (s) | Demand Flow (pcu) | Sat Flow (pcu/Hr) | Capacity (pcu) | Deg <br> Sat <br> (\%) | Turners In Gaps (pcu) | Turners <br> When Unopposed (pcu) | Turners In Intergreen (pcu) | Total Delay (pcuHr) | Av. <br> Delay <br> Per PCU <br> (s/pcu) | Mean <br> Max Queue (pcu) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network | - | - | - |  | - | - | - | - | - | - | 92.9\% | 121 | 6 | 0 | 134.5 | - | - |
| J1: Pingle Drive / Oxford | - | - | - |  | - | - | - | - | - | - | 51.7\% | 0 | 0 | 0 | 9.5 | - | - |
| 1/1 | Oxford Road (nb) Ahead | U | C1:A |  | 2 | 194 | - | 436 | 1915 | 1564 | 27.9\% | - | - | - | 0.3 | 2.2 | 1.5 |
| 1/2 | Oxford Road (nb) Ahead | U | C1:A |  | 2 | 194 | - | 553 | 2055 | 1678 | 33.0\% | - | - | - | 0.3 | 2.2 | 1.8 |
| 1/3 | Oxford Road (nb) Right | U | C1:F |  | 2 | 39 | - | 71 | 2042 | 349 | 20.4\% | - | - | - | 0.9 | 44.6 | 2.7 |
| 1/4 | Oxford Road (nb) Right | U | C1:F |  | 2 | 39 | - | 62 | 2042 | 349 | 17.8\% | - | - | - | 0.7 | 39.7 | 2.3 |
| 2/2+2/1 | Pingle Drive Left | U | C1:E |  | 2 | 47 | - | 81 | 2005:1870 | 321+179 | $\begin{gathered} 16.2 \text { : } \\ 16.2 \% \end{gathered}$ | - | - | - | 1.0 | 43.5 | 1.6 |
| 2/3 | Pingle Drive Right | U | C1:D |  | 2 | 18 | - | 50 | 2067 | 172 | 29.0\% | - | - | - | 0.9 | 66.6 | 1.9 |
| 3/1 | Oxford Road (sb) Left | U | C1:C |  | 2 | 185 | - | 175 | 1908 | 1487 | 11.8\% | - | - | - | 0.2 | 4.6 | 1.5 |
| 3/2 | Oxford Road (sb) Ahead | U | C1:B |  | 2 | 169 | - | 776 | 2105 | 1500 | 51.7\% | - | - | - | 2.2 | 10.3 | 12.4 |
| 3/3 | Oxford Road (sb) Ahead | U | C1:B |  | 2 | 169 | - | 776 | 2105 | 1500 | 51.7\% | - | - | - | 2.2 | 10.3 | 12.4 |
| 4/1 | Oxford Road (nb) | U | - |  | - | - | - | 436 | 1940 | 1940 | 22.5\% | - | - | - | 0.1 | 1.2 | 0.1 |
| 4/2 | Oxford Road (nb) | U | - |  | - | - | - | 603 | 2080 | 2080 | 29.0\% | - | - | - | 0.2 | 1.2 | 0.2 |
| 5/1 | Pingle Drive | U | - |  | - | - | - | 246 | 1965 | 1965 | 12.5\% | - | - | - | 0.1 | 1.0 | 0.1 |
| 5/2 | Pingle Drive | U | - |  | - | - | - | 62 | 2105 | 2105 | 2.9\% | - | - | - | 0.0 | 0.9 | 0.0 |
| 6/1 | Right Turn Lane Right | U | C1:G |  | 2 | 33 | - | 71 | 1980 | 289 | 24.6\% | - | - | - | 0.2 | 9.7 | 0.2 |
| 6/2 | Right Turn Lane Right | U | C1:G |  | 2 | 33 | - | 62 | 1980 | 289 | 21.5\% | - | - | - | 0.2 | 9.4 | 0.2 |
| Ped Link: P1 | Unnamed Ped Link | - | C1:H |  | 2 | 21 | - | 0 | - | 0 | 0.0\% | - | - | - | - | - | - |


| Ped Link: P2 | Unnamed Ped Link | - | C1:I | 2 | 171 | - | 0 | - | 0 | 0.0\% | - | - | - | - | - | - |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Ped Link: P3 | Unnamed Ped Link | - | C1:J | 2 | 157 | - | 0 | - | 0 | 0.0\% | - | - | - | - | - | - |
| J2: Esso Roundabout | - | - | - | - | - | - | - | - | - | 90.4\% | 121 | 6 | 0 | 63.0 | - | - |
| 1/1 | Oxford Road Left Ahead | U | C2:A | 2 | 181 | - | 380 | 2007 | 1530 | 24.8\% | - | - | - | 0.5 | 4.7 | 3.4 |
| 1/2 | Oxford Road Ahead | U | C2:A | 2 | 181 | - | 358 | 2155 | 1643 | 21.8\% | - | - | - | 0.4 | 4.2 | 2.9 |
| 1/3 | Oxford Road Right | U | C2:F | 2 | 97 | - | 542 | 1973 | 814 | 66.6\% | - | - | - | 2.9 | 19.5 | 8.8 |
| 1/4 | Oxford Road Right | U | C2:F | 2 | 97 | - | 538 | 1973 | 814 | 66.1\% | - | - | - | 2.9 | 19.3 | 9.0 |
| 2/1 | Central Link Right | U | C2:G | 2 | 85 | - | 542 | 2029 | 736 | 73.7\% | - | - | - | 2.2 | 14.8 | 11.0 |
| 2/2 | Central Link Right | U | C2:G | 2 | 85 | - | 538 | 2024 | 734 | 73.3\% | - | - | - | 2.1 | 14.0 | 10.6 |
| 3/1 | Ped Crossing Ahead | U | C2:J | 2 | 204 | - | 614 | 1965 | 1687 | 36.4\% | - | - | - | 0.3 | 1.8 | 4.1 |
| 3/2 | Ped Crossing Ahead | U | C2:J | 2 | 204 | - | 1081 | 2105 | 1807 | 59.8\% | - | - | - | 0.8 | 2.6 | 2.1 |
| 4/1 | Services <br> Entry Left <br> Ahead | O | - | - | - | - | 127 | 2058 | 506 | 25.1\% | 121 | 6 | 0 | 0.2 | 5.0 | 0.8 |
| 6/1 | Oxford Road (sb) Left | U | C2:B | 2 | 80 | - | 533 | 2015 | 688 | 77.4\% | - | - | - | 6.2 | 42.0 | 18.7 |
| 6/2 | Oxford Road <br> (sb) Ahead | U | C2:B | 2 | 80 | - | 602 | 2105 | 719 | 83.7\% | - | - | - | 7.5 | 44.8 | 22.4 |
| 6/3 | Oxford Road (sb) Ahead | U | C2:B | 2 | 80 | - | 498 | 2105 | 719 | 69.2\% | - | - | - | 5.3 | 38.5 | 16.5 |
| 7/1 | Internal (eb) Ahead | U | C2:C | 2 | 37 | - | 82 | 2015 | 327 | 25.0\% | - | - | - | 1.0 | 44.5 | 2.7 |
| 7/2 | Internal (eb) Right | U | C2:C | 2 | 37 | - | 55 | 1889 | 307 | 17.9\% | - | - | - | 0.8 | 50.3 | 1.7 |
| 8/1 | Right Ahead | U | C2:E | 2 | 80 | - | 612 | 2105 | 719 | 85.1\% | - | - | - | 4.4 | 26.0 | 6.0 |
| 8/2 | Right Ahead | U | C2:E | 2 | 80 | - | 543 | 2105 | 719 | 75.5\% | - | - | - | 2.6 | 17.4 | 4.5 |
| 9/1 | Ahead Right | U | C2:H | 2 | 37 | - | 249 | 2014 | 327 | 76.1\% | - | - | - | 4.3 | 62.3 | 9.6 |



| Basic Results Summary |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 4/2 | Lakeview Drive | U | - |  | - | - | - | 560 | 1965 | 1965 | 28.5\% | - | - | - | 0.2 | 1.3 | 0.2 |
| Ped Link: P1 | Unnamed Ped Link | - | C3:G |  | 1 | 35 | - | 0 | - | 0 | 0.0\% | - | - | - | - | - | - |
| Ped Link: P2 | Unnamed Ped Link | - | C3:H |  | 2 | 77 | - | 0 | - | 0 | 0.0\% | - | - | - | - | - | - |
| Ped Link: P3 | Unnamed Ped Link | - | C3:I |  | 2 | 77 | - | 0 | - | 0 | 0.0\% | - | - | - | - | - | - |
| Ped Link: P4 | Unnamed Ped Link | - | C3:K |  | 2 | 101 | - | 0 | - | 0 | 0.0\% | - | - | - | - | - | - |
| J4: Oxford Road / Saxon Fields | - | - | - |  | - | - | - | - | - | - | 75.1\% | 0 | 0 | 0 | 17.6 | - | - |
| 1/2+1/1 | Oxford Road Ahead Left | U | C4:C | C4:J | 2 | 148 | 0 | 821 | 2205:1709 | 1364+27 | $\begin{gathered} 59.0: \\ 59.0 \% \end{gathered}$ | - | - | - | 3.7 | 16.4 | 17.3 |
| 1/3 | Oxford Road Ahead | U | C4:C |  | 2 | 148 | - | 678 | 2205 | 1378 | 49.2\% | - | - | - | 2.8 | 14.8 | 13.1 |
| 1/4 | Oxford Road Ahead | U | C4:C |  | 2 | 148 | - | 644 | 2105 | 1316 | 49.0\% | - | - | - | 2.7 | 14.9 | 12.5 |
| 3/2+3/1 | Saxon Fields Left Right | U | C4:D | C4:I | 2 | 28 | 0 | 298 | 1619:1894 | 202+194 | $\begin{gathered} 75.1: \\ 75.1 \% \end{gathered}$ | - | - | - | 5.6 | 68.0 | 6.4 |
| 4/1 | Ahead | U | C4:A |  | 2 | 178 | - | 960 | 2205 | 1654 | 58.0\% | - | - | - | 1.0 | 3.6 | 11.9 |
| 4/2 | Ahead | U | C4:A |  | 2 | 178 | - | 1027 | 2205 | 1654 | 62.1\% | - | - | - | 1.3 | 4.7 | 27.4 |
| 4/3 | Right | U | C4:B |  | 2 | 28 | - | 37 | 1874 | 234 | 15.8\% | - | - | - | 0.5 | 50.0 | 1.3 |
| Ped Link: P1 | Unnamed Ped Link | - | C4:G |  | 2 | 10 | - | 0 | - | 0 | 0.0\% | - | - | - | - | - | - |
| Ped Link: P2 | Unnamed Ped Link | - | C4:H |  | 2 | 26 | - | 0 | - | 0 | 0.0\% | - | - | - | - | - | - |
| Ped Link: P3 | Unnamed Ped Link | - | C4:F |  | 2 | 148 | - | 0 | - | 0 | 0.0\% | - | - | - | - | - | - |
| Ped Link: P4 | Unnamed Ped Link | - | C4:E |  | 2 | 16 | - | 0 | - | 0 | 0.0\% | - | - | - | - | - | - |
|  |  |  | Stream Strea | $\begin{aligned} & \text { PRC } \\ & \text { PRC } \\ & \text { PRC } \\ & \text { PRC } \\ & \text { PRC } \end{aligned}$ | innal | es (\%): <br> es (\%): <br> es (\%): <br> es (\%): <br> es (\%): <br> (\%): | $\begin{array}{r} 73.9 \\ -0.5 \\ 50.4 \\ -3.2 \\ 19.8 \\ -3.2 \end{array}$ |  | Delay for Sign Delay for Sign Delay for Sign Delay for Sign Delay for Sign Total Delay Ov | ed Lanes (p ed Lanes (p ed Lanes (p ed Lanes (p ed Lanes (p All Lanes(p | uHr): <br> uHr): <br> uHr): <br> uHr): <br> uHr): <br> uHr): | $\begin{array}{r} 9.10 \\ 61.12 \\ 1.10 \\ 43.73 \\ 17.63 \\ 134.49 \end{array}$ | Cycle Time (s): <br> Cycle Time (s): <br> Cycle Time (s): <br> Cycle Time (s): <br> Cycle Time (s): |  |  |  |  |

## motion

A41 / Bicester Park \& Ride / Vendee Drive

## Junctions 9

## ARCADY 9 - Roundabout Module

## Version: 9.0.2.5947

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Filename: Vendee Drive - A41 (With Consented Improvements) - 2018-06-21.j9
Path: N:|Projects\Imbic2 170211\Analysis\Modelling\Vendee Drive
Report generation date: 03/07/2018 10:48:27

```
„2026 BTM, AM
"2026 BTM, PM
"2026 Baseline, AM
"2026 Baseline, PM
"2026 BTM + 60sqm, AM
"2026 BTM + 60sqm, PM
```

Summary of junction performance

|  | AM |  |  |  |  |  |  | PM |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Queue (Veh) | Delay <br> (s) | RFC | LOS | Junction <br> Delay (s) | Junction LOS | Network Residual Capacity | Queue (Veh) | Delay (s) | RFC | LOS | Junction <br> Delay (s) | $\begin{aligned} & \text { Junction } \\ & \text { LOS } \end{aligned}$ | Network Residual Capacity |
|  | 2026 BTM |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Arm 1 | 0.3 | 3.87 | 0.22 | A | 3.81 | A | $\begin{gathered} 38 \% \\ \text { [Arm 4] } \end{gathered}$ | 0.3 | 5.10 | 0.25 | A | 5.47 | A | $\begin{gathered} 12 \% \\ {[\text { Arm 5] }} \end{gathered}$ |
| Arm 2 | 2.0 | 3.54 | 0.67 | A |  |  |  | 3.1 | 4.52 | 0.76 | A |  |  |  |
| Arm 3 | 0.2 | 5.67 | 0.14 | A |  |  |  | 0.4 | 7.20 | 0.29 | A |  |  |  |
| Arm 4 | 1.9 | 3.96 | 0.66 | A |  |  |  | 3.3 | 6.39 | 0.77 | A |  |  |  |
| Arm 5 | 0.0 | 6.49 | 0.02 | A |  |  |  | 0.1 | 13.19 | 0.12 | B |  |  |  |
|  | 2026 Baseline |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Arm 1 | 0.5 | 4.81 | 0.31 | A | 4.29 | A | $\begin{gathered} 28 \% \\ \text { [Arm 4] } \end{gathered}$ | 0.3 | 5.20 | 0.26 | A | 7.28 | A | 8 \% |
| Arm 2 | 2.1 | 3.64 | 0.68 | A |  |  |  | 5.4 | 7.00 | 0.84 | A |  |  |  |
| Arm 3 | 0.2 | 5.80 | 0.14 | A |  |  |  | 0.6 | 10.90 | 0.38 | B |  |  |  |
| Arm 4 | 2.6 | 4.79 | 0.72 | A |  |  |  | 3.9 | 7.39 | 0.80 | A |  |  | [Arm 5] |
| Arm 5 | 0.0 | 7.73 | 0.02 | A |  |  |  | 0.2 | 16.18 | 0.14 | C |  |  |  |
|  | 2026 BTM + 60sqm |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Arm 1 | 0.5 | 5.28 | 0.35 | A | 4.54 | A | 25 \% | 0.4 | 5.24 | 0.26 | A | 8.37 | A | 7 \% |
| Arm 2 | 2.1 | 3.66 | 0.68 | A |  |  |  | 6.8 | 8.56 | 0.87 | A |  |  |  |
| Arm 3 | 0.2 | 5.83 | 0.14 | A |  |  |  | 0.7 | 13.14 | 0.42 | B |  |  |  |
| Arm 4 | 2.9 | 5.24 | 0.74 | A |  |  | [Arm 4] | 4.1 | 7.80 | 0.81 | A |  |  | [Arm 5] |
| Arm 5 | 0.0 | 8.33 | 0.02 | A |  |  |  | 0.2 | 17.54 | 0.15 | C |  |  |  |

There are warnings associated with one or more model runs - see the 'Data Errors and Warnings' tables for each Analysis or Demand Set.

[^1]
## File summary

File Description

| Title | Vemdee Drove / A41 - Improved |
| :--- | :--- |
| Location | Bicester |
| Site number |  |
| Date | $20 / 07 / 2017$ |
| Version |  |
| Status | (new file) |
| Identifier |  |
| Client |  |
| Jobnumber |  |
| Enumerator | MOTION\klewis |
| Description |  |

## Units

| Distance units | Speed units | Traffic units input | Traffic units results | Flow units | Average delay units | Total delay units | Rate of delay units |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| m | kph | Veh | Veh | perHour | s | -Min | perMin |

## Analysis Options

| Vehicle <br> length (m) | Calculate Queue <br> Percentiles | Calculate detailed <br> queueing delay | Calculate residual <br> capacity | Residual capacity <br> criteria type | RFC <br> Threshold | Average Delay <br> threshold (s) | Queue threshold <br> (PCU) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 5.75 |  |  | $\checkmark$ | Delay | 0.85 | 36.00 | 20.00 |

## Demand Set Summary

| ID | Scenario name | Time Period name | Traffic profile type | Start time (HH:mm) | Finish time (HH:mm) | Time period length (min) | Time segment length (min) | Run automatically |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| D1 | 2026 BTM | AM | FLAT | 07:45 | 09:15 | 90 | 15 | $\checkmark$ |
| D2 | 2026 BTM | PM | FLAT | 16:45 | 18:15 | 90 | 15 | $\checkmark$ |
| D3 | 2026 Baseline | AM | FLAT | 07:45 | 09:15 | 90 | 15 | $\checkmark$ |
| D4 | 2026 Baseline | PM | FLAT | 16:45 | 18:15 | 90 | 15 | $\checkmark$ |
| D5 | 2026 BTM + 60sqm | AM | FLAT | 07:45 | 09:15 | 90 | 15 | $\checkmark$ |
| D6 | 2026 BTM + 60sqm | PM | FLAT | 16:45 | 18:15 | 90 | 15 | $\checkmark$ |

## Analysis Set Details

| ID | Include in report | Network flow scaling factor (\%) | Network capacity scaling factor (\%) |
| :---: | :---: | :---: | :---: |
| A1 | $\checkmark$ | 100.000 | 100.000 |

THE FUTURE

## 2026 BTM, AM

Data Errors and Warnings

| Severity | Area | Item | Description |
| :--- | :--- | :--- | :--- |
| Warning | Geometry | Arm 1 - Roundabout <br> Geometry | Effective flare length is over 30m, which is outside the normal range. Treat capacities with increasing caution. |
| Warning | Geometry | Arm 2 - Roundabout <br> Geometry | Effective flare length is over 30m, which is outside the normal range. Treat capacities with increasing caution. |
| Warning | Geometry | Arm 3-Roundabout <br> Geometry | Effective flare length is over 30m, which is outside the normal range. Treat capacities with increasing caution. |

## Junction Network

## Junctions

| Junction | Name | Junction Type | Arm order | Junction Delay (s) | Junction LOS |
| :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | untitled | Standard Roundabout | $1,2,3,4,5$ | 3.81 | A |

## Junction Network Options

| Driving side | Lighting | Network residual capacity (\%) | First arm reaching threshold |
| :---: | :---: | :---: | :---: |
| Left | Normal/unknown | 38 | Arm 4 |

## Arms

## Arms

| Arm | Name | Description |
| :---: | :--- | :--- |
| $\mathbf{1}$ | Vendee Drive |  |
| $\mathbf{2}$ | A41 |  |
| $\mathbf{3}$ | Unnamed Road |  |
| $\mathbf{4}$ | A41 |  |
| $\mathbf{5}$ | Park \& Ride |  |

Roundabout Geometry

| Arm | V - Approach road halfwidth (m) | E-Entry width (m) | I' - Effective flare length (m) | R - Entry radius (m) | D - Inscribed circle diameter (m) | PHI - Conflict (entry) angle (deg) | Exit only |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 3.75 | 8.20 | 92.0 | 20.0 | 70.0 | 35.0 |  |
| 2 | 7.50 | 12.00 | 38.0 | 36.0 | 70.0 | 18.0 |  |
| 3 | 3.50 | 10.50 | 32.0 | 20.0 | 70.0 | 22.5 |  |
| 4 | 7.00 | 12.00 | 25.0 | 35.0 | 70.0 | 25.0 |  |
| 5 | 3.50 | 8.00 | 14.0 | 15.0 | 70.0 | 30.0 |  |

## Slope / Intercept / Capacity

Roundabout Slope and Intercept used in model

| Arm | Final slope | Final intercept (PCU/hr) |
| :---: | :---: | :---: |
| $\mathbf{1}$ | 0.590 | 2264 |
| $\mathbf{2}$ | 0.799 | 3468 |
| $\mathbf{3}$ | 0.617 | 2368 |
| $\mathbf{4}$ | 0.745 | 3161 |
| $\mathbf{5}$ | 0.502 | 1704 |

The slope and intercept shown above include any corrections and adjustments

## Traffic Demand

Demand Set Details

| ID | Scenario <br> name | Time Period <br> name | Traffic profile <br> type | Start time <br> (HH:mm) | Finish time <br> (HH:mm) | Time period length <br> $(\mathbf{m i n})$ | Time segment length <br> (min) | Run <br> automatically |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| D1 | 2026 BTM | AM | FLAT | $07: 45$ | $09: 15$ | 90 | 15 |  |


| Vehicle mix varies over turn | Vehicle mix varies over entry | Vehicle mix source | PCU Factor for a HV (PCU) |
| :---: | :---: | :---: | :---: |
| $\checkmark$ | $\checkmark$ | HV Percentages | 2.00 |

Demand overview (Traffic)

| Arm | Linked arm | Profile type | Use O-D data | Average Demand (Veh/hr) | Scaling Factor (\%) |
| :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ |  | FLAT | $\checkmark$ | 261 | 100.000 |
| $\mathbf{2}$ |  | FLAT | $\checkmark$ | 2055 | 100.000 |
| $\mathbf{3}$ |  | FLAT | $\checkmark$ | 100 | 100.000 |
| $\mathbf{4}$ |  | FLAT | $\checkmark$ | 1745 | 100.000 |
| $\mathbf{5}$ |  | FLAT | $\checkmark$ | 11 | 100.000 |

## Origin-Destination Data

Demand (Veh/hr)

|  | To |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| From |  | $\mathbf{1}$ | $\mathbf{2}$ | $\mathbf{3}$ | $\mathbf{4}$ | $\mathbf{5}$ |
|  | $\mathbf{1}$ | 0 | 152 | 2 | 107 | 0 |
|  | $\mathbf{2}$ | 180 | 187 | 0 | 1668 | 20 |
|  | $\mathbf{3}$ | 0 | 87 | 0 | 13 | 0 |
|  | $\mathbf{4}$ | 261 | 1259 | 167 | 3 | 55 |
|  | $\mathbf{5}$ | 1 | 0 | 0 | 10 | 0 |

## Vehicle Mix

Heavy Vehicle Percentages

|  | To |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| From |  | $\mathbf{1}$ | $\mathbf{2}$ | $\mathbf{3}$ | $\mathbf{4}$ | $\mathbf{5}$ |
|  | $\mathbf{1}$ | 0 | 0 | 0 | 0 | 0 |
|  | $\mathbf{2}$ | 6 | 0 | 0 | 6 | 0 |
|  | $\mathbf{3}$ | 0 | 34 | 0 | 7 | 0 |
|  | $\mathbf{4}$ | 2 | 6 | 1 | 0 | 0 |
|  | $\mathbf{5}$ | 0 | 0 | 0 | 0 | 0 |

## Results

Results Summary for whole modelled period

| Arm | Max RFC | Max delay (s) | Max Queue (Veh) | Max LOS | Average Demand <br> (Veh/hr) | Total Junction <br> Arrivals (Veh) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | 0.22 | 3.87 | 0.3 | A | 261 | 392 |
| $\mathbf{2}$ | 0.67 | 3.54 | 2.0 | A | 2055 | 3082 |
| $\mathbf{3}$ | 0.14 | 5.67 | 0.2 | A | 100 | 150 |
| $\mathbf{4}$ | 0.66 | 3.96 | 1.9 | A | 1745 | 2617 |
| $\mathbf{5}$ | 0.02 | 6.49 | 0.0 |  | 11 | 17 |

THE FUTURE

## Main Results for each time segment

07:45-08:00

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 261 | 65 | 1705 | 1195 | 0.218 | 260 | 440 | 0.0 | 0.3 | 3.846 | A |
| 2 | 2055 | 514 | 288 | 3071 | 0.669 | 2047 | 1678 | 0.0 | 2.0 | 3.489 | A |
| 3 | 100 | 25 | 2166 | 739 | 0.135 | 99 | 168 | 0.0 | 0.2 | 5.628 | A |
| 4 | 1745 | 436 | 472 | 2655 | 0.657 | 1737 | 1794 | 0.0 | 1.9 | 3.893 | A |
| 5 | 11 | 3 | 2135 | 571 | 0.019 | 11 | 75 | 0.0 | 0.0 | 6.432 | A |

08:00-08:15

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 261 | 65 | 1713 | 1190 | 0.219 | 261 | 442 | 0.3 | 0.3 | 3.874 | A |
| 2 | 2055 | 514 | 289 | 3070 | 0.669 | 2055 | 1685 | 2.0 | 2.0 | 3.545 | A |
| 3 | 100 | 25 | 2175 | 734 | 0.136 | 100 | 169 | 0.2 | 0.2 | 5.674 | A |
| 4 | 1745 | 436 | 474 | 2653 | 0.658 | 1745 | 1801 | 1.9 | 1.9 | 3.963 | A |
| 5 | 11 | 3 | 2144 | 566 | 0.019 | 11 | 75 | 0.0 | 0.0 | 6.489 | A |

08:15-08:30

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 261 | 65 | 1713 | 1190 | 0.219 | 261 | 442 | 0.3 | 0.3 | 3.874 | A |
| 2 | 2055 | 514 | 289 | 3070 | 0.669 | 2055 | 1685 | 2.0 | 2.0 | 3.545 | A |
| 3 | 100 | 25 | 2175 | 734 | 0.136 | 100 | 169 | 0.2 | 0.2 | 5.674 | A |
| 4 | 1745 | 436 | 474 | 2653 | 0.658 | 1745 | 1801 | 1.9 | 1.9 | 3.963 | A |
| 5 | 11 | 3 | 2144 | 566 | 0.019 | 11 | 75 | 0.0 | 0.0 | 6.490 | A |

08:30-08:45

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity <br> (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 261 | 65 | 1713 | 1190 | 0.219 | 261 | 442 | 0.3 | 0.3 | 3.874 | A |
| 2 | 2055 | 514 | 289 | 3070 | 0.669 | 2055 | 1685 | 2.0 | 2.0 | 3.545 | A |
| 3 | 100 | 25 | 2175 | 734 | 0.136 | 100 | 169 | 0.2 | 0.2 | 5.675 | A |
| 4 | 1745 | 436 | 474 | 2653 | 0.658 | 1745 | 1801 | 1.9 | 1.9 | 3.963 | A |
| 5 | 11 | 3 | 2144 | 566 | 0.019 | 11 | 75 | 0.0 | 0.0 | 6.490 | A |

08:45-09:00

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity <br> (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 261 | 65 | 1713 | 1190 | 0.219 | 261 | 442 | 0.3 | 0.3 | 3.874 | A |
| 2 | 2055 | 514 | 289 | 3070 | 0.669 | 2055 | 1685 | 2.0 | 2.0 | 3.545 | A |
| 3 | 100 | 25 | 2175 | 734 | 0.136 | 100 | 169 | 0.2 | 0.2 | 5.675 | A |
| 4 | 1745 | 436 | 474 | 2653 | 0.658 | 1745 | 1801 | 1.9 | 1.9 | 3.963 | A |
| 5 | 11 | 3 | 2144 | 566 | 0.019 | 11 | 75 | 0.0 | 0.0 | 6.490 | A |

09:00-09:15

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 261 | 65 | 1713 | 1190 | 0.219 | 261 | 442 | 0.3 | 0.3 | 3.874 | A |
| 2 | 2055 | 514 | 289 | 3070 | 0.669 | 2055 | 1685 | 2.0 | 2.0 | 3.545 | A |
| 3 | 100 | 25 | 2175 | 734 | 0.136 | 100 | 169 | 0.2 | 0.2 | 5.675 | A |
| 4 | 1745 | 436 | 474 | 2653 | 0.658 | 1745 | 1801 | 1.9 | 1.9 | 3.963 | A |
| 5 | 11 | 3 | 2144 | 566 | 0.019 | 11 | 75 | 0.0 | 0.0 | 6.490 | A |

THE FUTURE

## 2026 BTM, PM

Data Errors and Warnings

| Severity | Area | Item | Description |
| :--- | :--- | :--- | :--- |
| Warning | Geometry | Arm 1 - Roundabout <br> Geometry | Effective flare length is over 30m, which is outside the normal range. Treat capacities with increasing caution. |
| Warning | Geometry | Arm 2 - Roundabout <br> Geometry | Effective flare length is over 30m, which is outside the normal range. Treat capacities with increasing caution. |
| Warning | Geometry | Arm 3-Roundabout <br> Geometry | Effective flare length is over 30m, which is outside the normal range. Treat capacities with increasing caution. |

## Junction Network

## Junctions

| Junction | Name | Junction Type | Arm order | Junction Delay (s) | Junction LOS |
| :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | untitled | Standard Roundabout | $1,2,3,4,5$ | 5.47 | A |

## Junction Network Options

| Driving side | Lighting | Network residual capacity (\%) | First arm reaching threshold |
| :---: | :---: | :---: | :---: |
| Left | Normal/unknown | 12 | Arm 5 |

## Traffic Demand

Demand Set Details

| ID | Scenario <br> name | Time Period <br> name | Traffic profile <br> type | Start time <br> (HH:mm) | Finish time <br> (HH:mm) | Time period length <br> $(\mathbf{m i n})$ | Time segment length <br> (min) | Run <br> automatically |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| D2 | 2026 BTM | PM | FLAT | $16: 45$ | $18: 15$ | 90 |  |  |


| Vehicle mix varies over turn | Vehicle mix varies over entry | Vehicle mix source | PCU Factor for a HV (PCU) |
| :---: | :---: | :---: | :---: |
| $\checkmark$ | $\checkmark$ | HV Percentages | 2.00 |

Demand overview (Traffic)

| Arm | Linked arm | Profile type | Use O-D data | Average Demand (Veh/hr) | Scaling Factor (\%) |
| :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ |  | FLAT | $\checkmark$ | 233 | 100.000 |
| $\mathbf{2}$ |  | FLAT | $\checkmark$ | 2505 | 100.000 |
| $\mathbf{3}$ |  | FLAT | $\checkmark$ | 200 | 100.000 |
| $\mathbf{4}$ |  | FLAT | $\checkmark$ | 1896 | 100.000 |
| $\mathbf{5}$ |  | FLAT | $\checkmark$ | 37 | 100.000 |

## Origin-Destination Data

Demand (Veh/hr)

|  | To |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| From |  | $\mathbf{1}$ | $\mathbf{2}$ | $\mathbf{3}$ | $\mathbf{4}$ | $\mathbf{5}$ |
|  | $\mathbf{1}$ | 0 | 97 | 0 | 136 | 0 |
|  | $\mathbf{2}$ | 194 | 549 | 0 | 1751 | 11 |
|  | $\mathbf{3}$ | 3 | 82 | 0 | 114 | 1 |
|  | $\mathbf{4}$ | 375 | 1473 | 40 | 0 | 8 |
|  | $\mathbf{5}$ | 5 | 0 | 0 | 32 | 0 |

THE FUTURE

Heavy Vehicle Percentages

|  | To |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| From |  | $\mathbf{1}$ | $\mathbf{2}$ | $\mathbf{3}$ | $\mathbf{4}$ | $\mathbf{5}$ |
|  | $\mathbf{1}$ | 0 | 0 | 0 | 1 | 0 |
|  | $\mathbf{2}$ | 0 | 0 | 0 | 0 | 0 |
|  | $\mathbf{3}$ | 0 | 1 | 0 | 4 | 0 |
|  | $\mathbf{4}$ | 0 | 4 | 0 | 0 | 0 |
|  | $\mathbf{5}$ | 0 | 0 | 0 | 0 | 0 |

## Results

Results Summary for whole modelled period

| Arm | Max RFC | Max delay (s) | Max Queue (Veh) | Max LOS | Average Demand <br> (Veh/hr) | Total Junction <br> Arrivals (Veh) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | 0.25 | 5.10 | 0.3 | A | 233 | 349 |
| $\mathbf{2}$ | 0.76 | 4.52 | 3.1 | A | 2505 | 3758 |
| $\mathbf{3}$ | 0.29 | 7.20 | 0.4 | A | 200 | 300 |
| $\mathbf{4}$ | 0.77 | 6.39 | 3.3 | A | 1896 | 2844 |
| $\mathbf{5}$ | 0.12 | 13.19 | 0.1 | B | 37 | 56 |

## Main Results for each time segment

16:45-17:00

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 233 | 58 | 2162 | 948 | 0.246 | 232 | 573 | 0.0 | 0.3 | 5.019 | A |
| 2 | 2505 | 626 | 207 | 3302 | 0.759 | 2493 | 2187 | 0.0 | 3.1 | 4.384 | A |
| 3 | 200 | 50 | 2659 | 708 | 0.283 | 198 | 40 | 0.0 | 0.4 | 7.046 | A |
| 4 | 1896 | 474 | 836 | 2462 | 0.770 | 1883 | 2022 | 0.0 | 3.3 | 6.087 | A |
| 5 | 37 | 9 | 2699 | 319 | 0.116 | 36 | 20 | 0.0 | 0.1 | 12.728 | B |

17:00-17:15

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 233 | 58 | 2176 | 939 | 0.248 | 233 | 577 | 0.3 | 0.3 | 5.096 | A |
| 2 | 2505 | 626 | 208 | 3301 | 0.759 | 2505 | 2201 | 3.1 | 3.1 | 4.521 | A |
| 3 | 200 | 50 | 2673 | 700 | 0.286 | 200 | 40 | 0.4 | 0.4 | 7.201 | A |
| 4 | 1896 | 474 | 840 | 2459 | 0.771 | 1896 | 2033 | 3.3 | 3.3 | 6.385 | A |
| 5 | 37 | 9 | 2716 | 310 | 0.119 | 37 | 20 | 0.1 | 0.1 | 13.182 | B |

17:15-17:30

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 233 | 58 | 2176 | 939 | 0.248 | 233 | 577 | 0.3 | 0.3 | 5.097 | A |
| 2 | 2505 | 626 | 208 | 3301 | 0.759 | 2505 | 2201 | 3.1 | 3.1 | 4.523 | A |
| 3 | 200 | 50 | 2673 | 700 | 0.286 | 200 | 40 | 0.4 | 0.4 | 7.202 | A |
| 4 | 1896 | 474 | 840 | 2459 | 0.771 | 1896 | 2033 | 3.3 | 3.3 | 6.390 | A |
| 5 | 37 | 9 | 2716 | 310 | 0.119 | 37 | 20 | 0.1 | 0.1 | 13.187 | B |

17:30-17:45

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 233 | 58 | 2176 | 939 | 0.248 | 233 | 577 | 0.3 | 0.3 | 5.097 | A |
| 2 | 2505 | 626 | 208 | 3301 | 0.759 | 2505 | 2201 | 3.1 | 3.1 | 4.523 | A |
| 3 | 200 | 50 | 2673 | 700 | 0.286 | 200 | 40 | 0.4 | 0.4 | 7.202 | A |
| 4 | 1896 | 474 | 840 | 2459 | 0.771 | 1896 | 2033 | 3.3 | 3.3 | 6.390 | A |
| 5 | 37 | 9 | 2716 | 310 | 0.119 | 37 | 20 | 0.1 | 0.1 | 13.188 | B |

17:45-18:00

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 233 | 58 | 2176 | 939 | 0.248 | 233 | 577 | 0.3 | 0.3 | 5.097 | A |
| 2 | 2505 | 626 | 208 | 3301 | 0.759 | 2505 | 2201 | 3.1 | 3.1 | 4.523 | A |
| 3 | 200 | 50 | 2673 | 700 | 0.286 | 200 | 40 | 0.4 | 0.4 | 7.203 | A |
| 4 | 1896 | 474 | 840 | 2459 | 0.771 | 1896 | 2033 | 3.3 | 3.3 | 6.393 | A |
| 5 | 37 | 9 | 2716 | 310 | 0.119 | 37 | 20 | 0.1 | 0.1 | 13.188 | B |

18:00-18:15

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start <br> queue <br> (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 233 | 58 | 2176 | 939 | 0.248 | 233 | 577 | 0.3 | 0.3 | 5.097 | A |
| 2 | 2505 | 626 | 208 | 3301 | 0.759 | 2505 | 2201 | 3.1 | 3.1 | 4.523 | A |
| 3 | 200 | 50 | 2673 | 700 | 0.286 | 200 | 40 | 0.4 | 0.4 | 7.203 | A |
| 4 | 1896 | 474 | 840 | 2459 | 0.771 | 1896 | 2033 | 3.3 | 3.3 | 6.393 | A |
| 5 | 37 | 9 | 2716 | 310 | 0.119 | 37 | 20 | 0.1 | 0.1 | 13.189 | B |

THE FUTURE

## 2026 Baseline, AM

Data Errors and Warnings

| Severity | Area | Item | Description |
| :--- | :--- | :--- | :--- |
| Warning | Geometry | Arm 1 - Roundabout <br> Geometry | Effective flare length is over 30m, which is outside the normal range. Treat capacities with increasing caution. |
| Warning | Geometry | Arm 2 - Roundabout <br> Geometry | Effective flare length is over 30m, which is outside the normal range. Treat capacities with increasing caution. |
| Warning | Geometry | Arm 3-Roundabout <br> Geometry | Effective flare length is over 30m, which is outside the normal range. Treat capacities with increasing caution. |

## Junction Network

## Junctions

| Junction | Name | Junction Type | Arm order | Junction Delay (s) | Junction LOS |
| :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | untitled | Standard Roundabout | $1,2,3,4,5$ | 4.29 | A |

## Junction Network Options

| Driving side | Lighting | Network residual capacity (\%) | First arm reaching threshold |
| :---: | :---: | :---: | :---: |
| Left | Normal/unknown | 28 | Arm 4 |

## Traffic Demand

Demand Set Details

| ID | Scenario <br> name | Time Period <br> name | Traffic profile <br> type | Start time <br> $(H H: m m)$ | Finish time <br> $(\mathbf{H H}: \mathbf{m m})$ | Time period length <br> $(\mathbf{m i n})$ | Time segment length <br> (min) | Run <br> automatically |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| D3 | 2026 Baseline | AM | FLAT | $07: 45$ | $09: 15$ | 90 | 15 |  |


| Vehicle mix varies over turn | Vehicle mix varies over entry | Vehicle mix source | PCU Factor for a HV (PCU) |
| :---: | :---: | :---: | :---: |
| $\checkmark$ | $\checkmark$ | HV Percentages | 2.00 |

Demand overview (Traffic)

| Arm | Linked arm | Profile type | Use O-D data | Average Demand (Veh/hr) | Scaling Factor (\%) |
| :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ |  | FLAT | $\checkmark$ | 343 | 100.000 |
| $\mathbf{2}$ |  | FLAT | $\checkmark$ | 2080 | 100.000 |
| $\mathbf{3}$ |  | FLAT | $\checkmark$ | 100 | 100.000 |
| $\mathbf{4}$ |  | FLAT | $\checkmark$ | 1931 | 100.000 |
| $\mathbf{5}$ |  | FLAT | $\checkmark$ | 11 | 100.000 |

## Origin-Destination Data

Demand (Veh/hr)

|  | To |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| From |  | $\mathbf{1}$ | $\mathbf{2}$ | $\mathbf{3}$ | $\mathbf{4}$ | $\mathbf{5}$ |  |
|  | $\mathbf{1}$ | 0 | 234 | 2 | 107 | 0 |  |
|  | $\mathbf{2}$ | 188 | 187 | 0 | 1685 | 20 |  |
|  | $\mathbf{3}$ | 0 | 87 | 0 | 13 | 0 |  |
|  | $\mathbf{4}$ | 261 | 1445 | 167 | 3 | 55 |  |
|  | $\mathbf{5}$ | $\mathbf{1}$ | 0 | 0 | 10 | 0 |  |

THE FUTURE

Heavy Vehicle Percentages

|  | To |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| From |  | $\mathbf{1}$ | $\mathbf{2}$ | $\mathbf{3}$ | $\mathbf{4}$ | $\mathbf{5}$ |
|  | $\mathbf{1}$ | 0 | 0 | 0 | 0 | 0 |
|  | $\mathbf{2}$ | 6 | 0 | 0 | 6 | 0 |
|  | $\mathbf{3}$ | 0 | 34 | 0 | 8 | 0 |
|  | $\mathbf{4}$ | 2 | 4 | 1 | 0 | 0 |
|  | $\mathbf{5}$ | 0 | 0 | 0 | 0 | 0 |

## Results

Results Summary for whole modelled period

| Arm | Max RFC | Max delay (s) | Max Queue (Veh) | Max LOS | Average Demand <br> (Veh/hr) | Total Junction <br> Arrivals (Veh) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | 0.31 | 4.81 | 0.5 | A | 343 | 515 |
| $\mathbf{2}$ | 0.68 | 3.64 | 2.1 | A | 2080 | 3120 |
| $\mathbf{3}$ | 0.14 | 5.80 | 0.2 | A | 100 | 150 |
| $\mathbf{4}$ | 0.72 | 4.79 | 2.6 | A | 1931 | 2897 |
| $\mathbf{5}$ | 0.02 | 7.73 | 0.0 |  | 11 | 17 |

## Main Results for each time segment

07:45-08:00

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 343 | 86 | 1889 | 1097 | 0.313 | 341 | 448 | 0.0 | 0.5 | 4.752 | A |
| 2 | 2080 | 520 | 287 | 3071 | 0.677 | 2072 | 1943 | 0.0 | 2.1 | 3.574 | A |
| 3 | 100 | 25 | 2191 | 725 | 0.138 | 99 | 168 | 0.0 | 0.2 | 5.743 | A |
| 4 | 1931 | 483 | 480 | 2684 | 0.719 | 1921 | 1811 | 0.0 | 2.5 | 4.659 | A |
| 5 | 11 | 3 | 2326 | 483 | 0.023 | 11 | 75 | 0.0 | 0.0 | 7.625 | A |

08:00-08:15

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 343 | 86 | 1899 | 1091 | 0.314 | 343 | 450 | 0.5 | 0.5 | 4.813 | A |
| 2 | 2080 | 520 | 289 | 3070 | 0.678 | 2080 | 1953 | 2.1 | 2.1 | 3.635 | A |
| 3 | 100 | 25 | 2200 | 721 | 0.139 | 100 | 169 | 0.2 | 0.2 | 5.795 | A |
| 4 | 1931 | 483 | 482 | 2682 | 0.720 | 1931 | 1818 | 2.5 | 2.5 | 4.789 | A |
| 5 | 11 | 3 | 2338 | 477 | 0.023 | 11 | 75 | 0.0 | 0.0 | 7.727 | A |

08:15-08:30

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity <br> (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 343 | 86 | 1899 | 1091 | 0.314 | 343 | 450 | 0.5 | 0.5 | 4.813 | A |
| 2 | 2080 | 520 | 289 | 3070 | 0.678 | 2080 | 1953 | 2.1 | 2.1 | 3.635 | A |
| 3 | 100 | 25 | 2200 | 721 | 0.139 | 100 | 169 | 0.2 | 0.2 | 5.795 | A |
| 4 | 1931 | 483 | 482 | 2682 | 0.720 | 1931 | 1818 | 2.5 | 2.6 | 4.791 | A |
| 5 | 11 | 3 | 2338 | 477 | 0.023 | 11 | 75 | 0.0 | 0.0 | 7.727 | A |

08:30-08:45

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity <br> (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 343 | 86 | 1899 | 1091 | 0.314 | 343 | 450 | 0.5 | 0.5 | 4.814 | A |
| 2 | 2080 | 520 | 289 | 3070 | 0.678 | 2080 | 1953 | 2.1 | 2.1 | 3.635 | A |
| 3 | 100 | 25 | 2200 | 721 | 0.139 | 100 | 169 | 0.2 | 0.2 | 5.796 | A |
| 4 | 1931 | 483 | 482 | 2682 | 0.720 | 1931 | 1818 | 2.6 | 2.6 | 4.791 | A |
| 5 | 11 | 3 | 2338 | 477 | 0.023 | 11 | 75 | 0.0 | 0.0 | 7.727 | A |

08:45-09:00

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 343 | 86 | 1899 | 1091 | 0.314 | 343 | 450 | 0.5 | 0.5 | 4.814 | A |
| 2 | 2080 | 520 | 289 | 3070 | 0.678 | 2080 | 1953 | 2.1 | 2.1 | 3.635 | A |
| 3 | 100 | 25 | 2200 | 721 | 0.139 | 100 | 169 | 0.2 | 0.2 | 5.796 | A |
| 4 | 1931 | 483 | 482 | 2682 | 0.720 | 1931 | 1818 | 2.6 | 2.6 | 4.791 | A |
| 5 | 11 | 3 | 2338 | 477 | 0.023 | 11 | 75 | 0.0 | 0.0 | 7.728 | A |

09:00-09:15

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 343 | 86 | 1899 | 1091 | 0.314 | 343 | 450 | 0.5 | 0.5 | 4.814 | A |
| 2 | 2080 | 520 | 289 | 3070 | 0.678 | 2080 | 1953 | 2.1 | 2.1 | 3.635 | A |
| 3 | 100 | 25 | 2200 | 721 | 0.139 | 100 | 169 | 0.2 | 0.2 | 5.796 | A |
| 4 | 1931 | 483 | 482 | 2682 | 0.720 | 1931 | 1818 | 2.6 | 2.6 | 4.791 | A |
| 5 | 11 | 3 | 2338 | 477 | 0.023 | 11 | 75 | 0.0 | 0.0 | 7.728 | A |

THE FUTURE

## 2026 Baseline, PM

Data Errors and Warnings

| Severity | Area | Item | Description |
| :--- | :--- | :--- | :--- |
| Warning | Geometry | Arm 1 - Roundabout <br> Geometry | Effective flare length is over 30m, which is outside the normal range. Treat capacities with increasing caution. |
| Warning | Geometry | Arm 2 - Roundabout <br> Geometry | Effective flare length is over 30m, which is outside the normal range. Treat capacities with increasing caution. |
| Warning | Geometry | Arm 3-Roundabout <br> Geometry | Effective flare length is over 30m, which is outside the normal range. Treat capacities with increasing caution. |

## Junction Network

## Junctions

| Junction | Name | Junction Type | Arm order | Junction Delay (s) | Junction LOS |
| :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | untitled | Standard Roundabout | $1,2,3,4,5$ | 7.28 | A |

## Junction Network Options

| Driving side | Lighting | Network residual capacity (\%) | First arm reaching threshold |
| :---: | :---: | :---: | :---: |
| Left | Normal/unknown | 8 | Arm 5 |

## Traffic Demand

Demand Set Details

| ID | Scenario <br> name | Time Period <br> name | Traffic profile <br> type | Start time <br> $(\mathbf{H H}: \mathbf{m m})$ | Finish time <br> $(\mathbf{H H}: \mathbf{m m})$ | Time period length <br> $(\mathbf{m i n})$ | Time segment length <br> (min) | Run <br> automatically |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| D4 | 2026 Baseline | PM | FLAT | $16: 45$ | $18: 15$ | 90 | 15 |  |


| Vehicle mix varies over turn | Vehicle mix varies over entry | Vehicle mix source | PCU Factor for a HV (PCU) |
| :---: | :---: | :---: | :---: |
| $\checkmark$ | $\checkmark$ | HV Percentages | 2.00 |

Demand overview (Traffic)

| Arm | Linked arm | Profile type | Use O-D data | Average Demand (Veh/hr) | Scaling Factor (\%) |
| :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ |  | FLAT | $\checkmark$ | 239 | 100.000 |
| $\mathbf{2}$ |  | FLAT | $\checkmark$ | 2787 | 100.000 |
| $\mathbf{3}$ |  | FLAT | $\checkmark$ | 200 | 100.000 |
| $\mathbf{4}$ |  | FLAT | $\checkmark$ | 1909 | 100.000 |
| $\mathbf{5}$ |  | FLAT | $\checkmark$ | 37 | 100.000 |

## Origin-Destination Data

Demand (Veh/hr)

|  | To |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| From |  | $\mathbf{1}$ | $\mathbf{2}$ | $\mathbf{3}$ | $\mathbf{4}$ | $\mathbf{5}$ |
|  | $\mathbf{1}$ | 0 | 103 | 0 | 136 | 0 |
|  | $\mathbf{2}$ | 281 | 549 | 0 | 1946 | 11 |
|  | $\mathbf{3}$ | 3 | 82 | 0 | 114 | 1 |
|  | $\mathbf{4}$ | 375 | 1486 | 40 | 0 | 8 |
|  | $\mathbf{5}$ | $\mathbf{5}$ | 0 | 0 | 32 | 0 |

THE FUTURE

Heavy Vehicle Percentages

|  | To |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| From |  | $\mathbf{1}$ | $\mathbf{2}$ | $\mathbf{3}$ | $\mathbf{4}$ | $\mathbf{5}$ |
|  | $\mathbf{1}$ | 0 | 0 | 0 | 1 | 0 |
|  | $\mathbf{2}$ | 0 | 0 | 0 | 0 | 0 |
|  | $\mathbf{3}$ | 0 | 1 | 0 | 4 | 0 |
|  | $\mathbf{4}$ | 0 | 4 | 0 | 0 | 0 |
|  | $\mathbf{5}$ | 0 | 0 | 0 | 0 | 0 |

## Results

Results Summary for whole modelled period

| Arm | Max RFC | Max delay (s) | Max Queue (Veh) | Max LOS | Average Demand <br> (Veh/hr) | Total Junction <br> Arrivals (Veh) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | 0.26 | 5.20 | 0.3 | A | 239 | 359 |
| $\mathbf{2}$ | 0.84 | 7.00 | 5.4 | A | 2787 | 4181 |
| $\mathbf{3}$ | 0.38 | 10.90 | 0.6 | B | 200 | 300 |
| $\mathbf{4}$ | 0.80 | 7.39 | 3.9 | A | 1909 | 2863 |
| $\mathbf{5}$ | 0.14 | 16.18 | 0.2 | C | 37 | 56 |

## Main Results for each time segment

16:45-17:00

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 239 | 60 | 2172 | 942 | 0.254 | 238 | 659 | 0.0 | 0.3 | 5.103 | A |
| 2 | 2787 | 697 | 206 | 3302 | 0.844 | 2766 | 2203 | 0.0 | 5.2 | 6.491 | A |
| 3 | 200 | 50 | 2933 | 544 | 0.368 | 198 | 40 | 0.0 | 0.6 | 10.344 | B |
| 4 | 1909 | 477 | 920 | 2401 | 0.795 | 1894 | 2211 | 0.0 | 3.7 | 6.909 | A |
| 5 | 37 | 9 | 2794 | 271 | 0.137 | 36 | 20 | 0.0 | 0.2 | 15.323 | C |

17:00-17:15

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 239 | 60 | 2189 | 932 | 0.257 | 239 | 664 | 0.3 | 0.3 | 5.197 | A |
| 2 | 2787 | 697 | 208 | 3301 | 0.844 | 2786 | 2220 | 5.2 | 5.3 | 6.981 | A |
| 3 | 200 | 50 | 2954 | 531 | 0.377 | 200 | 40 | 0.6 | 0.6 | 10.883 | B |
| 4 | 1909 | 477 | 927 | 2396 | 0.797 | 1909 | 2228 | 3.7 | 3.8 | 7.368 | A |
| 5 | 37 | 9 | 2815 | 260 | 0.142 | 37 | 20 | 0.2 | 0.2 | 16.160 | C |

17:15-17:30

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 239 | 60 | 2189 | 931 | 0.257 | 239 | 664 | 0.3 | 0.3 | 5.198 | A |
| 2 | 2787 | 697 | 208 | 3301 | 0.844 | 2787 | 2220 | 5.3 | 5.3 | 6.995 | A |
| 3 | 200 | 50 | 2955 | 530 | 0.377 | 200 | 40 | 0.6 | 0.6 | 10.892 | B |
| 4 | 1909 | 477 | 927 | 2396 | 0.797 | 1909 | 2228 | 3.8 | 3.9 | 7.381 | A |
| 5 | 37 | 9 | 2816 | 260 | 0.143 | 37 | 20 | 0.2 | 0.2 | 16.177 | C |

17:30-17:45

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 239 | 60 | 2189 | 931 | 0.257 | 239 | 664 | 0.3 | 0.3 | 5.199 | A |
| 2 | 2787 | 697 | 208 | 3301 | 0.844 | 2787 | 2220 | 5.3 | 5.4 | 6.998 | A |
| 3 | 200 | 50 | 2955 | 530 | 0.377 | 200 | 40 | 0.6 | 0.6 | 10.896 | B |
| 4 | 1909 | 477 | 927 | 2396 | 0.797 | 1909 | 2228 | 3.9 | 3.9 | 7.384 | A |
| 5 | 37 | 9 | 2816 | 259 | 0.143 | 37 | 20 | 0.2 | 0.2 | 16.181 | C |

17:45-18:00

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 239 | 60 | 2189 | 931 | 0.257 | 239 | 664 | 0.3 | 0.3 | 5.199 | A |
| 2 | 2787 | 697 | 208 | 3301 | 0.844 | 2787 | 2220 | 5.4 | 5.4 | 7.000 | A |
| 3 | 200 | 50 | 2955 | 530 | 0.377 | 200 | 40 | 0.6 | 0.6 | 10.896 | B |
| 4 | 1909 | 477 | 927 | 2396 | 0.797 | 1909 | 2228 | 3.9 | 3.9 | 7.387 | A |
| 5 | 37 | 9 | 2816 | 259 | 0.143 | 37 | 20 | 0.2 | 0.2 | 16.182 | C |

18:00-18:15

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 239 | 60 | 2189 | 931 | 0.257 | 239 | 664 | 0.3 | 0.3 | 5.199 | A |
| 2 | 2787 | 697 | 208 | 3301 | 0.844 | 2787 | 2220 | 5.4 | 5.4 | 7.003 | A |
| 3 | 200 | 50 | 2955 | 530 | 0.377 | 200 | 40 | 0.6 | 0.6 | 10.897 | B |
| 4 | 1909 | 477 | 927 | 2396 | 0.797 | 1909 | 2228 | 3.9 | 3.9 | 7.387 | A |
| 5 | 37 | 9 | 2816 | 259 | 0.143 | 37 | 20 | 0.2 | 0.2 | 16.183 | C |

THE FUTURE

## 2026 BTM + 60sqm, AM

Data Errors and Warnings

| Severity | Area | Item | Description |
| :--- | :--- | :--- | :--- |
| Warning | Geometry | Arm 1 - Roundabout <br> Geometry | Effective flare length is over 30m, which is outside the normal range. Treat capacities with increasing caution. |
| Warning | Geometry | Arm 2 - Roundabout <br> Geometry | Effective flare length is over 30m, which is outside the normal range. Treat capacities with increasing caution. |
| Warning | Geometry | Arm 3-Roundabout <br> Geometry | Effective flare length is over 30m, which is outside the normal range. Treat capacities with increasing caution. |

## Junction Network

## Junctions

| Junction | Name | Junction Type | Arm order | Junction Delay (s) | Junction LOS |
| :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | untitled | Standard Roundabout | $1,2,3,4,5$ | 4.54 | A |

## Junction Network Options

| Driving side | Lighting | Network residual capacity (\%) | First arm reaching threshold |
| :---: | :---: | :---: | :---: |
| Left | Normal/unknown | 25 | Arm 4 |

## Traffic Demand

Demand Set Details

| ID | Scenario name | Time Period <br> name | Traffic profile <br> type | Start time <br> $(\mathbf{H H}: \mathrm{mm})$ | Finish time <br> $($ HH:mm $)$ | Time period length <br> $(\mathbf{m i n})$ | Time segment length <br> $(\mathbf{m i n})$ | Run <br> automatically |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| D5 | 2026 BTM +60 sqm | AM | FLAT | $07: 45$ | $09: 15$ | 90 | 15 |  |


| Vehicle mix varies over turn | Vehicle mix varies over entry | Vehicle mix source | PCU Factor for a HV (PCU) |
| :---: | :---: | :---: | :---: |
| $\checkmark$ | $\checkmark$ | HV Percentages | 2.00 |

## Demand overview (Traffic)

| Arm | Linked arm | Profile type | Use O-D data | Average Demand (Veh/hr) | Scaling Factor (\%) |
| :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ |  | FLAT | $\checkmark$ | 371 | 100.000 |
| $\mathbf{2}$ |  | FLAT | $\checkmark$ | 2088 | 100.000 |
| $\mathbf{3}$ |  | FLAT | $\checkmark$ | 100 | 100.000 |
| $\mathbf{4}$ |  | FLAT | $\checkmark$ | 1993 | 100.000 |
| $\mathbf{5}$ |  | FLAT | $\checkmark$ | 11 | 100.000 |

## Origin-Destination Data

Demand (Veh/hr)

|  | To |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| From |  | $\mathbf{1}$ | $\mathbf{2}$ | $\mathbf{3}$ | $\mathbf{4}$ | $\mathbf{5}$ |  |
|  | $\mathbf{1}$ | 0 | 262 | 2 | 107 | 0 |  |
|  | $\mathbf{2}$ | 190 | 187 | 0 | 1691 | 20 |  |
|  | $\mathbf{3}$ | 0 | 87 | 0 | 13 | 0 |  |
|  | $\mathbf{4}$ | 261 | 1507 | 167 | 3 | 55 |  |
|  | $\mathbf{5}$ | $\mathbf{1}$ | 0 | 0 | 10 | 0 |  |

Heavy Vehicle Percentages

|  | To |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| From |  | $\mathbf{1}$ | $\mathbf{2}$ | $\mathbf{3}$ | $\mathbf{4}$ | $\mathbf{5}$ |
|  | $\mathbf{1}$ | 0 | 0 | 0 | 0 | 0 |
|  | $\mathbf{2}$ | 6 | 0 | 0 | 6 | 0 |
|  | $\mathbf{3}$ | 0 | 34 | 0 | 8 | 0 |
|  | $\mathbf{4}$ | 2 | 4 | 1 | 0 | 0 |
|  | $\mathbf{5}$ | 0 | 0 | 0 | 0 | 0 |

## Results

Results Summary for whole modelled period

| Arm | Max RFC | Max delay (s) | Max Queue (Veh) | Max LOS | Average Demand <br> (Veh/hr) | Total Junction <br> Arrivals (Veh) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | 0.35 | 5.28 | 0.5 | A | 371 | 557 |
| $\mathbf{2}$ | 0.68 | 3.66 | 2.1 | A | 2088 | 3132 |
| $\mathbf{3}$ | 0.14 | 5.83 | 0.2 | A | 100 | 150 |
| $\mathbf{4}$ | 0.74 | 5.24 | 2.9 | A | 1993 | 2990 |
| $\mathbf{5}$ | 0.02 | 8.33 | 0.0 | A | 11 | 17 |

## Main Results for each time segment

07:45-08:00

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 371 | 93 | 1950 | 1059 | 0.350 | 369 | 450 | 0.0 | 0.5 | 5.198 | A |
| 2 | 2088 | 522 | 287 | 3071 | 0.680 | 2080 | 2032 | 0.0 | 2.1 | 3.600 | A |
| 3 | 100 | 25 | 2199 | 722 | 0.139 | 99 | 168 | 0.0 | 0.2 | 5.779 | A |
| 4 | 1993 | 498 | 482 | 2682 | 0.743 | 1982 | 1816 | 0.0 | 2.8 | 5.063 | A |
| 5 | 11 | 3 | 2389 | 450 | 0.024 | 11 | 75 | 0.0 | 0.0 | 8.192 | A |

08:00-08:15

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 371 | 93 | 1961 | 1053 | 0.352 | 371 | 452 | 0.5 | 0.5 | 5.279 | A |
| 2 | 2088 | 522 | 289 | 3070 | 0.680 | 2088 | 2043 | 2.1 | 2.1 | 3.665 | A |
| 3 | 100 | 25 | 2208 | 717 | 0.139 | 100 | 169 | 0.2 | 0.2 | 5.833 | A |
| 4 | 1993 | 498 | 484 | 2680 | 0.744 | 1993 | 1824 | 2.8 | 2.9 | 5.236 | A |
| 5 | 11 | 3 | 2402 | 443 | 0.025 | 11 | 75 | 0.0 | 0.0 | 8.324 | A |

08:15-08:30

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 371 | 93 | 1961 | 1053 | 0.352 | 371 | 452 | 0.5 | 0.5 | 5.280 | A |
| 2 | 2088 | 522 | 289 | 3070 | 0.680 | 2088 | 2043 | 2.1 | 2.1 | 3.665 | A |
| 3 | 100 | 25 | 2208 | 717 | 0.139 | 100 | 169 | 0.2 | 0.2 | 5.833 | A |
| 4 | 1993 | 498 | 484 | 2680 | 0.744 | 1993 | 1824 | 2.9 | 2.9 | 5.237 | A |
| 5 | 11 | 3 | 2402 | 443 | 0.025 | 11 | 75 | 0.0 | 0.0 | 8.325 | A |

08:30-08:45

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity <br> (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 371 | 93 | 1961 | 1053 | 0.352 | 371 | 452 | 0.5 | 0.5 | 5.280 | A |
| 2 | 2088 | 522 | 289 | 3070 | 0.680 | 2088 | 2043 | 2.1 | 2.1 | 3.665 | A |
| 3 | 100 | 25 | 2208 | 717 | 0.139 | 100 | 169 | 0.2 | 0.2 | 5.833 | A |
| 4 | 1993 | 498 | 484 | 2680 | 0.744 | 1993 | 1824 | 2.9 | 2.9 | 5.237 | A |
| 5 | 11 | 3 | 2402 | 443 | 0.025 | 11 | 75 | 0.0 | 0.0 | 8.325 | A |

08:45-09:00

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 371 | 93 | 1961 | 1053 | 0.352 | 371 | 452 | 0.5 | 0.5 | 5.280 | A |
| 2 | 2088 | 522 | 289 | 3070 | 0.680 | 2088 | 2043 | 2.1 | 2.1 | 3.665 | A |
| 3 | 100 | 25 | 2208 | 717 | 0.139 | 100 | 169 | 0.2 | 0.2 | 5.833 | A |
| 4 | 1993 | 498 | 484 | 2680 | 0.744 | 1993 | 1824 | 2.9 | 2.9 | 5.237 | A |
| 5 | 11 | 3 | 2402 | 443 | 0.025 | 11 | 75 | 0.0 | 0.0 | 8.326 | A |

09:00-09:15

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 371 | 93 | 1961 | 1053 | 0.352 | 371 | 452 | 0.5 | 0.5 | 5.280 | A |
| 2 | 2088 | 522 | 289 | 3070 | 0.680 | 2088 | 2043 | 2.1 | 2.1 | 3.665 | A |
| 3 | 100 | 25 | 2208 | 717 | 0.139 | 100 | 169 | 0.2 | 0.2 | 5.833 | A |
| 4 | 1993 | 498 | 484 | 2680 | 0.744 | 1993 | 1824 | 2.9 | 2.9 | 5.237 | A |
| 5 | 11 | 3 | 2402 | 443 | 0.025 | 11 | 75 | 0.0 | 0.0 | 8.326 | A |

THE FUTURE

## 2026 BTM + 60sqm, PM

Data Errors and Warnings

| Severity | Area | Item | Description |
| :--- | :--- | :--- | :--- |
| Warning | Geometry | Arm 1 - Roundabout <br> Geometry | Effective flare length is over 30m, which is outside the normal range. Treat capacities with increasing caution. |
| Warning | Geometry | Arm 2 - Roundabout <br> Geometry | Effective flare length is over 30m, which is outside the normal range. Treat capacities with increasing caution. |
| Warning | Geometry | Arm 3 - Roundabout <br> Geometry | Effective flare length is over 30m, which is outside the normal range. Treat capacities with increasing caution. |

## Junction Network

## Junctions

| Junction | Name | Junction Type | Arm order | Junction Delay (s) | Junction LOS |
| :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | untitled | Standard Roundabout | $1,2,3,4,5$ | 8.37 | A |

## Junction Network Options

| Driving side | Lighting | Network residual capacity (\%) | First arm reaching threshold |
| :---: | :---: | :---: | :---: |
| Left | Normal/unknown | 7 | Arm 5 |

## Traffic Demand

Demand Set Details

| ID | Scenario name | Time Period <br> name | Traffic profile <br> type | Start time <br> $(H H: m m)$ | Finish time <br> $(H H: m m)$ | Time period length <br> $(\mathbf{m i n})$ | Time segment length <br> $(\mathbf{m i n})$ | Run <br> automatically |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| D6 | 2026 BTM +60 sqm | PM | FLAT | $16: 45$ | $18: 15$ | 90 | 15 |  |


| Vehicle mix varies over turn | Vehicle mix varies over entry | Vehicle mix source | PCU Factor for a HV (PCU) |
| :---: | :---: | :---: | :---: |
| $\checkmark$ | $\checkmark$ | HV Percentages | 2.00 |

Demand overview (Traffic)

| Arm | Linked arm | Profile type | Use O-D data | Average Demand (Veh/hr) | Scaling Factor (\%) |
| :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ |  | FLAT | $\checkmark$ | 241 | 100.000 |
| $\mathbf{2}$ |  | FLAT | $\checkmark$ | 2881 | 100.000 |
| $\mathbf{3}$ |  | FLAT | $\checkmark$ | 200 | 100.000 |
| $\mathbf{4}$ |  | FLAT | $\checkmark$ | 1914 | 100.000 |
| $\mathbf{5}$ |  | FLAT | $\checkmark$ | 37 | 100.000 |

## Origin-Destination Data

Demand (Veh/hr)

|  | To |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| From |  | $\mathbf{1}$ | $\mathbf{2}$ | $\mathbf{3}$ | $\mathbf{4}$ | $\mathbf{5}$ |
|  | $\mathbf{1}$ | 0 | 105 | 0 | 136 | 0 |
|  | $\mathbf{2}$ | 310 | 549 | 0 | 2011 | 11 |
|  | $\mathbf{3}$ | 3 | 82 | 0 | 114 | 1 |
|  | $\mathbf{4}$ | 375 | 1491 | 40 | 0 | 8 |
|  | $\mathbf{5}$ | $\mathbf{5}$ | 0 | 0 | 32 | 0 |

THE FUTURE

Heavy Vehicle Percentages

|  | To |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| From |  | $\mathbf{1}$ | $\mathbf{2}$ | $\mathbf{3}$ | $\mathbf{4}$ | $\mathbf{5}$ |
|  | $\mathbf{1}$ | 0 | 0 | 0 | 1 | 0 |
|  | $\mathbf{2}$ | 0 | 0 | 0 | 0 | 0 |
|  | $\mathbf{3}$ | 0 | 1 | 0 | 4 | 0 |
|  | $\mathbf{4}$ | 0 | 4 | 0 | 0 | 0 |
|  | $\mathbf{5}$ | 0 | 0 | 0 | 0 | 0 |

## Results

Results Summary for whole modelled period

| Arm | Max RFC | Max delay (s) | Max Queue (Veh) | Max LOS | Average Demand <br> (Veh/hr) | Total Junction <br> Arrivals (Veh) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\mathbf{1}$ | 0.26 | 5.24 | 0.4 | A | 241 | 362 |
| $\mathbf{2}$ | 0.87 | 8.56 | 6.8 | A | 2881 | 4322 |
| $\mathbf{3}$ | 0.42 | 13.14 | 0.7 | B | 200 | 300 |
| $\mathbf{4}$ | 0.81 | 7.80 | 4.1 | A | 1914 | 2871 |
| $\mathbf{5}$ | 0.15 | 17.54 | 0.2 | C | 37 | 56 |

## Main Results for each time segment

16:45-17:00

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 241 | 60 | 2175 | 940 | 0.256 | 240 | 687 | 0.0 | 0.3 | 5.133 | A |
| 2 | 2881 | 720 | 206 | 3302 | 0.872 | 2855 | 2208 | 0.0 | 6.4 | 7.670 | A |
| 3 | 200 | 50 | 3022 | 490 | 0.408 | 197 | 40 | 0.0 | 0.7 | 12.191 | B |
| 4 | 1914 | 478 | 947 | 2381 | 0.804 | 1898 | 2272 | 0.0 | 3.9 | 7.233 | A |
| 5 | 37 | 9 | 2826 | 255 | 0.145 | 36 | 20 | 0.0 | 0.2 | 16.434 | C |

17:00-17:15

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 241 | 60 | 2193 | 929 | 0.260 | 241 | 693 | 0.3 | 0.3 | 5.234 | A |
| 2 | 2881 | 720 | 208 | 3301 | 0.873 | 2880 | 2226 | 6.4 | 6.6 | 8.509 | A |
| 3 | 200 | 50 | 3048 | 474 | 0.422 | 200 | 40 | 0.7 | 0.7 | 13.099 | B |
| 4 | 1914 | 478 | 956 | 2375 | 0.806 | 1914 | 2292 | 3.9 | 4.0 | 7.777 | A |
| 5 | 37 | 9 | 2849 | 243 | 0.152 | 37 | 20 | 0.2 | 0.2 | 17.495 | C |

17:15-17:30

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 241 | 60 | 2194 | 928 | 0.260 | 241 | 693 | 0.3 | 0.3 | 5.236 | A |
| 2 | 2881 | 720 | 208 | 3301 | 0.873 | 2881 | 2227 | 6.6 | 6.7 | 8.542 | A |
| 3 | 200 | 50 | 3049 | 474 | 0.422 | 200 | 40 | 0.7 | 0.7 | 13.130 | B |
| 4 | 1914 | 478 | 956 | 2375 | 0.806 | 1914 | 2293 | 4.0 | 4.1 | 7.794 | A |
| 5 | 37 | 9 | 2850 | 242 | 0.153 | 37 | 20 | 0.2 | 0.2 | 17.526 | C |

17:30-17:45

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 241 | 60 | 2194 | 928 | 0.260 | 241 | 693 | 0.3 | 0.3 | 5.237 | A |
| 2 | 2881 | 720 | 208 | 3301 | 0.873 | 2881 | 2227 | 6.7 | 6.7 | 8.552 | A |
| 3 | 200 | 50 | 3049 | 474 | 0.422 | 200 | 40 | 0.7 | 0.7 | 13.137 | B |
| 4 | 1914 | 478 | 956 | 2375 | 0.806 | 1914 | 2293 | 4.1 | 4.1 | 7.799 | A |
| 5 | 37 | 9 | 2850 | 242 | 0.153 | 37 | 20 | 0.2 | 0.2 | 17.533 | C |

17:45-18:00

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 241 | 60 | 2194 | 928 | 0.260 | 241 | 693 | 0.3 | 0.3 | 5.237 | A |
| 2 | 2881 | 720 | 208 | 3301 | 0.873 | 2881 | 2227 | 6.7 | 6.8 | 8.559 | A |
| 3 | 200 | 50 | 3049 | 474 | 0.422 | 200 | 40 | 0.7 | 0.7 | 13.141 | B |
| 4 | 1914 | 478 | 956 | 2375 | 0.806 | 1914 | 2293 | 4.1 | 4.1 | 7.805 | A |
| 5 | 37 | 9 | 2850 | 242 | 0.153 | 37 | 20 | 0.2 | 0.2 | 17.536 | C |

18:00-18:15

| Arm | Total Demand (Veh/hr) | Junction Arrivals (Veh) | Circulating flow (Veh/hr) | Capacity (Veh/hr) | RFC | Throughput (Veh/hr) | Throughput (exit side) (Veh/hr) | Start queue (Veh) | End queue (Veh) | Delay (s) | LOS |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 241 | 60 | 2194 | 928 | 0.260 | 241 | 693 | 0.3 | 0.4 | 5.237 | A |
| 2 | 2881 | 720 | 208 | 3301 | 0.873 | 2881 | 2227 | 6.8 | 6.8 | 8.562 | A |
| 3 | 200 | 50 | 3049 | 474 | 0.422 | 200 | 40 | 0.7 | 0.7 | 13.142 | B |
| 4 | 1914 | 478 | 956 | 2375 | 0.806 | 1914 | 2293 | 4.1 | 4.1 | 7.803 | A |
| 5 | 37 | 9 | 2850 | 242 | 0.153 | 37 | 20 | 0.2 | 0.2 | 17.537 | C |

## motion

A41 / A4421 - Rodney House Roundabout

Basic Results Summary
Basic Results Summary

## User and Project Details

| Project: | Bicester Office Park |
| :--- | :--- |
| Title: | Rodney House Roundabout <br> Location: |
| Bicester |  |

Scenario 1: '20026 AM' (FG1: '2026 AM', Plan 1: 'Network Control Plan 1')
Network Layout Diagram


Basic Results Summary
Network Results

| Item | Lane <br> Description | Lane Type | Full Phase | Arrow Phase | Num Greens | Total Green (s) | Arrow <br> Green <br> (s) | Demand <br> Flow <br> (pcu) | Sat Flow (pcu/Hr) | Capacity (pcu) | $\begin{array}{\|l\|l} \hline \text { Deg } \\ \text { Sat } \\ (\%) \end{array}$ | Turners In Gaps (pcu) | Turners When Unopposed (pcu) | Turners In Intergreen (pcu) | Total Delay (pcuHr) | Av. Delay Per PCU (s/pcu) | Mean Max Queue (pcu) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network | - | - | - |  | - | - | - | - | - | - | 68.2\% | 0 | 0 | 0 | 38.9 | - | - |
| Unnamed Junction | - | - | - |  | - | - | - | - | - | - | 68.2\% | 0 | 0 | 0 | 38.9 | - | - |
| 1/2+1/1 | A41 Left Ahead | U | J |  | 1 | 51 | - | 967 | 2080:1940 | 693+724 | $\begin{aligned} & 68.2 \text { : } \\ & 68.2 \% \end{aligned}$ | - | - | - | 3.9 | 14.6 | 7.9 |
| 1/3 | A41 Ahead | U | J |  | 1 | 51 | - | 96 | 2080 | 1202 | 8.0\% | - | - | - | 0.3 | 10.1 | 1.1 |
| 2/1 | Gravenhill Rd Left | U | M |  | 1 | 18 | - | 223 | 1894 | 400 | 55.8\% | - | - | - | 2.6 | 41.9 | 5.6 |
| 2/2+2/3 | Gravenhill Rd Ahead | U | M |  | 1 | 18 | - | 282 | 2044:2044 | $251+375$ | $\begin{aligned} & 45.0: \\ & 45.0 \% \end{aligned}$ | - | - | - | 2.8 | 35.4 | 4.0 |
| 3/2+3/1 | A41 U-Turn Ahead | U | A |  | 1 | 61 | - | 1006 | 2029:1848 | 965+820 | $\begin{aligned} & \text { 56.4: } \\ & 56.4 \% \end{aligned}$ | - | - | - | 2.3 | 8.2 | 6.4 |
| 3/3 | A41 Ahead | U | A |  | 1 | 61 | - | 552 | 2029 | 1398 | 39.5\% | - | - | - | 1.2 | 8.1 | 6.2 |
| 4/2+4/1 | B4100 Left Ahead | U | D |  | 1 | 9 | - | 142 | 2005:1870 | 223+81 | $\begin{aligned} & 46.7: \\ & 46.7 \% \end{aligned}$ | - | - | - | 1.9 | 48.3 | 2.9 |
| 4/3 | B4100 <br> Ahead | U | D |  | 1 | 9 | - | 114 | 2005 | 223 | 51.2\% | - | - | - | 1.7 | 54.1 | 3.2 |
| 5/2+5/1 | A4421 Left Ahead | U | G |  | 1 | 22 | - | 403 | 2005:1848 | 495+144 | $\begin{aligned} & 63.0: \\ & 63.0 \% \end{aligned}$ | - | - | - | 4.1 | 36.4 | 7.7 |
| 5/3 | A4421 <br> Ahead | U | G |  | 1 | 22 | - | 227 | 2005 | 512 | 44.3\% | - | - | - | 2.2 | 34.4 | 5.1 |
| 11/1 | Ahead | U | N |  | 1 | 60 | - | 664 | 1900 | 1288 | 51.6\% | - | - | - | 1.2 | 6.4 | 4.3 |
| 11/2 | Ahead Right | U | N |  | 1 | 60 | - | 708 | 1900 | 1288 | 55.0\% | - | - | - | 1.4 | 7.3 | 5.6 |
| 11/3 | Right | U | N |  | 1 | 60 | - | 105 | 1900 | 1288 | 8.2\% | - | - | - | 0.2 | 6.0 | 0.8 |
| 12/1 | Ahead | U | B |  | 1 | 17 | - | 28 | 1900 | 380 | 7.4\% | - | - | - | 0.1 | 16.9 | 0.7 |
| 12/2 | Ahead Right | U | B |  | 1 | 17 | - | 186 | 1900 | 380 | 48.9\% | - | - | - | 1.5 | 29.6 | 4.3 |
| 12/3 | Right | U | B |  | 1 | 17 | - | 178 | 1900 | 380 | 46.8\% | - | - | - | 1.1 | 21.7 | 4.8 |
| 13/1 | Ahead | U | E |  | 1 | 69 | - | 419 | 1900 | 1478 | 28.4\% | - | - | - | 0.4 | 3.2 | 3.7 |
| 13/2 | Ahead Right | U | E |  | 1 | 69 | - | 651 | 1900 | 1478 | 44.1\% | - | - | - | 0.8 | 4.6 | 7.5 |
| 13/3 | Right | U | E |  | 1 | 69 | - | 623 | 1900 | 1478 | 42.2\% | - | - | - | 0.8 | 4.5 | 7.5 |

Basic Results Summary


Basic Results Summary
Scenario 2: '2026 PM' (FG2: '2026 PM', Plan 1: 'Network Control Plan 1')
Network Layout Diagram


Basic Results Summary
Network Results

| Item | Lane <br> Description | Lane Type | Full Phase | Arrow Phase | Num Greens | Total Green (s) | Arrow <br> Green <br> (s) | Demand <br> Flow <br> (pcu) | Sat Flow (pcu/Hr) | Capacity (pcu) | Deg <br> Sat <br> (\%) | Turners In Gaps (pcu) | Turners When Unopposed (pcu) | Turners In Intergreen (pcu) | Total Delay (pcuHr) | Av. Delay Per PCU (s/pcu) | Mean Max Queue (pcu) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network | - | - | - |  | - | - | - | - | - | - | 82.1\% | 0 | 0 | 0 | 47.5 | - | - |
| Unnamed Junction | - | - | - |  | - | - | - | - | - | - | 82.1\% | 0 | 0 | 0 | 47.5 | - | - |
| 1/2+1/1 | A41 Left Ahead | U | J |  | 1 | 54 | - | 1194 | 2080:1940 | 673+782 | $\begin{aligned} & \text { 82.1: } \\ & \text { 82.1\% } \end{aligned}$ | - | - | - | 5.7 | 17.1 | 16.1 |
| 1/3 | A41 Ahead | U | J |  | 1 | 54 | - | 262 | 2080 | 1271 | 20.6\% | - | - | - | 0.7 | 9.6 | 3.0 |
| 2/1 | Gravenhill Rd Left | U | M |  | 1 | 12 | - | 179 | 1894 | 274 | 65.4\% | - | - | - | 2.7 | 55.0 | 5.2 |
| 2/2+2/3 | Gravenhill Rd Ahead | U | M |  | 1 | 12 | - | 181 | 2044:2044 | 165+295 | $\begin{aligned} & 39.4: \\ & 39.4 \% \end{aligned}$ | - | - | - | 2.1 | 41.1 | 2.9 |
| 3/2+3/1 | A41 U-Turn Ahead | U | A |  | 1 | 60 | - | 1096 | 2029:1852 | 638+976 | $\begin{aligned} & \hline 67.9: \\ & 67.9 \% \end{aligned}$ | - | - | - | 3.1 | 10.2 | 9.3 |
| 3/3 | A41 Ahead | U | A |  | 1 | 60 | - | 660 | 2029 | 1375 | 48.0\% | - | - | - | 1.7 | 9.4 | 8.2 |
| 4/2+4/1 | B4100 Left Ahead | U | D |  | 1 | 18 | - | 249 | 2005:1870 | 423+304 | $\begin{aligned} & 34.3: \\ & 34.3 \% \end{aligned}$ | - | - | - | 2.3 | 33.7 | 3.3 |
| 4/3 | B4100 <br> Ahead | U | D |  | 1 | 18 | - | 188 | 2005 | 423 | 44.4\% | - | - | - | 2.0 | 38.5 | 4.5 |
| 5/2+5/1 | A4421 Left Ahead | U | G |  | 1 | 21 | - | 329 | 2005:1848 | 488+56 | $\begin{aligned} & 60.5: \\ & 60.5 \% \end{aligned}$ | - | - | - | 3.5 | 38.0 | 7.2 |
| 5/3 | A4421 <br> Ahead | U | G |  | 1 | 21 | - | 234 | 2005 | 490 | 47.7\% | - | - | - | 2.3 | 36.1 | 5.4 |
| 11/1 | Ahead | U | N |  | 1 | 66 | - | 753 | 1900 | 1414 | 53.2\% | - | - | - | 1.1 | 5.2 | 4.3 |
| 11/2 | Ahead Right | U | N |  | 1 | 66 | - | 833 | 1900 | 1414 | 58.9\% | - | - | - | 1.4 | 5.9 | 5.2 |
| 11/3 | Right | U | N |  | 1 | 66 | - | 262 | 1900 | 1414 | 18.5\% | - | - | - | 0.4 | 5.0 | 2.0 |
| 12/1 | Ahead | U | B |  | 1 | 18 | - | 26 | 1900 | 401 | 6.5\% | - | - | - | 0.2 | 22.0 | 0.5 |
| 12/2 | Ahead Right | U | B |  | 1 | 18 | - | 224 | 1900 | 401 | 55.8\% | - | - | - | 1.9 | 29.8 | 4.9 |
| 12/3 | Right | U | B |  | 1 | 18 | - | 200 | 1900 | 401 | 49.9\% | - | - | - | 1.1 | 20.6 | 4.7 |
| 13/1 | Ahead | U | E |  | 1 | 60 | - | 600 | 1900 | 1288 | 46.6\% | - | - | - | 1.2 | 7.1 | 6.4 |
| 13/2 | Ahead Right | U | E |  | 1 | 60 | - | 591 | 1900 | 1288 | 45.9\% | - | - | - | 1.7 | 10.3 | 10.0 |
| 13/3 | Right | U | E |  | 1 | 60 | - | 702 | 1900 | 1288 | 54.5\% | - | - | - | 1.1 | 5.4 | 6.6 |

Basic Results Summary


Basic Results Summary
Scenario 3: '2026 Baseline AM' (FG3: '2026 Baseline AM', Plan 1: 'Network Control Plan 1')
Network Layout Diagram


Basic Results Summary
Network Results

| Item | Lane <br> Description | Lane Type | Full Phase | Arrow Phase | Num Greens | Total Green (s) | Arrow <br> Green <br> (s) | Demand <br> Flow <br> (pcu) | Sat Flow (pcu/Hr) | Capacity (pcu) | $\begin{array}{\|l\|l} \hline \text { Deg } \\ \text { Sat } \\ (\%) \end{array}$ | Turners In Gaps (pcu) | Turners When Unopposed (pcu) | Turners In Intergreen (pcu) | Total Delay (pcuHr) | Av. Delay Per PCU (s/pcu) | Mean Max Queue (pcu) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network | - | - | - |  | - | - | - | - | - | - | 73.3\% | 0 | 0 | 0 | 43.0 | - | - |
| Unnamed Junction | - | - | - |  | - | - | - | - | - | - | 73.3\% | 0 | 0 | 0 | 43.0 | - | - |
| 1/2+1/1 | A41 Left Ahead | U | J |  | 1 | 53 | - | 1045 | 2080:1940 | 650+776 | $\begin{aligned} & 73.3: \\ & 73.3 \% \end{aligned}$ | - | - | - | 4.2 | 14.6 | 11.6 |
| 1/3 | A41 Ahead | U | J |  | 1 | 53 | - | 101 | 2080 | 1248 | 8.1\% | - | - | - | 0.3 | 9.2 | 1.1 |
| 2/1 | Gravenhill Rd Left | U | M |  | 1 | 17 | - | 223 | 1894 | 379 | 58.9\% | - | - | - | 2.7 | 44.1 | 5.7 |
| 2/2+2/3 | Gravenhill Rd Ahead | U | M |  | 1 | 17 | - | 282 | 2044:2044 | $351+327$ | $\begin{aligned} & 41.6: \\ & 41.6 \% \end{aligned}$ | - | - | - | 2.8 | 35.5 | 3.5 |
| 3/2+3/1 | A41 U-Turn Ahead | U | A |  | 1 | 60 | - | 972 | 2029:1848 | 931+872 | $\begin{aligned} & 53.9: \\ & 53.9 \% \end{aligned}$ | - | - | - | 2.3 | 8.4 | 5.9 |
| 3/3 | A41 Ahead | U | A |  | 1 | 60 | - | 602 | 2029 | 1375 | 43.8\% | - | - | - | 1.5 | 9.0 | 7.2 |
| 4/2+4/1 | B4100 Left Ahead | U | D |  | 1 | 13 | - | 142 | 2005:1870 | 312+114 | $\begin{aligned} & 33.3: \\ & 33.3 \% \end{aligned}$ | - | - | - | 1.6 | 39.9 | 2.6 |
| 4/3 | B4100 <br> Ahead | U | D |  | 1 | 13 | - | 156 | 2005 | 312 | 50.0\% | - | - | - | 2.0 | 46.3 | 4.1 |
| 5/2+5/1 | A4421 Left Ahead | U | G |  | 1 | 26 | - | 452 | 2005:1848 | 567+143 | $\begin{aligned} & 63.6: \\ & 63.6 \% \end{aligned}$ | - | - | - | 4.2 | 33.1 | 8.5 |
| 5/3 | A4421 <br> Ahead | U | G |  | 1 | 26 | - | 220 | 2005 | 601 | 36.6\% | - | - | - | 1.8 | 29.5 | 4.6 |
| 11/1 | Ahead | U | N |  | 1 | 61 | - | 770 | 1900 | 1309 | 58.8\% | - | - | - | 1.5 | 6.9 | 5.0 |
| 11/2 | Ahead Right | U | N |  | 1 | 61 | - | 764 | 1900 | 1309 | 58.4\% | - | - | - | 1.5 | 6.9 | 5.1 |
| 11/3 | Right | U | N |  | 1 | 61 | - | 110 | 1900 | 1309 | 8.4\% | - | - | - | 0.2 | 5.7 | 0.9 |
| 12/1 | Ahead | U | B |  | 1 | 18 | - | 23 | 1900 | 401 | 5.7\% | - | - | - | 0.0 | 7.8 | 0.5 |
| 12/2 | Ahead Right | U | B |  | 1 | 18 | - | 210 | 1900 | 401 | 52.4\% | - | - | - | 1.4 | 24.4 | 5.0 |
| 12/3 | Right | U | B |  | 1 | 18 | - | 159 | 1900 | 401 | 39.6\% | - | - | - | 0.7 | 15.5 | 3.9 |
| 13/1 | Ahead | U | E |  | 1 | 65 | - | 442 | 1900 | 1393 | 31.7\% | - | - | - | 0.8 | 6.6 | 3.6 |
| 13/2 | Ahead Right | U | E |  | 1 | 65 | - | 631 | 1900 | 1393 | 45.3\% | - | - | - | 1.1 | 6.3 | 4.2 |
| 13/3 | Right | U | E |  | 1 | 65 | - | 632 | 1900 | 1393 | 45.4\% | - | - | - | 0.6 | 3.6 | 5.3 |

Basic Results Summary

| 14/1 | Ahead | U | H |  |  | 1 | 52 |  | 611 | 1900 | 1119 | 54.6\% | - | - |  | 1.7 | 9.8 | 5.9 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 14/2 | Ahead Right | U | H |  |  | 1 | 52 |  | 684 | 1900 | 1119 | 61.1\% | - | - |  | 2.0 | 10.8 | 6.1 |
| 14/3 | Right | U | H |  |  | 1 | 52 |  | 156 | 1900 | 1119 | 13.9\% | - | - | - | 0.6 | 14.7 | 4.0 |
| 15/1 | Ahead | U | K |  |  | 1 | 25 |  | 393 | 1900 | 549 | 71.6\% | - | - | - | 3.9 | 35.9 | 8.8 |
| 15/2 | Right | U | K |  |  | 1 | 25 |  | 298 | 1900 | 549 | 54.3\% | - | - | - | 1.7 | 20.6 | 6.6 |
| 15/3 | Right | U | K |  |  | 1 | 25 |  | 297 | 1900 | 549 | 54.1\% | - | - | - | 1.8 | 22.3 | 6.6 |
| C1 Stream: 1 PRC for Signalled Lanes (\%): <br> C1 Stream: 2 PRC for Signalled Lanes (\%): <br> C1 Stream: 3 PRC for Signalled Lanes (\%): <br> C1 Stream: 4 PRC for Signalled Lanes (\%): <br> C1 Stream: 5 PRC for Signalled Lanes (\%): <br>  PRC Over All Lanes (\%): |  |  |  |  |  |  |  |  | 67.0 Total Delay for Signalled Lanes (pcuHr): <br> 79.9 Total Delay for Signalled Lanes (pcuHr): <br> 41.5 Total Delay for Signalled Lanes (pcuHr): <br> 22.8 Total Delay for Signalled Lanes (pcuHr): <br> 52.9 Total Delay for Signalled Lanes (pcuHr): <br> 22.8 Total Delay Over All Lanes(pcuHr): |  |  |  | $\begin{array}{r} 5.93 \\ 6.14 \\ 10.29 \\ 11.98 \\ 8.62 \\ 42.96 \end{array}$ | Cycle Time (s): $\quad 90$ <br> Cycle Time (s): 90 <br> Cycle Time (s): $\quad 90$ <br> Cycle Time (s): 90 <br> Cycle Time (s): 90 |  |  |  |  |

Basic Results Summary
Scenario 4: '2026 Baseline PM' (FG4: '2026 Baseline PM', Plan 1: 'Network Control Plan 1')
Network Layout Diagram


Basic Results Summary
Network Results

| Item | Lane <br> Description | Lane Type | Full Phase | Arrow Phase | Num Greens | Total Green (s) | Arrow <br> Green <br> (s) | Demand <br> Flow <br> (pcu) | Sat Flow (pcu/Hr) | Capacity (pcu) | Deg <br> Sat <br> (\%) | Turners In Gaps (pcu) | Turners When Unopposed (pcu) | Turners In Intergreen (pcu) | Total Delay (pcuHr) | Av. Delay Per PCU (s/pcu) | Mean Max Queue (pcu) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network | - | - | - |  | - | - | - | - | - | - | 83.5\% | 0 | 0 | 0 | 50.9 | - | - |
| Unnamed Junction | - | - | - |  | - | - | - | - | - | - | 83.5\% | 0 | 0 | 0 | 50.9 | - | - |
| 1/2+1/1 | A41 Left Ahead | U | J |  | 1 | 55 | - | 1200 | 2080:1940 | 619+826 | $\begin{aligned} & 83.1: \\ & 83.1 \% \end{aligned}$ | - | - | - | 5.8 | 17.4 | 17.8 |
| 1/3 | A41 Ahead | U | J |  | 1 | 55 | - | 262 | 2080 | 1294 | 20.2\% | - | - | - | 0.7 | 9.1 | 2.9 |
| 2/1 | Gravenhill Rd Left | U | M |  | 1 | 14 | - | 179 | 1894 | 316 | 56.7\% | - | - | - | 2.4 | 47.6 | 4.7 |
| 2/2+2/3 | Gravenhill Rd Ahead | U | M |  | 1 | 14 | - | 181 | 2044:2044 | 198+322 | $\begin{aligned} & 34.8: \\ & 34.8 \% \end{aligned}$ | - | - | - | 1.9 | 38.1 | 2.7 |
| 3/2+3/1 | A41 U-Turn Ahead | U | A |  | 1 | 62 | - | 1280 | 2029:1851 | 685+991 | $\begin{aligned} & \hline 76.4: \\ & 76.4 \% \end{aligned}$ | - | - | - | 3.8 | 10.8 | 11.1 |
| 3/3 | A41 Ahead | U | A |  | 1 | 62 | - | 649 | 2029 | 1420 | 45.7\% | - | - | - | 1.5 | 8.3 | 7.5 |
| 4/2+4/1 | B4100 Left Ahead | U | D |  | 1 | 13 | - | 249 | 2005:1870 | 312+226 | $\begin{aligned} & 46.5: \\ & 46.0 \% \end{aligned}$ | - | - | - | 2.8 | 40.6 | 3.7 |
| 4/3 | B4100 <br> Ahead | U | D |  | 1 | 13 | - | 191 | 2005 | 312 | 61.2\% | - | - | - | 2.7 | 50.2 | 5.2 |
| 5/2+5/1 | A4421 Left Ahead | U | G |  | 1 | 23 | - | 375 | 2005:1848 | 529+53 | $\begin{aligned} & 64.5: \\ & 64.5 \% \end{aligned}$ | - | - | - | 3.9 | 37.4 | 8.4 |
| 5/3 | A4421 <br> Ahead | U | G |  | 1 | 23 | - | 191 | 2005 | 535 | 35.7\% | - | - | - | 1.7 | 32.0 | 4.1 |
| 11/1 | Ahead | U | N |  | 1 | 64 | - | 796 | 1900 | 1372 | 58.0\% | - | - | - | 1.4 | 6.5 | 5.2 |
| 11/2 | Ahead Right | U | N |  | 1 | 64 | - | 795 | 1900 | 1372 | 57.9\% | - | - | - | 1.4 | 6.4 | 5.1 |
| 11/3 | Right | U | N |  | 1 | 64 | - | 269 | 1900 | 1372 | 19.6\% | - | - | - | 0.4 | 5.6 | 2.2 |
| 12/1 | Ahead | U | B |  | 1 | 16 | - | 19 | 1900 | 359 | 5.3\% | - | - | - | 0.1 | 10.4 | 0.4 |
| 12/2 | Ahead Right | U | B |  | 1 | 16 | - | 240 | 1900 | 359 | 66.9\% | - | - | - | 2.6 | 38.4 | 5.5 |
| 12/3 | Right | U | B |  | 1 | 16 | - | 191 | 1900 | 359 | 53.2\% | - | - | - | 1.4 | 25.6 | 4.7 |
| 13/1 | Ahead | U | E |  | 1 | 65 | - | 660 | 1900 | 1393 | 47.4\% | - | - | - | 1.0 | 5.6 | 4.3 |
| 13/2 | Ahead Right | U | E |  | 1 | 65 | - | 695 | 1900 | 1393 | 49.9\% | - | - | - | 1.6 | 8.4 | 11.2 |
| 13/3 | Right | U | E |  | 1 | 65 | - | 668 | 1900 | 1393 | 47.9\% | - | - | - | 0.7 | 3.7 | 5.8 |

Basic Results Summary


Basic Results Summary
Scenario 5: '2026 AM + 60sqm' (FG5: '2026 AM + 60sqm ', Plan 1: 'Network Control Plan 1')
Network Layout Diagram


Basic Results Summary
Network Results

| Item | Lane <br> Description | Lane Type | Full Phase | Arrow Phase | Num Greens | Total Green (s) | Arrow <br> Green <br> (s) | Demand <br> Flow <br> (pcu) | Sat Flow (pcu/Hr) | Capacity (pcu) | Deg <br> Sat <br> (\%) | Turners In Gaps (pcu) | Turners When Unopposed (pcu) | Turners In Intergreen (pcu) | Total Delay (pcuHr) | Av. Delay Per PCU (s/pcu) | Mean Max Queue (pcu) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network | - | - | - |  | - | - | - | - | - | - | 74.2\% | 0 | 0 | 0 | 45.8 | - | - |
| Unnamed Junction | - | - | - |  | - | - | - | - | - | - | 74.2\% | 0 | 0 | 0 | 45.8 | - | - |
| 1/2+1/1 | A41 Left Ahead | U | J |  | 1 | 53 | - | 1073 | 2080:1940 | 688+759 | $\begin{aligned} & 74.2: \\ & 74.2 \% \end{aligned}$ | - | - | - | 4.4 | 14.8 | 11.1 |
| 1/3 | A41 Ahead | U | J |  | 1 | 53 | - | 101 | 2080 | 1248 | 8.1\% | - | - | - | 0.3 | 9.2 | 1.1 |
| 2/1 | Gravenhill Rd Left | U | M |  | 1 | 16 | - | 223 | 1894 | 358 | 62.3\% | - | - | - | 2.9 | 46.8 | 5.9 |
| 2/2+2/3 | Gravenhill Rd Ahead | U | M |  | 1 | 16 | - | 282 | 2044:2044 | 249+346 | $\begin{aligned} & 47.4: \\ & 47.4 \% \end{aligned}$ | - | - | - | 2.9 | 37.6 | 4.0 |
| 3/2+3/1 | A41 U-Turn Ahead | U | A |  | 1 | 65 | - | 997 | 2029:1848 | 997+897 | $\begin{aligned} & 52.6: \\ & 52.6 \% \end{aligned}$ | - | - | - | 1.7 | 6.3 | 5.2 |
| 3/3 | A41 Ahead | U | A |  | 1 | 65 | - | 582 | 2029 | 1488 | 39.1\% | - | - | - | 1.0 | 6.5 | 5.7 |
| 4/2+4/1 | B4100 Left Ahead | U | D |  | 1 | 15 | - | 142 | 2005:1870 | 356+130 | $\begin{aligned} & 29.2: \\ & 29.2 \% \end{aligned}$ | - | - | - | 1.5 | 37.0 | 2.5 |
| 4/3 | B4100 <br> Ahead | U | D |  | 1 | 15 | - | 169 | 2005 | 356 | 47.4\% | - | - | - | 2.0 | 42.8 | 4.2 |
| 5/2+5/1 | A4421 Left Ahead | U | G |  | 1 | 25 | - | 413 | 2005:1848 | 547+154 | $\begin{aligned} & 58.9: \\ & 58.9 \% \end{aligned}$ | - | - | - | 3.7 | 32.6 | 7.5 |
| 5/3 | A4421 <br> Ahead | U | G |  | 1 | 25 | - | 272 | 2005 | 579 | 47.0\% | - | - | - | 2.4 | 32.2 | 6.0 |
| 11/1 | Ahead | U | N |  | 1 | 62 | - | 802 | 1900 | 1330 | 60.3\% | - | - | - | 1.8 | 8.2 | 9.5 |
| 11/2 | Ahead Right | U | N |  | 1 | 62 | - | 795 | 1900 | 1330 | 59.8\% | - | - | - | 1.9 | 8.5 | 10.0 |
| 11/3 | Right | U | N |  | 1 | 62 | - | 101 | 1900 | 1330 | 7.6\% | - | - | - | 0.2 | 7.4 | 1.4 |
| 12/1 | Ahead | U | B |  | 1 | 13 | - | 32 | 1900 | 296 | 10.8\% | - | - | - | 0.2 | 22.8 | 0.8 |
| 12/2 | Ahead Right | U | B |  | 1 | 13 | - | 183 | 1900 | 296 | 61.9\% | - | - | - | 1.7 | 33.8 | 4.0 |
| 12/3 | Right | U | B |  | 1 | 13 | - | 177 | 1900 | 296 | 59.9\% | - | - | - | 1.4 | 28.3 | 5.0 |
| 13/1 | Ahead | U | E |  | 1 | 63 | - | 425 | 1900 | 1351 | 31.5\% | - | - | - | 0.6 | 5.4 | 3.0 |
| 13/2 | Ahead Right | U | E |  | 1 | 63 | - | 678 | 1900 | 1351 | 50.2\% | - | - | - | 1.3 | 6.8 | 5.8 |
| 13/3 | Right | U | E |  | 1 | 63 | - | 606 | 1900 | 1351 | 44.9\% | - | - | - | 1.3 | 7.9 | 6.5 |

Basic Results Summary


Basic Results Summary
Scenario 6: '2026 PM + 60sqm' (FG6: '2026 PM + 60sqm', Plan 1: 'Network Control Plan 1')
Network Layout Diagram


Basic Results Summary
Network Results

| Item | Lane <br> Description | Lane Type | Full Phase | Arrow Phase | Num Greens | Total Green (s) | Arrow <br> Green <br> (s) | Demand <br> Flow <br> (pcu) | Sat Flow (pcu/Hr) | Capacity (pcu) | Deg <br> Sat <br> (\%) | Turners In Gaps (pcu) | Turners When Unopposed (pcu) | Turners In Intergreen (pcu) | Total Delay (pcuHr) | Av. Delay Per PCU (s/pcu) | Mean Max Queue (pcu) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Network | - | - | - |  | - | - | - | - | - | - | 83.5\% | 0 | 0 | 0 | 50.8 | - | - |
| Unnamed Junction | - | - | - |  | - | - | - | - | - | - | 83.5\% | 0 | 0 | 0 | 50.8 | - | - |
| 1/2+1/1 | A41 Left Ahead | U | J |  | 1 | 55 | - | 1233 | 2080:1940 | 694+789 | $\begin{aligned} & 83.1: \\ & 83.1 \% \end{aligned}$ | - | - | - | 5.8 | 16.9 | 16.8 |
| 1/3 | A41 Ahead | U | J |  | 1 | 55 | - | 231 | 2080 | 1294 | 17.8\% | - | - | - | 0.6 | 8.9 | 2.5 |
| 2/1 | Gravenhill Rd Left | U | M |  | 1 | 13 | - | 179 | 1894 | 295 | 60.8\% | - | - | - | 2.5 | 50.8 | 4.9 |
| 2/2+2/3 | Gravenhill Rd Ahead | U | M |  | 1 | 13 | - | 181 | 2044:2044 | 287+303 | $\begin{aligned} & 30.7: \\ & 30.7 \% \end{aligned}$ | - | - | - | 1.9 | 38.0 | 2.3 |
| 3/2+3/1 | A41 U-Turn Ahead | U | A |  | 1 | 62 | - | 1264 | 2029:1848 | 758+965 | $\begin{aligned} & \hline 73.3: \\ & 73.3 \% \end{aligned}$ | - | - | - | 3.5 | 10.0 | 9.8 |
| 3/3 | A41 Ahead | U | A |  | 1 | 62 | - | 724 | 2029 | 1420 | 51.0\% | - | - | - | 1.8 | 8.9 | 8.8 |
| 4/2+4/1 | B4100 Left Ahead | U | D |  | 1 | 13 | - | 249 | 2005:1870 | $312+224$ | $\begin{aligned} & 46.5: \\ & 46.5 \% \end{aligned}$ | - | - | - | 2.8 | 40.6 | 3.7 |
| 4/3 | B4100 <br> Ahead | U | D |  | 1 | 13 | - | 192 | 2005 | 312 | 61.6\% | - | - | - | 2.7 | 50.3 | 5.3 |
| 5/2+5/1 | A4421 Left Ahead | U | G |  | 1 | 20 | - | 316 | 2005:1848 | 468+56 | $\begin{aligned} & 60.3: \\ & 60.3 \% \end{aligned}$ | - | - | - | 3.4 | 39.0 | 7.0 |
| 5/3 | A4421 <br> Ahead | U | G |  | 1 | 20 | - | 251 | 2005 | 468 | 53.7\% | - | - | - | 2.7 | 38.5 | 6.0 |
| 11/1 | Ahead | U | N |  | 1 | 65 | - | 789 | 1900 | 1393 | 56.6\% | - | - | - | 1.3 | 5.9 | 7.0 |
| 11/2 | Ahead Right | U | N |  | 1 | 65 | - | 844 | 1900 | 1393 | 60.6\% | - | - | - | 1.7 | 7.4 | 10.7 |
| 11/3 | Right | U | N |  | 1 | 65 | - | 231 | 1900 | 1393 | 16.6\% | - | - | - | 0.3 | 5.1 | 1.5 |
| 12/1 | Ahead | U | B |  | 1 | 16 | - | 57 | 1900 | 359 | 15.9\% | - | - | - | 0.5 | 31.1 | 1.3 |
| 12/2 | Ahead Right | U | B |  | 1 | 16 | - | 207 | 1900 | 359 | 57.7\% | - | - | - | 2.1 | 36.7 | 5.0 |
| 12/3 | Right | U | B |  | 1 | 16 | - | 186 | 1900 | 359 | 51.8\% | - | - | - | 1.6 | 31.1 | 4.6 |
| 13/1 | Ahead | U | E |  | 1 | 65 | - | 601 | 1900 | 1393 | 43.1\% | - | - | - | 1.0 | 6.1 | 4.3 |
| 13/2 | Ahead Right | U | E |  | 1 | 65 | - | 700 | 1900 | 1393 | 50.2\% | - | - | - | 1.3 | 6.8 | 5.3 |
| 13/3 | Right | U | E |  | 1 | 65 | - | 766 | 1900 | 1393 | 55.0\% | - | - | - | 1.1 | 5.0 | 7.1 |

Basic Results Summary



[^0]:    Values shown are the highest values encountered over all time segments. Delay is the maximum value of average delay per arriving vehicle. Junction LOS and Junction Delay are demand-weighted averages. Network Residual Capacity indicates the amount by which network flow could be increased before a user-definable threshold (see Analysis Options) is met.

[^1]:    Values shown are the highest values encountered over all time segments. Delay is the maximum value of average delay per arriving vehicle. Junction LOS and Junction Delay are demand-weighted averages. Network Residual Capacity indicates the amount by which network flow could be increased before a user-definable threshold (see Analysis Options) is met.

