

Title: Howes Lane (A4095) / Bucknell Road / A4095

Junction

Technical Note 10 v10

Date: March 2023

1.0 Introduction

1.1.1 Jubb has been commissioned by Hallam Land Management Ltd (HLM) to provide highways and transportation advice in relation to a proposal for a residential-led mixed use development on land northeast of the railway line in North West Bicester 'Hawkwell Village'.

- 1.1.2 A planning application (Ref: 21/04275/OUT) was submitted in December 2021 for a residential led mixed use development for up to 3,100 dwellings.
- 1.1.3 The Site forms part of the allocated North West Bicester Eco-Town development. Historical traffic modelling of the allocation has shown a need for the implementation of the A4095 Strategic Link Road (SLR) to bypass the Howes Lane (A4095) / Bucknell Road / A4095 double junction.
- 1.1.4 The Howes Lane (A4095) / Bucknell Road / A4095 consists of a roundabout junction to the north of the railway line and a priority junction to the south of the railway line and currently operates over capacity and offers no meaningful active travel crossing facilities.
- 1.1.5 Oxfordshire County Council (OCC) received funding from Homes England and the Oxfordshire Housing and Growth Deal to implement infrastructure in order to enable proposed development to come forward. The funding allocated to the SLR has enabled the completion of the rail underbridge. Unfortunately, due to time limits on spending of the allocated fund, the remaining monies have been reallocated to other strategic highway schemes in Oxfordshire. Hallam are currently in discussion with OCC and Cherwell District Council (CDC) to investigate other potential funding mechanisms for the SLR.
- 1.1.6 The SLR is a large strategic piece of highway infrastructure and will cost a considerable sum of money. Hawkwell Village will need to deliver and release a significant quantum of dwellings in order to be able to financially contribute its share of the s106 requirements to fund the road.
- 1.1.7 Therefore, this Technical Note (TN) undertakes a review of the operation of the existing Howes Lane (A4095) / Bucknell Road / A4095 junctions and promotes an interim signalisation scheme to mitigate the effects of the early phases of the development, before the SLR is delivered. The scheme offers improved active travel crossing facilities and advance cycle stop lines with associated feeder lanes. When the SLR has been delivered, traffic levels travelling through the Howes Lane (A4095) / Bucknell Road / A4095 junctions decreases and there will be an opportunity to deliver a 'final solution' scheme which concentrates on active travel movements through the junction.
- 1.1.8 Following the submission of Versions 6 and 8 of this TN to OCC and the return of initial comments from the Highway Officer, Version 9 provided a redesign of the junction and subsequent remodelling to address the matters raised by OCC i.e. pedestrian route across Bucknell Road, proximity of junction to Network Rail land and existing properties, visibility to signal heads and future location of signs/street lights. Following further discussions with OCC this version addresses the issues raised regarding the possible conflict of articulated vehicles turning left from Howes Lane with vehicles waiting at the stop line on Bucknell Road and a reduction in the length of the right turn lane into Howes Lane to 2 pcus. The design has been updated to provide cycling infrastructure through the introduction of advance stop lines and feeder lanes for cyclists on all movements. Additionally, junction capacity modelling has been undertaken using both the Bicester Transport Model (BTM) and the 'vision' trip rates.

- 1.1.9 The Firethorn planning application (Ref: 21/01630/OUT) for 530 residential dwellings, with a recommendation by the planning officer for permission to be granted (subject to the expiry of the consultation period, conditions and subject to a s106 agreement), was considered by Councillors at the CDC Planning Committee held on 12th January 2023; the application was deferred due to the need of Councillors to consider the submission of late comments.
- 1.1.10 Following extensive discussions between the applicant's transport consultants and OCC Highways regarding the effect of the development on the Howes Lane (A4095) / Bucknell Road / A4095 junctions, OCC have concluded that a 50% increase in delay at the junction is not a 'severe' impact and that in "the context of an increasingly urban setting, drivers will become accustomed to congestion on all routes into and around Bicester by 2026". The accepted increase will increase delay for vehicles travelling through the junction from 6 minutes to 9 minutes.
- 1.1.11 It is therefore clear from this decision that OCC considered that the additional traffic associated with the 530 dwellings would not impact on the safety of pedestrian and cycle movements through the junction.
- 1.1.12 OCC have also given weight to the delivery of the SLR, stating "the most challenging element of the road scheme, the bridge under the railway, has already been delivered" and that "there are current development proposals on the land required for the scheme, which means the land can potentially be safeguarded". OCC Highways conclude that the impact of the development, given the future delivery of the SLR will be temporary and is not considered to be severe.
- 1.1.13 OCC removed their Highways Objection and thus a precedent has been set, regardless of the result of the Planning Committee. This decision would enable a minimum of 530 dwellings in North West Bicester to be occupied with no mitigation at the Howes Lane (A4095)/Bucknell Road/A4095 junction.
- 1.1.14 It is understood that since the Firethorn application was deferred at committee the applicants have lodged a planning appeal.

2.0 Technical Background

- 2.1.1 OCC have specified that the recently revalidated BTM is the preferred modelling tool to assess future development and mitigation options in Bicester. It is OCC's model and the development trip generation used in this assessment is undertaken using both the model's trip generation and the 'vision' trip generation as discussed with OCC. Therefore, it is understood that the traffic flow movements produced by the BTM and used for the purposes of this assessment, are in accordance with the requirements of OCC.
- 2.1.2 Tetra Tech, custodians of the Bicester Transport Model (BTM), were commissioned by HLM to undertake SATURN model runs of the following scenarios:
 - 2026 Reference Case Base + Committed Traffic;
 - 2026 Hawkwell with Development 1a 675 dwellings;
 - 2026 Hawkwell with Development 2a 1250 dwellings;
 - 2026 Hawkwell with development 1b 675 dwellings; and
 - 2026 Hawkwell with development 2b 1250 dwellings.
- 2.1.3 Tetra Tech provided turning movements for each scenario attached at **Appendix A**. From these turning movements, Traffic Flow Diagrams, attached at **Appendix B**, have been produced.
- 2.1.4 It should be noted that this version of the TN uses the revised turning movements provided by Tetra Tech (26th January 2023). It is understood that an issue was identified requiring the demand model to be altered which led to the 2026 and 2031 'with development' scenarios to be rerun.

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3.0 Review of the Design of the Existing Junctions

Pedestrian Infrastructure and Movements

- 3.1.1 The junction currently offers no controlled crossing facilities for pedestrians. The following facilities are currently available at the southern priority junction to enable pedestrian movements:
 - Dropped kerb crossing on the A4095(W) arm crossing 3 lanes of traffic;
 - Dropped kerb crossing on Bucknell Road in the vicinity immediately south of the railway bridge with limited visibility to vehicles turning left from the A4095(W) arm and requiring pedestrians to look behind: and
 - No facilities on the Bucknell Road(S) arm.
- 3.1.2 **Figure 3.1** records the width of the existing footways in the vicinity of the southern junction.
- 3.1.3 The DfT report 'Inclusive Mobility' sets out a standard width of 2m for footways with a minimum width of 1.5m where there are physical restraints; it also states that the width can be reduced to an absolute minimum of 1m but that this can only occur over a maximum distance of 6m.

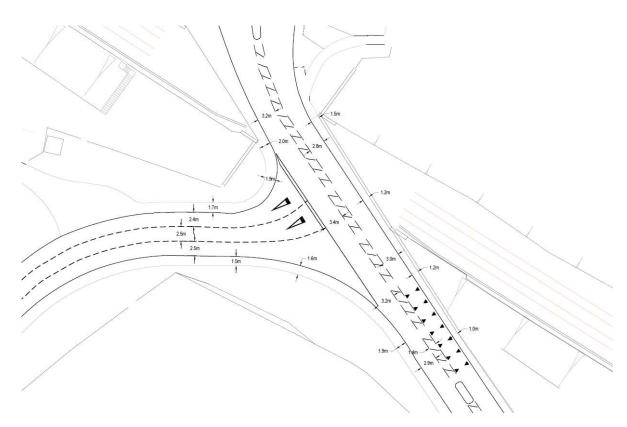


Figure 3.1 – Existing geometry at the Southern Priority Junction

- 3.1.4 It can be seen that the eastern footway is narrow and does not meet the minimum width and the remaining footways are of varying widths between 1.0 and 1.5m.
- 3.1.5 The southern footway varies in width between 1.5 and 1.9m and the northern footway varies in width between 1.7 2.0m.
- 3.1.6 A classified count of all movements through the two junctions was undertaken on Tuesday 8th February 2022 during the AM and PM peak periods.
- 3.1.7 The recorded pedestrian movements for the AM and PM peak hours are shown in Figure 3.2.

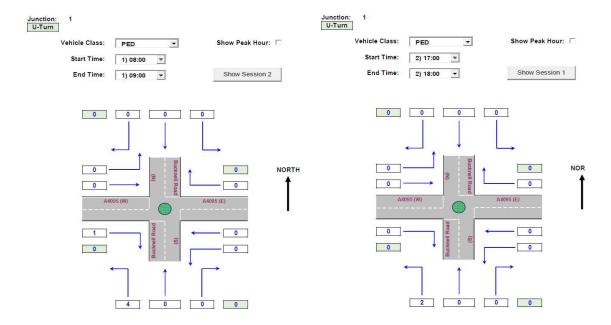


Figure 3.2 – 2022 Survey – Peak Hour Pedestrian Movements

- 3.1.8 The survey indicates that there were 5 and 2 pedestrian movements through the junction during the AM and PM peak hours respectively which consisted of:
 - All AM movements were between the A4095(W) and the Bucknell Road (S) arms; no movements were recorded across Bucknell Road i.e. in the vicinity of the railway bridge; and
 - Both PM movements were between the A4095(W) and the Bucknell Road(S) arms; no movements were recorded across Bucknell Road.
- 3.1.9 The results indicate that there is only a limited number of pedestrian movements through the junction during the peak hours and highlights that movements across Bucknell Road (immediately south of the railway bridge) will be very low. It is recognised by OCC that the main desire line for pedestrians from the Hawkwell Village development will be towards the town centre along the existing active travel route adjacent to the railway line and therefore, additional pedestrian movements across the junction arising from the proposed development will be limited.

Cyclist Movements

- 3.1.10 Cycle movements through the junction take place within the carriageway with cyclists/vehicles giving way in the normal manner at priority and roundabout junctions. The movement of cyclists through these types of junctions gives rise to a large number of conflict points between vehicles and cyclists i.e. cyclists are required to judge vehicle speeds and whether they have time to pull out and the visibility of cyclists by drivers when travelling through a junction with the possibility of drivers pulling out into a cyclist's path.
- 3.1.11 The recorded cyclist movements for the AM and PM peak hours are shown in Figure 3.3.

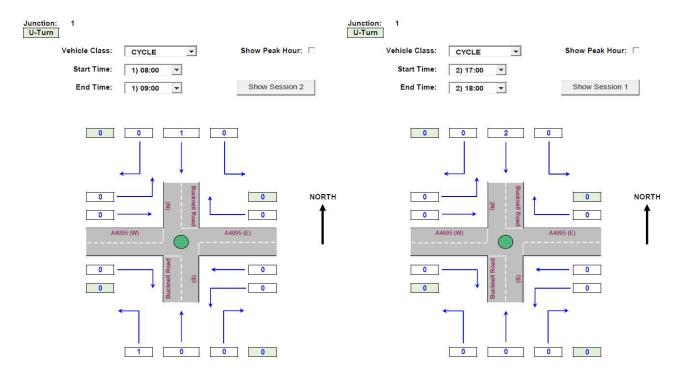


Figure 3.3 – 2022 Survey – Peak Hour Pedestrian Movements

- 3.1.12 The survey indicates that there were 2 cyclist movements through the junction during both the AM and PM peak hours and consisted of:
 - 1 movement between the Bucknell Road(S) and the A4095(W) arms and 1 movement between the Bucknell Road (N) and the Bucknell Road (S) arms in the AM peak hour; and
 - Both movements were between the Bucknell Road (N) and Bucknell Road(S) arms in the PM peak hour.
- 3.1.13 The results indicate that there is only a limited number of cyclist movements through the junction during the peak hours. It is recognised by OCC that the main desire line for cyclists from the Hawkwell Village development will be towards the town centre along the existing active travel route adjacent to the railway line and therefore, additional cycle movements through the junction arising from the proposed development will be limited.

Articulated Vehicle Movements

3.1.14 Tracking of articulated vehicles through the existing southern junction has been undertaken and is shown at **Appendix C** and reproduced in **Figure 3.4**.



Figure 3.4 – Existing Southern Junction Vehicle Tracking

- 3.1.15 The tracking clearly shows that a left-turning articulated vehicle approaching the junction on the A4095(W) arm requires the use of both approach lanes i.e. blocks right-turners, and is unable to carry out its manoeuvre without southbound vehicles on Bucknell Road (underneath the bridge) stopping as the articulated vehicle passes over the centre line of the carriageway. This articulated vehicle manoeuvre and the requirement for other vehicles to stop is shown clearly in Figure 2.4 of the Velocity Transport Planning (VTP) Technical Note 08 which is reproduced below in **Figure 3.5.**
- 3.1.16 Tracking of an articulated vehicle turning right into A4095(W) also indicates the requirement for the vehicle to use the southern approach lane on the A4095(W) arm i.e. vehicles turning right from the A4095(W) arm would either need to clear their move before the articulated vehicle movement or to wait a sufficient distance from the junction to enable the articulated vehicle to undertake its manoeuvre.
- 3.1.17 The tracking indicates that the east/west movement of articulated vehicles through the southern junction is currently unsuitable and gives rise to highway safety concerns; the movement of articulated vehicles relies on the ability of other vehicles to give-way in unexpected places whilst progressing through the junction i.e. where their movement has right of way / is free flowing and they are not expected to have to stop.



Figure 3.5 – Observed Articulated Vehicle Movement through Southern Junction

3.1.18 The recorded articulated vehicle movements for the AM and PM peak hours are shown in **Figure 3.6.**

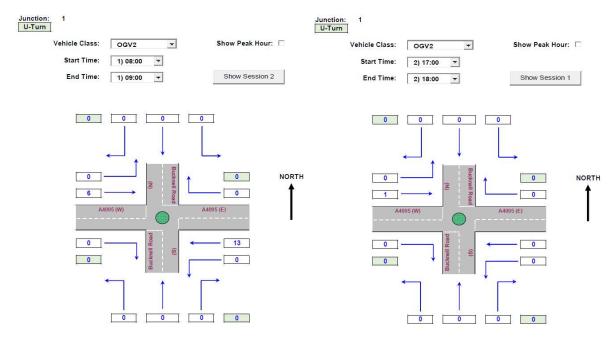


Figure 3.6 – 2022 Survey – Peak Hour Articulated Vehicle Movements

3.1.19 It can be seen that articulated vehicle movements only occur between the A4095 eastern and western arms.

3.1.20 In the AM peak hour there are 6 articulated vehicle movements from A4095(W) to A4095(E) and 13 articulated vehicle movements from A4095(E) to A4095(W). In the PM peak hour there is a single articulated vehicle movement from A4095(W) to A4095(E).

Junction Capacity Operation

- 3.1.21 Using the traffic flows provided by the BTM, the '2026 Reference Case' operation of the existing junctions has been undertaken using the JUNCTIONS 10 software. Due to the proximity of the two junctions and their operation being dependent on each other, the modelling has been undertaken using 'linked lane simulation'. A summary of the results is shown in **Table 3.1** with the output report attached at **Appendix D**.
- 3.1.22 The roundabout is modelled as Junction 1 and the priority junction as Junction 2.

		АМ			PM	
Arm	Queue (PCU)	Delay (s)	Max LOS / RFC	Queue (PCU)	Delay (s)	Max LOS / RFC
J1 – Bucknell Road (North)	1.1	14.74	В	0.7	11.40	В
J1 - A4095 (East)	184.6	622.83	F	193.5	638.13	F
J1 – Bucknell Road (South)	1.2	4.68	А	1.2	4.50	А
J2 – Bucknell Road (South)	0.2	0.68	А	0.1	0.47	А
J2 – A4095 Howes Lane (West)	53.4	291.99	F	232.6	972.07	F
J2 – Bucknell Road (North)	3.1	16.51	С	3.0	14.67	В

Table 3.1 – Summary of Junction Results for Existing Junctions – Base + Committed Development

- 3.1.23 The 'linked lane simulation' results indicate that the existing junctions will operate over capacity in 2026 with committed traffic. The committed development traffic is generated by the following committed development that is included in the 2026 BTM: 8,085 dwellings, 15,642sqm retail use, 308,335sqm employment use and 1,899 jobs of other employment use.
- 3.1.24 Queues on the A4095 East arm of Junction 1 (roundabout) are predicted to be 185 pcus in the AM peak hour and 194 pcus in the PM peak hour. In the AM peak hour queues will extend to just before the A4095 / B4100 (Banbury Road) roundabout and in the PM peak hour the queue will extend across the A4095 / B4100 (Banbury Road) roundabout.
- 3.1.25 Queues on the A4095 West arm of Junction 2 (priority) are predicted to be 54 pcus in the AM peak hour and 233 pcus in the PM peak hour. These queues extend across the Avonbury Business Park and Thames Valley Police junctions in the AM peak hour and to just before the A4095/ B4030 / Middleton Stoney Road roundabout in the PM peak hour.
- 3.1.26 **Figure 3.7** shows a visual representation of the predicted traffic queues for the '2026 Reference Case' scenario.

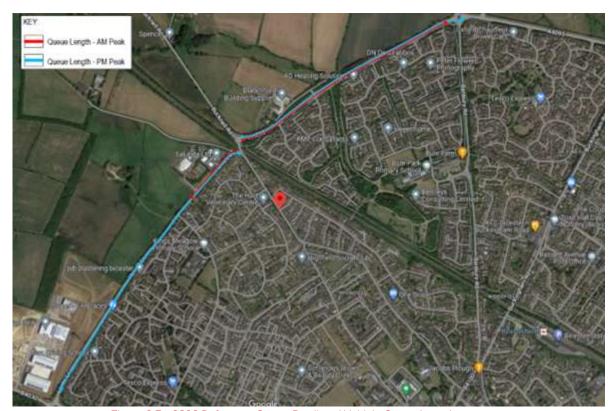


Figure 3.7 - 2026 Reference Case - Predicted Vehicle Queue Lengths

- 3.1.27 It should be noted that the BTM predicts 2,105 AM vehicle movements and 2,342 PM vehicle movements through the junctions in the 2026 Reference Case Scenario. The 2022 surveys recorded 1,904 and 1,636 vehicle movements in the AM and PM peak hours respectively. The growth between the recorded 2022 and the BTM '2026 Reference Case' is 11% in the AM peak hour and 43% in the PM peak hour. The 11% increase in the AM peak hour over a four-year period is considered to be an acceptable increase but the increase of 43% in the PM peak hour is excessively high. This anomaly between the BTM model results and surveyed traffic flows in the PM peak hour is also highlighted in VTP's TN 008 where a 61% difference was identified.
- 3.1.28 OCC have commented that the queues lengths reported by the '2026 Reference Case' modelling appear to be longer than modelling undertaken for other developments. As requested, the modelling has been undertaken using the BTM traffic flows and as the model provides future year traffic flows only, it is not possible to undertake a validation exercise. In addition, the modelling undertaken for the Firethorn development has assessed and validated only the southern priority junction; due to the proximity of the two junctions and their obvious interaction and subsequent blocking of movements, it is considered that the presented modelling is a more accurate assessment of the junctions' performance. Finally, as shown in this report, the mitigation scheme does not only mitigate the development traffic (i.e. the performance of the junction remains the same as the '2026 Reference Case' scenario) but it mitigate all traffic (i.e. background + committed) and therefore, the performance relative to the existing design is not of relevance; it is sufficient to understand that the existing junctions are operating over capacity and the mitigation scheme provides a comprehensive solution.

4.0 Proposed Interim Mitigation Scheme

4.1.1 In order to improve safety, active travel facilities and reduce queues and delay, it is proposed to signalise the two junctions with the added benefit of providing controlled pedestrian / cycle crossing points and introduces advanced stop lines and feeder lanes for cyclists on all approaches. The preliminary design drawing is attached at **Appendix E** and reproduced below in **Figure 4.1.**



Figure 4.1 – Proposed Junction Layout

Pedestrian Improvements

- 4.1.2 The widths of the footways on the southern junction have been rationalised and improved. The footway alongside the eastern edge of Bucknell Road has been widened from 1.0-1.5m to a constant 2.0m. The footway from Bucknell Road(S) to A4095(W) has been widened from 1.5-1.9m to a constant 2.0m. The footway from A4095(W) to Bucknell Road (N) has been rationalised from between 1.7-1.9m to 1.8m; there is the opportunity to widen this footway to 2.0m but the widening would require a reduction in width between the southern footway and the Goldsmith Close fence; it is considered that as this is an interim scheme and the surveyed level of pedestrian movements is minimal /will remain minimal that a 1.8m footway is suitable.
- 4.1.3 In the proximity of the existing dropped kerb uncontrolled pedestrian crossing point on Bucknell Road (immediately south of the railway bridge) a single movement controlled pedestrian crossing has been introduced providing a safer environmental for east/west pedestrian movements.
- 4.1.4 An additional controlled pedestrian crossing has been introduced on the Bucknell Road(S) arm where currently there are no pedestrian crossing facilities and provides a safer environment for pedestrians.

4.1.5 An uncontrolled pedestrian crossing facility with a mid-crossing refuge has been introduced on the A4095(W) arm. The refuge enables pedestrians to cross the road in two stages meaning that only one lane of traffic needs to be crossed at a time and significantly improving the safety of pedestrians who currently have to cross three lanes of traffic in a single movement.

Cycling Improvements

- 4.1.6 The introduction of signals will remove the number of conflicts with vehicles travelling through the junction as cyclists will be within a 'green light' stream of traffic which will have a priority to undertake its manoeuvre.
- 4.1.7 The design of the junction has been updated to accommodate advance stop lines and feeder lanes for cyclists on all approaches enabling cyclists to be at the head of a 'green light' movement.
- 4.1.8 The interim scheme is unable to deliver off-road cycling infrastructure, but the 'final solution' scheme will seek to facilitate off road movements through the transfer of road space from vehicles to active road users.
- 4.1.9 OCC have forwarded the observations and a design proposal from the Bicester Biker User Group (BBUG). The BBUG design was based on an earlier junction design and some of the requested concepts have been incorporated in later iterations of the scheme design. The proposed interim scheme now incorporates significant pedestrian and cyclist improvements as well as providing the vehicle capacity that is required until the SLR is open to traffic. When traffic is diverted along the SLR there will be significantly less traffic travelling through the junction. The 'final solution' scheme will reallocate road space to enable off-road provision to be provided and the observations of the BBUG will be further considered.

Articulated Vehicle Improvements

- 4.1.10 Whilst articulated vehicle movements were only observed travelling from/to the A4095(W) and A4095(E) the junction has been designed to enable the safe movement of articulated vehicles from and to all arms.
- 4.1.11 The vehicle tracking attached at **Appendix F** shows significant improvements to the left-out and right-in articulated vehicle movements from/to the A4095(W) arm. Both of these articulated vehicle movements can now be undertaken without the use of the A4095(W) right-turn lane and without the use of the eastern carriageway of Bucknell Road under and immediately south of the railway bridge. All articulated vehicle movements can now be undertaken alongside all other vehicle movements through the junction and significantly improve highway safety and reduce delays through the junction.
- 4.1.12 Following the most recent comments received from OCC the stop line on Bucknell Road immediately south of the rail bridge has been moved north to provide greater separation between articulated vehicle movements. Additionally the right turn lane into Howes Lane has been shortened to 2 pcus to reflect a more natural vehicle movement.

Detailed Design Matters

- 4.1.13 The visibility to signal heads is provided at **Appendix G**. This indicates that all primary signal heads on the main approach arms of the junction have 90m of visibility with 43m of visibility achievable for the signal heads within the central area of the junction.
- 4.1.14 The location of existing signage and where required, its repositioning is shown on the drawing attached at **Appendix H**.
- 4.1.15 The location of existing street lighting and where required its repositioning, is shown on the drawing attached at **Appendix H**.

- 4.1.16 The positioning of the signal head on the eastern side of Bucknell Road (south of the railway bridge) will not affect the safe movement of pedestrians. The existing footway is 1.2m wide and supports a street light (with a shaft diameter of a minimum 140mm). The proposal will widen the footway to 2.0m; signal head poles have a diameter of 114mm). Therefore, the signal head pole diameter is less than the street light and an additional 800mm of footway is being provided. Signal heads are required to have a minimum height clearance of 2.1m where pedestrians are present.
- 4.1.17 OCC raised an observation in respect of HGVs over-running the kerb when turning left from Howes Lane in the vicinity of pedestrians waiting at the proposed crossing. Whilst it is considered that there is sufficient carriageway for HGVs to undertake this movement it is proposed that at the detailed design stage high kerbs are introduced around this corner to ensure vehicles remain within the carriageway.

Junction Capacity Operation

- 4.1.18 Junction capacity modelling has been undertaken using the LINSIG software. A summary of the results is shown in **Table 4.1** with the output reports attached at **Appendix I.**
- 4.1.19 The results indicate that with 1,250 dwellings all arms of the junction operate with a Degree of Saturation below 100%. The modelling indicates that all queued vehicles will move through the junction using an optimised 180 second cycle time.
- 4.1.20 The latest design of the interim scheme has taken a slightly different approach, raising pedestrian and cycling movements and amenity for the Goldsmith Close properties above that of vehicle delay for the following reasons:
 - Following the decision of OCC that the traffic associated with the Firethorn application (530 dwellings) did not have a 'severe' impact on delay at the junction and drivers will become accustomed to congestion on all routes into and around Bicester by 2026;
 - the impact of the development, given the future delivery of the SLR will be temporary;
 - comments received from OCC on previous interim schemes requesting improved pedestrian and cycling movement and amenity.
- 4.1.21 Stage 1 and Stage 2 for Stream 1 and Stream 2 have been altered to allow the opposed phase to run first.

			AM			PM						
Arm	Lane	DoS (%)	Queue (PCUs)	Delay s/PCU	DoS (%)	Queue (PCUs)	Delay s/PCU					
2026 + Commi	tted + 6	75 dwelli	ngs (BTM	Trip Gene	eration)							
Bucknell Road (North)	-	33.0	8.5	44.9	38.6	9.4	52.7					
A4095 (East)	Left	95.2	43.6	74.5	88.2	39.5	49.1					
A4093 (Last)	Right	93.1	-	108.5	88.2	-	96.4					
Bucknell Road (Internal Southbound)	-	95.1	9.8	18.6	94.3	5.5	10.8					
Bucknell Road (South)	Left	96.6	38.3	105.8	89.5	30.2	82.2					
A4095 (West)	Left	58.0	17.2	36.7	85.6	34.6	50.7					
A4093 (West)	Right	58.0	-	86.1	85.6	-	96.8					
Bucknell Road (Internal	Left	54.4	2.8	2.2	73.1	9.1	2.8					
Northbound)	Right	54.4	-	0.3	73.1	-	0.5					
PRC (%)			-7.3			-4.8						
Cycle Time		180s										
2026 + Commit	ted + 12	50 dwell	ings (BTM	1 Trip Gen	eration)							

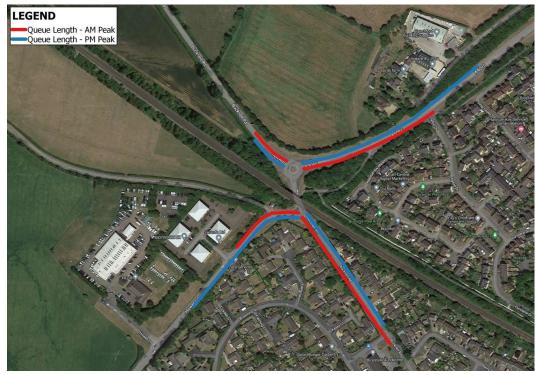
Arm Bucknell Road (North) A4095 (East)	Lane Left Right	DoS (%) 41.8	Queue (PCUs)	Delay s/PCU	DoS (%)	Queue	Delay
A4095 (East)			0.5		(/0)	(PCUs)	s/PCU
		01.0	9.5	58.1	35.0	9.4	43.3
	Right	81.3	31.9	39.9	95.7	47.4	74.9
		96.6	-	92.3	95.7	_	117.3
Bucknell Road (Internal Southbound)	-	93.8	10.7	17.9	93.8	8.0	10.2
Bucknell Road (South)	Left	95.2	36.9	97.5	98.1	35.3	126.7
A / OOF (\Mask)	Left	57.2	17.0	35.8	79.0	30.6	39.4
A4095 (West)	Right	57.2	-	87.8	79.0	-	91.9
Bucknell Road (Internal	Left	52.3	11.5	6.1	74.7	3.5	2.5
Northbound)	Right	52.3	-	1.0	74.7	-	0.6
PRC (%)	1		-7.3			-9.0	
Cycle Time				18	0s		
2026 + Comm	itted + 67	5 dwellir	ngs (Vision	n Trip Ger	eration)		
Bucknell Road (North)	-	34.3	8.9	45.2	37.7	9.1	52.5
A4095 (East)	Left	92.7	39.5	65.5	87.1	38.2	47.6
A+093 (Last)	Right	88.6	_	100.4	87.1	-	95.2
Bucknell Road (Internal Southbound)	-	93.7	8.8	16.9	92.6	5.0	10.5
Bucknell Road (South)	Left	91.8	34.5	82.4	89.6	30.2	82.4
A4095 (West)	Left	58.8	17.2	39.3	85.9	34.8	51.0
A4093 (West)	Right	58.8	-	86.6	85.9	-	97.1
Bucknell Road (Internal	Left	54.3	3.5	2.4	72.4	7.5	2.5
Northbound)	Right	54.3	_	0.6	72.4	-	0.5
PRC (%)	·		-4.1			-2.9	
Cycle Time				18	0s		
2026 + Commi	tted + 12	50 dwelli	ngs (Visio	n Trip Ge	neration)		
Bucknell Road (North)	-	40.5	9.6	54.7	33.1	8.8	42.9
A4095 (East)	Left	81.8	31.7	42.8	94.3	45.1	69.6
7.17000 (Last)	Right	94.7	-	92.7	94.3	-	12.5
Bucknell Road (Internal Southbound)	-	91.9	10.4	18.1	90.9	7.0	8.8
Bucknell Road (South)	Left	94.8	37.0	95.0	933	31.1	97.8
A4095 (West)	Left	55.8	16.2	35.9	82.1	32.4	43.6
M4030 (WESL)	Right	55.8	-	87.8	82.1	-	93.9
Bucknell Road (Internal	Left	52.5	12.1	6.8	73.3	2.7	2.0
Northbound)	Right	52.5	_	1.1	73.3	-	0.6
PRC (%)			-5.4	-		-4.8	
Cycle Time				18	0s		

Table 4.1 – Summary of LINSIG Results for Signals Mitigation Scheme

- 4.1.22 The existing junction is known to be congested and, this congestion at a land constrained junction, is the main reason for the delivery of the SLR. The loss of the Homes England funding means that unless alternative funding is sourced, the cost of this expensive piece of infrastructure will have to be met by the developments that will increase traffic flows along the A4095. To enable the Hawkwell Village development to be able to fund its share of the SLR requires a period of house sales to be undertaken. The aim of this TN is to provide a mitigation solution that provides additional vehicle capacity until the developer is able to fund its share of the SLR; a positive of the design is that pedestrian and cycling safety can be improved through the provision of crossing facilities and the widening and ratification of existing footways for pedestrians and through the introduction of advanced stop lines and feeder lanes for cyclists.
- 4.1.23 OCC have commented that 180s cycle time is too long for pedestrian phases; this proposal is an interim scheme and OCC, as the highway network manager, is able to reduce the cycle time to improve pedestrian wait times at the expense of vehicle capacity. Additionally, the number of pedestrian movements through the junction is very low and the number of pedestrian movements that will use the crossing facilities is even lower; the Hawkwell Village acknowledged main pedestrian desire line will be along the active travel route adjacent to the railway line to the town centre and therefore, in practical terms there will not be a significant increase in pedestrian movements due to the development.
- 4.1.24 The queues on the A4095 eastern arm are significantly improved. With 1,250 dwellings there is a queue of 32 pcus (BTM and 'Vision') in the AM peak hour and 48 pcus (BTM)/45 pcus ('Vision') in the PM peak hour. Comparing the junction's operation in 2026 with development against the BTM '2026 Reference Case' there is a reduction in queues on this arm of 153 pcus (BTM and 'Vision') in the AM peak hour and 146 pcus (BTM)/149 pcus ('Vision') in the PM peak hour. Maximum delay reduces from 623s per pcu to 92s per pcu (BTM)/ 93s per pcu ('Vision') in the AM peak hour and from 638s per pcu to 117s per pcu (BTM) / 70s per pcu ('Vision') in the PM peak hour, a saving of 8-10 minutes on travel times in the peak hours. These results indicate that the interim scheme will operate with a significantly lower level of delay than the 9 minutes deemed acceptable by OCC when considering the impact of traffic associated with the 530 Firethorn dwellings at the junctions.
- 4.1.25 The queues on the A4095 western arm also see a significant reduction generating a queue of 17 pcus (BTM)/ 16 pcus ('Vision') in the AM peak hour and a queue of 31 pcus (BTM)/ 32pcus ('Vision') in the PM peak hour with the traffic generation of 1,250 dwellings and removes any knock-on impact on nearby junctions with the exception of the Avonbury Business Park junction in the PM peak hour. Comparing the junction's operation in 2026 with development against the BTM '2026 Reference Case' there is a reduction in queues on this arm of 36 pcus (BTM)/37 pcus ('Vision') in the AM peak hour and 202 pcus (BTM)/201 pcus ('Vision') in the PM peak hour. Maximum delay reduces from 292s per pcu to 88s per pcu (BTM and 'Vision') in the AM peak hour and from 972s per pcu to 92s per pcu (BTM)/ 84s per pcu ('Vision') in the PM peak hour, a saving of 3½ minutes in the AM peak hour and 14½ minutes in the PM peak hour. These results indicate that the interim scheme will operate with a significantly lower level of delay than the 9 minutes deemed acceptable by OCC when considering the impact of traffic associated with the 530 Firethorn dwellings at the junctions.
- 4.1.26 The introduction of signals introduces a delay to the movements on Bucknell Road (i.e. north and south arms). However, the assessment of 1,250 dwellings, indicates a queue of 9 10 pcus (BTM and 'Vision') on the Bucknell Road North arm in the AM and PM peak hours respectively and queues of 37 (BTM and 'Vision') and 37 pcus (BTM)/31pcus ('Vision') on the Bucknell Road South arm in the AM and PM peak hours respectively. The predicted delay is considered within a normal range for a signalised junction in an urban environment and is below the 9 minutes deemed acceptable by OCC when considering the impact of traffic associated with the 530 Firethorn dwellings at the junctions.
- 4.1.27 The revised interim scheme differs from previous designs as it only provides a single lane approach on the Bucknell Road(S) arm in order to improve amenity for the Goldsmith Close properties and retain the existing trees; this change has meant that during the AM peak hour the vehicle queue will stretch across the Kingsley Road/Bucknell Road(S) junction and it is proposed to introduce 'Keep Clear' road markings.
- 4.1.28 **Figure 4.2** and **Figure 4.3** show a visual representation of the predicted traffic queues for the '2026 Reference Case + 1250 dwellings' BTM and 'Vision' scenarios.



Figure 4.2 – 2026 + 1250 Dwellings (BTM) – Interim Scheme Predicted Vehicle Queue Lengths



 $\textit{Figure 4.3} - 2026 + 1250 \ \textit{Dwellings ('Vision')} - \textit{Interim Scheme Predicted Vehicle Queue Lengths}$

5.0 Future Junction Design

- 5.1.1 From discussions with OCC it is apparent that the final scheme should provide significant active travel improvements and that a significantly different design to the interim scheme is considered to be required. As shown in previous versions of this TN, when the SLR is delivered there will be a significant reduction in vehicle movements through this junction and therefore, there will be an opportunity to design a scheme which reduces carriageway space and increases space for active travel routes.
- 5.1.2 A design for the future design of this junction is no longer included in this TN and will be provided at a later date following discussion with A2Dominion.
- 5.1.3 OCC have requested further detail in respect of the proposal to provide an all-vehicle link road between the western Hawkwell Village site access and the Bucknell Road / A4095 existing roundabout junction i.e. to the south of the SLR. This all-vehicle link road enables the removal of the previously proposed junction on the SLR to the east of the western site access and will reduce delay to vehicles on the SLR and delay/obstruction to the free flow of cycle movements on the southern side of the SLR.
- 5.1.4 The proposal is shown in the drawing attached at **Appendix J**; it should be noted that this drawing includes the previous 'final solution' scheme for the A405/Bucknell Road junctions which is now to be redesigned as mentioned above.
- 5.1.5 The proposal will be introduced alongside the proposed Bucknell Road traffic calming scheme that was introduced in Appendix B of TN05 and is reproduced at **Appendix K** of this report. The introduction of traffic calming measures has been designed to increase journey times along Bucknell Road and reduce the attractiveness of the route which is currently used to access the M40 Junction 10.

6.0 Summary and Conclusion

- 6.1.1 The existing Howes Lane (A4095) / Bucknell Road / A4095 junctions currently exceed capacity. Modelling of the two junctions for a '2026 + Committed Development' scenario indicates that there will be significant queuing on the two A4095 arms and therefore, any further development would only increase the gueues and journey times through the junction.
- 6.1.2 An interim mitigation scheme that signalises the two junctions and improves pedestrian severance is proposed to enable initial phases of the proposed Hawkwell Village development to come forward in order to enable funding of the A4095 Strategic Link Road. The scheme also improves articulated vehicle movement through the junction with HGVs accessing and egressing the Howes Lane arm no longer being reliant on the position of other vehicles within the junction and introduces advanced cycle stop lines and feeder lanes to improve cycling movements through the junction.
- 6.1.3 Using traffic flows from the recently revalidated Bicester Transport Model and in accordance with the requirements of OCC, the junction modelling undertaken indicates that the proposed interim scheme will operate within capacity and offer a benefit to the existing configuration, to all road users in terms of queueing, journey times, crossing facilities for pedestrians and cycling improvements, for at least 1,250 dwellings.
- 6.1.4 The reductions in terms of queues and delays at this junction would be extremely beneficial to the local transport network (both public and private modes) and the local economy and also enable the vital delivery of allocated housing at NW Bicester by enabling the occupation of dwellings from which the sale monies will enable funding of the SLR.
- 6.1.5 **Table 6.1** undertakes a comparison of the existing and proposed junction layouts and sets out the interim effects of the proposed scheme.

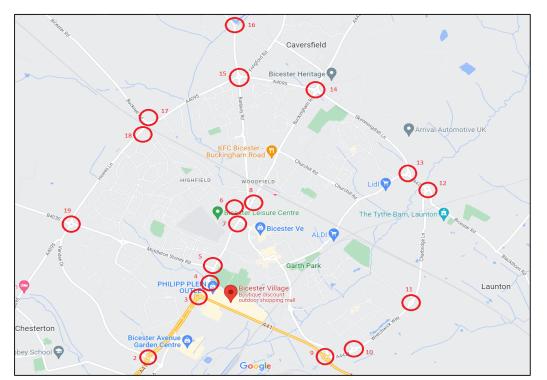
LOCATION	EXISTING JUNCTION	PROPOSED INTERIM JUNCTION	EFFECT
		1250 DWELLINGS	
PEDESTRIANS			
Crossing Facilities			
Bucknell Road (between the two existing junctions)	Uncontrolled crossing in close proximity of vehicles approaching from behind the line of vision	Controlled pedestrian crossing	Positive
Bucknell Road (S)	No crossing facilities	Controlled pedestrian crossing	Positive
A4095 (W)	Uncontrolled crossing across 3 lanes of traffic in close proximity to turning vehicles	Uncontrolled crossing with pedestrian refuge – 1 lane of traffic to be crossed on each movement	Positive
Footway Widths			
East of Bucknell Road	Varying width 1.0 – 1.5m	Widened to constant 2.0m	Positive
Bucknell Road(S) to A4095(W)	Varying width 1.5 – 1.9m	Widened to constant 2.0m	Positive
A4095(W) to Bucknell Road (immediately south of the railway bridge)	Varying width 1.7 – 2.0m	Constant 1.8m	Neutral The footway could be widened to 2.0m but this would narrow the green area between the junction and Goldsmith Close properties by 200mm to 1.3m
CYCLISTS			
	No infrastructure. Cyclist/vehicles give- way to one another on the majority of movements through the junction.	The provision of signals will reduce the number of conflict points between vehicles and cyclists. The proposed scheme will introduce advance stop lines and filter lanes on all approaches.	Positive

LOCATION	EXISTING JUNCTION	PROPOSED INTERIM JUNCTION	EFFECT
		1250 DWELLINGS	
ARTICULATED VEHICLES	•		
A4095(W) left-turn to Bucknell Road	Left-turning articulated vehicles have to use both lanes on approach. Articulated vehicles travel over the centre line of Bucknell Road requiring oncoming southbound vehicles to stop in an unexpected location.	Articulated vehicle manoeuvres take place entirely within the designated lane and do not cross the centre line into opposing traffic stream	Positive. There is an opportunity to increase the green area between the junction and Goldsmith Close by allowing left-turning articulated vehicles to use both lanes on the A4095(W) approach.
Bucknell road right-turn to A4095(W)	Articulated vehicles travel into the A4095(W) right turn approach lane.	Articulated vehicle manoeuvres take place entirely within the designated lane.	Positive
Whole Junction		Whilst articulated vehicles were only recorded travelling through the junction between the A4095(W) and A4095(E) arms, the design of the junction has been undertaken to enable or HGV movements to take place unopposed.	Positive
JUNCTION OPERATION Delay			
A4095(W)	292/972s	88/92 (BTM)	Positive – Delay is 7
AT000(VV)	202/01/20	88/94 ('Vision')	minutes less than the 9 minutes considered acceptable by OCC.
A4095(E)	623/638s	92/117 (BTM) 93/70 ('Vision')	Positive – Delay is 7 minutes less than the 9 minutes considered acceptable by OCC

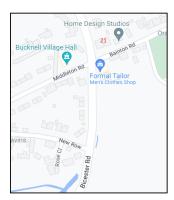
LOCATION	EXISTING JUNCTION	PROPOSED INTERIM JUNCTION	EFFECT
		1250 DWELLINGS	
Bucknell Road(S)	1/1s	98/127 (BTM)	Positive – Delay is 7 minutes less than the
		95/98 ('Vision')	9 minutes considered acceptable by OCC.
Bucknell Road(N)	15/11s	58/43 (BTM)	Positive – Delay is 8
		55/43 ('Vision')	minutes less than the 9 minutes considered acceptable by OCC.
AMENITY - PROPERTIES	ON GOLDSMITH CLOSE		
Goldsmith Close		Approach has been	Neutral
Properties		narrowed to single lane on Bucknell Road	
		(S)to retain existing	
		distance between road	
		and property	
		boundaries.	
		Junction geometry	
		has been minimised to	
		enable the retention of the existing trees and	
		a 1.5m verge.	

- 6.1.6 The table indicates that the interim scheme would overall have a positive effect on the operation of the junction and the pedestrian and cycling environment.
- 6.1.7 The proposed interim mitigation scheme should be considered by OCC as an interim scheme to accommodate at least 1,250 dwellings at Hawkwell Village until the A4095 Strategic Road Link can be funded and delivered. Thereafter, given the future decrease in vehicle movements through the junction, a final solution scheme can be implemented which concentrates on active travel.
- 6.1.8 Whilst an interim mitigation scheme that offers reduced vehicle delay and active travel improvements for the junction has been presented within the TN, the decision of OCC Highways on the vehicle impact (i.e. temporary and not severe) of the Firethorn application for 530 dwellings is considered to be a material consideration on the need to mitigate for the vehicular impact of the Hawkwell Village development at the junction. Therefore, as well as a technical review of the proposed mitigation schemes, the position on the need to mitigate the temporary impact of the Hawkwell Village development traffic at the junction, is sought from OCC.

Appendix A Bicester Traffic Model Outputs









Ref No	Junction Name
1	M40 Junction 9
2	A41 / Vendee Drive roundabout
3	A41 / B4030 Oxford Road signalised roundabout
4	A41 Oxford Road / Pingle Drive signal junction
5	Middleton Stoney Road / Kings End mini roundabout
6	Field Street / Bucknell Road priority junction
7	Queens Avenue / St Johns Street mini roundabout
8	Banbury Road / Field Street mini roundabout
9	A41 / A4421 / B4100 / Gravenhill Road roundabout
10	A4421 / Peregrine Way roundabout
11	Wretchwick Way / Charbridge Lane / Gavray Dr roundabout
12	A4421 / Bicester Road roundabout

Ref No	Junction Name
13	A4421 / Launton Road / Skimmingish Lane roundabout
14	A4421 / Skimmingdish Lane / Buckingham Road / A4095 roundabout
15	B4100 Banbury Road / A4095 Lords Lane roundabout
16	B4100 / Caversfield priority junction
17	A4095 Lords Lane / Bucknell Road roundabout
18	Howes Lane / Bucknell Road priority junction
19	Howes Lane / Middleton Stoney Rd / Vendee Dr roundabout
20	M40 Junction 10
21	Middleton Road / Bainton Road priority junction
22	Sie Access (Western)
23	Site Access (Eastern)

Turning Movement Data Junction Locations and References

Junction							AM Peal	k						Inter Pea	ak						PM Peak	(
Reference	Junction Description	From Arm	To Arm	Car	LGV	HGV (PCU)	HGV (Veh)	Bus (Veh)	Total Veh	Total PCUs	Car	LGV	HGV (PCU)	HGV (Veh)	Bus (Veh)	Total Veh	Total PCUs	Car	LGV	HGV (PCU)	HGV (Veh)	Bus (Veh)) Total Veh	Total PCUs
		Bucknell Road (N)	A4095 (E)	117	8	39	22	0	146	163	54	5	31	17	0	76	89	112	17	25	14	0	143	154
		Bucknell Road (N)	Bucknell Road (S)	26	19	23	13	0	57	68	49	15	20	11	0	76	84	43	30	7	4	0	76	79
		Bucknell Road (N)	Bucknell Road (N)	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	A4095 Lords Lane /	A4095 (E)	Bucknell Road (S)	499	60	160	89	0	648	719	534	92	111	62	0	688	737	704	51	50	28	0	783	805
17	Bucknell Road	A4095 (E)	Bucknell Road (N)	121	24	0	0	0	144	144	11	7	65	36	0	55	84	29	7	49	27	0	63	84
	roundabout	A4095 (E)	A4095 (E)	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		Bucknell Road (S)	Bucknell Road (N)	88	33	26	14	0	135	147	39	17	22	12	0	68	78	118	15	6	3	0	137	140
		Bucknell Road (S)	A4095 (E)	454	22	139	77	0	553	614	410	52	129	72	0	534	591	768	53	92	52	0	873	913
		Bucknell Road (S)	Bucknell Road (S)	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		Bucknell Road (N)	Bucknell Road (S)	140	19	0	0	0	159	159	141	21	0	0	0	162	162	255	32	0	0	0	287	287
		Bucknell Road (N)	Howes Lane (W)	424	60	183	102	0	586	667	443	86	131	73	0	601	659	492	49	56	32	0	572	597
18	Howes Lane / Bucknell Road prioity	Bucknell Road (S)	Howes Lane (W)	216	96	12	7	0	319	325	45	0	10	6	0	51	56	156	63	10	6	0	225	230
10	junction	Bucknell Road (S)	Bucknell Road (N)	257	32	2	1	0	291	292	137	14	2	1	0	152	153	305	19	2	1	0	325	326
	janodon	Howes Lane (W)	Bucknell Road (N)	285	22	162	90	0	397	469	312	55	149	83	0	450	516	582	49	96	54	0	685	727
		Howes Lane (W)	Bucknell Road (S)	12	1	0	0	0	13	13	19	1	0	0	0	19	19	14	0	3	2	0	16	17

Bicester Transport Model Junction Turning Movements (2026 Hawkwell with Development 1a Scenario)

Junction							AM Peal	k						Inter Pea	ık						PM Peak			
Reference	Junction Description	From Arm	To Arm	Car	LGV	HGV (PCU)	HGV (Veh)	Bus (Veh)	Total Veh	Total PCUs	Car	LGV	HGV (PCU)	HGV (Veh)	Bus (Veh)	Total Veh	Total PCUs	Car	LGV	HGV (PCU)	HGV (Veh)	Bus (Veh)	Total Veh	Total PCUs
		Bucknell Road (N)	A4095 (E)	125	8	39	22	0	154	171	45	5	31	17	0	67	81	115	17	25	14	0	146	157
		Bucknell Road (N)	Bucknell Road (S)	27	19	23	13	0	59	69	49	15	20	11	0	75	84	34	30	7	4	0	68	71
		Bucknell Road (N)	Bucknell Road (N)	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	A4095 Lords Lane /	A4095 (E)	Bucknell Road (S)	479	60	160	89	0	628	699	512	90	112	62	0	664	714	694	51	50	28	0	774	795
17	Bucknell Road	A4095 (E)	Bucknell Road (N)	119	24	0	0	0	143	143	11	7	65	36	0	55	83	28	7	49	27	0	62	83
	roundabout	A4095 (E)	A4095 (E)	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		Bucknell Road (S)	Bucknell Road (N)	87	33	26	14	0	134	145	47	17	22	12	0	76	86	111	15	6	3	0	129	132
		Bucknell Road (S)	A4095 (E)	452	19	139	77	0	548	610	403	52	122	68	0	523	577	759	58	92	52	0	869	910
		Bucknell Road (S)	Bucknell Road (S)	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		Bucknell Road (N)	Bucknell Road (S)	134	14	0	0	0	148	148	125	23	0	0	0	149	149	243	32	0	0	0	276	276
		Bucknell Road (N)	Howes Lane (W)	410	65	183	102	0	578	659	435	82	132	73	0	590	649	485	49	57	32	0	566	591
18	Howes Lane / Bucknell Road prioity	Bucknell Road (S)	Howes Lane (W)	213	96	12	7	0	316	322	52	4	10	6	0	62	66	167	63	10	6	0	236	241
18	junction	Bucknell Road (S)	Bucknell Road (N)	268	32	2	1	0	302	303	159	15	2	1	0	174	175	293	19	2	1	0	313	314
	janodon	Howes Lane (W)	Bucknell Road (N)	270	19	162	90	0	380	452	291	54	142	79	0	424	487	576	55	96	54	0	685	727
		Howes Lane (W)	Bucknell Road (S)	15	1	0	0	0	17	17	19	1	0	0	0	19	19	16	0	3	2	0	18	19

Bicester Transport Model Junction Turning Movements (2026 Hawkwell with Development 1b Scenario)

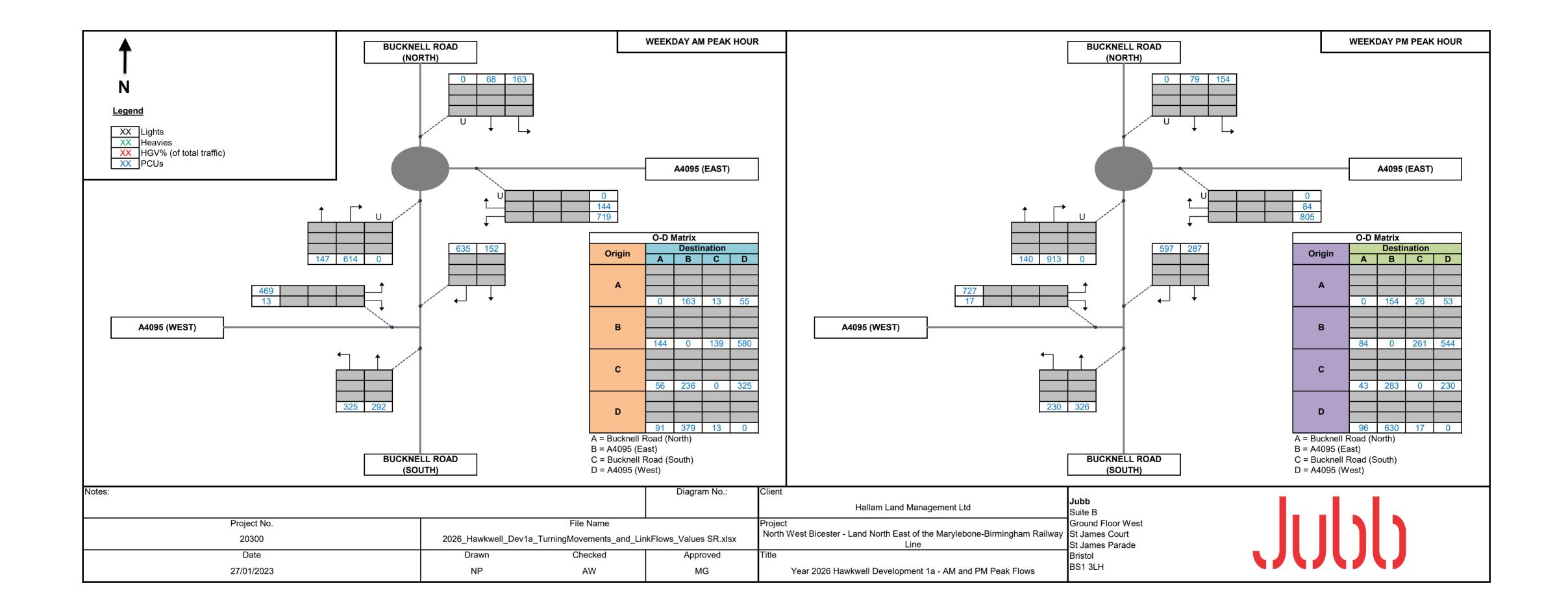
Junction							AM Peal	<						Inter Pea	ık						PM Peak			
Reference	Junction Description	From Arm	To Arm	Car	LGV	HGV (PCU)	HGV (Veh)	Bus (Veh)	Total Veh	Total PCUs	Car	LGV	HGV (PCU)	HGV (Veh)	Bus (Veh)	Total Veh	Total PCUs	Car	LGV	HGV (PCU)	HGV (Veh)	Bus (Veh)	Total Veh	Total PCUs
		Bucknell Road (N)	A4095 (E)	114	8	39	22	0	144	161	57	5	31	17	0	79	92	121	17	25	14	0	152	164
		Bucknell Road (N)	Bucknell Road (S)	23	19	23	13	0	55	65	49	15	20	11	0	76	84	56	30	7	4	0	89	92
		Bucknell Road (N)	Bucknell Road (N)	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	A4095 Lords Lane /	A4095 (E)	Bucknell Road (S)	513	57	160	89	0	660	731	530	92	110	61	0	682	731	704	42	50	28	0	775	796
17	Bucknell Road	A4095 (E)	Bucknell Road (N)	128	24	0	0	0	152	152	12	7	65	36	0	56	85	30	7	49	27	0	64	85
	roundabout	A4095 (E)	A4095 (E)	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		Bucknell Road (S)	Bucknell Road (N)	87	33	26	14	0	134	146	37	17	22	12	0	67	76	114	15	6	3	0	132	135
		Bucknell Road (S)	A4095 (E)	456	19	139	77	0	552	614	424	52	129	72	0	548	605	784	53	92	52	0	889	929
		Bucknell Road (S)	Bucknell Road (S)	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		Bucknell Road (N)	Bucknell Road (S)	153	19	0	0	0	172	172	147	21	0	0	0	168	168	249	32	0	0	0	281	281
		Bucknell Road (N)	Howes Lane (W)	430	57	183	102	0	589	670	432	86	130	72	0	590	647	511	40	56	32	0	583	608
40	Howes Lane /	Bucknell Road (S)	Howes Lane (W)	218	96	12	7	0	321	326	45	0	10	6	0	51	56	119	63	10	6	0	188	192
18	Bucknell Road prioity junction	Bucknell Road (S)	Bucknell Road (N)	257	32	2	1	0	291	292	143	14	2	1	0	159	160	315	19	2	1	0	335	336
	janodon	Howes Lane (W)	Bucknell Road (N)	286	20	162	90	0	396	468	318	55	149	83	0	456	522	583	49	96	54	0	686	729
		Howes Lane (W)	Bucknell Road (S)	12	1	0	0	0	13	13	19	1	0	0	0	19	19	12	0	3	2	0	14	15

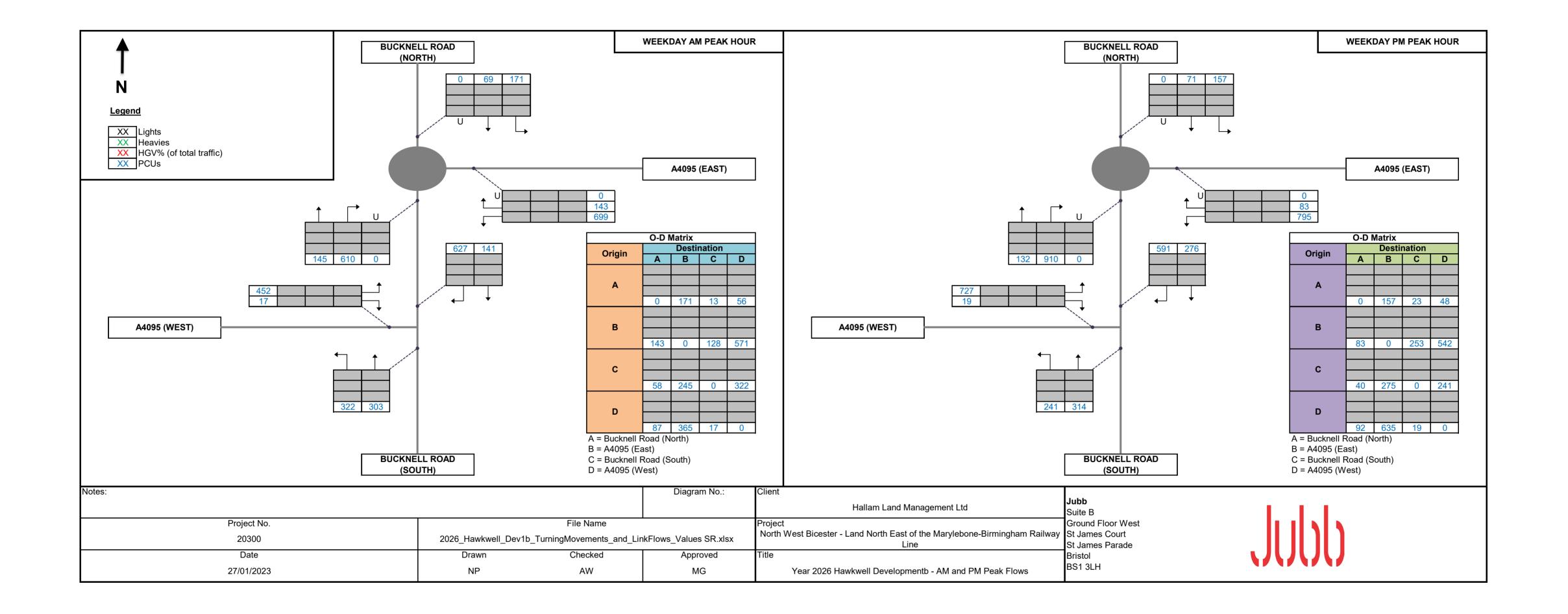
Bicester Transport Model Junction Turning Movements (2026 Hawkwell with Development 2a Scenario)

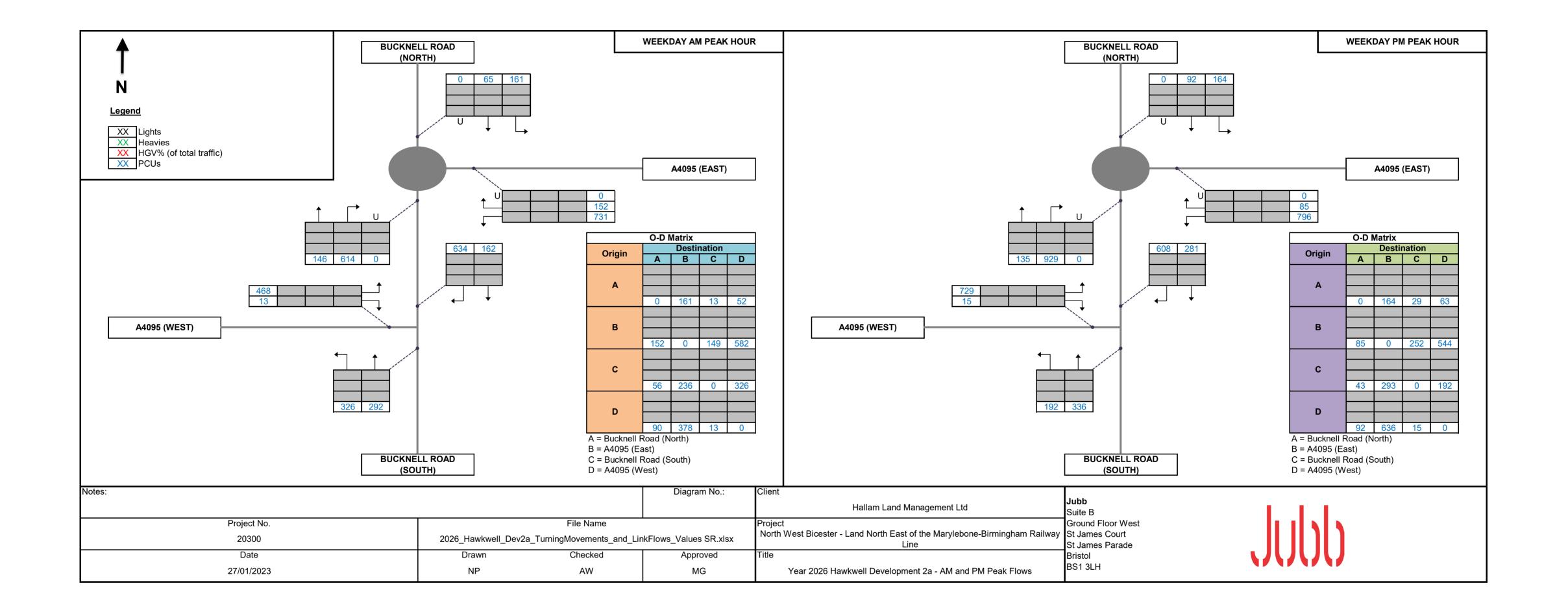
Junction							AM Peal	k						Inter Pea	ak						PM Peak			
Reference	Junction Description	From Arm	To Arm	Car	LGV	HGV (PCU)	HGV (Veh)	Bus (Veh)	Total Veh	Total PCUs	Car	LGV	HGV (PCU)	HGV (Veh)	Bus (Veh)	Total Veh	Total PCUs	Car	LGV	HGV (PCU)	HGV (Veh)	Bus (Veh)	Total Veh	Total PCUs
		Bucknell Road (N)	A4095 (E)	126	8	39	22	0	155	172	48	5	31	17	0	70	84	117	17	25	14	0	148	159
		Bucknell Road (N)	Bucknell Road (S)	22	19	23	13	0	54	64	48	15	20	11	0	75	83	47	30	7	4	0	81	84
		Bucknell Road (N)	Bucknell Road (N)	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	A4095 Lords Lane /	A4095 (E)	Bucknell Road (S)	489	60	160	89	0	639	710	507	89	110	61	0	658	706	691	45	50	28	0	763	785
17	Bucknell Road	A4095 (E)	Bucknell Road (N)	125	24	0	0	0	149	149	12	7	65	36	0	56	85	29	7	49	27	0	63	84
	roundabout	A4095 (E)	A4095 (E)	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		Bucknell Road (S)	Bucknell Road (N)	86	33	26	14	0	133	145	45	17	22	12	0	74	84	107	15	6	3	0	125	128
		Bucknell Road (S)	A4095 (E)	453	18	139	77	0	548	610	417	52	122	68	0	537	591	777	55	92	52	0	884	924
		Bucknell Road (S)	Bucknell Road (S)	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
		Bucknell Road (N)	Bucknell Road (S)	141	14	0	0	0	155	155	123	21	0	0	0	145	145	241	32	0	0	0	273	273
		Bucknell Road (N)	Howes Lane (W)	413	65	183	102	0	581	662	432	83	130	72	0	588	645	497	43	56	32	0	571	595
40	Howes Lane /	Bucknell Road (S)	Howes Lane (W)	214	96	12	7	0	317	322	51	3	10	6	0	60	64	135	63	10	6	0	204	208
18	Bucknell Road prioity junction	Bucknell Road (S)	Bucknell Road (N)	269	32	2	1	0	303	304	165	15	2	1	0	181	182	302	19	2	1	0	322	323
	janodon	Howes Lane (W)	Bucknell Road (N)	270	18	162	90	0	378	450	297	54	142	79	0	430	493	581	52	96	54	0	687	729
		Howes Lane (W)	Bucknell Road (S)	14	1	0	0	0	15	15	19	1	0	0	0	19	19	19	0	3	2	0	20	22

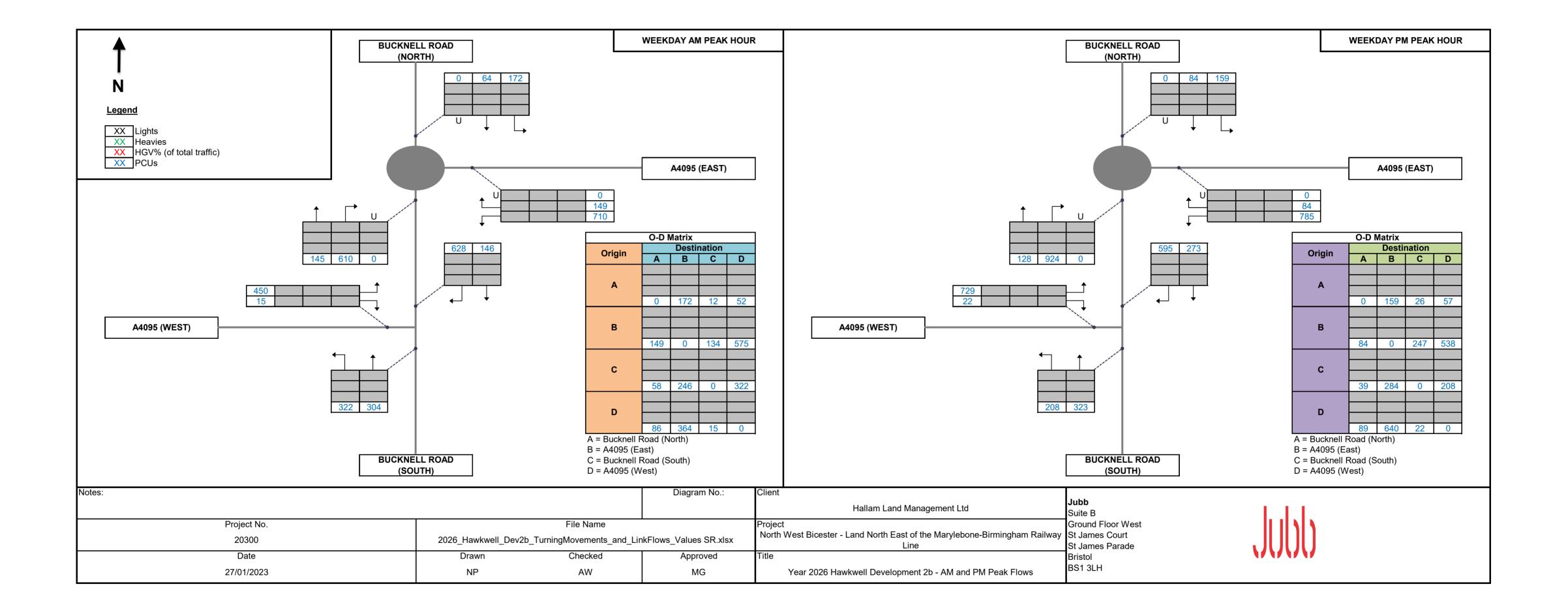
Bicester Transport Model Junction Turning Movements (2026 Hawkwell with Development 2b Scenario)

Appendix B Traffic Flow Diagrams

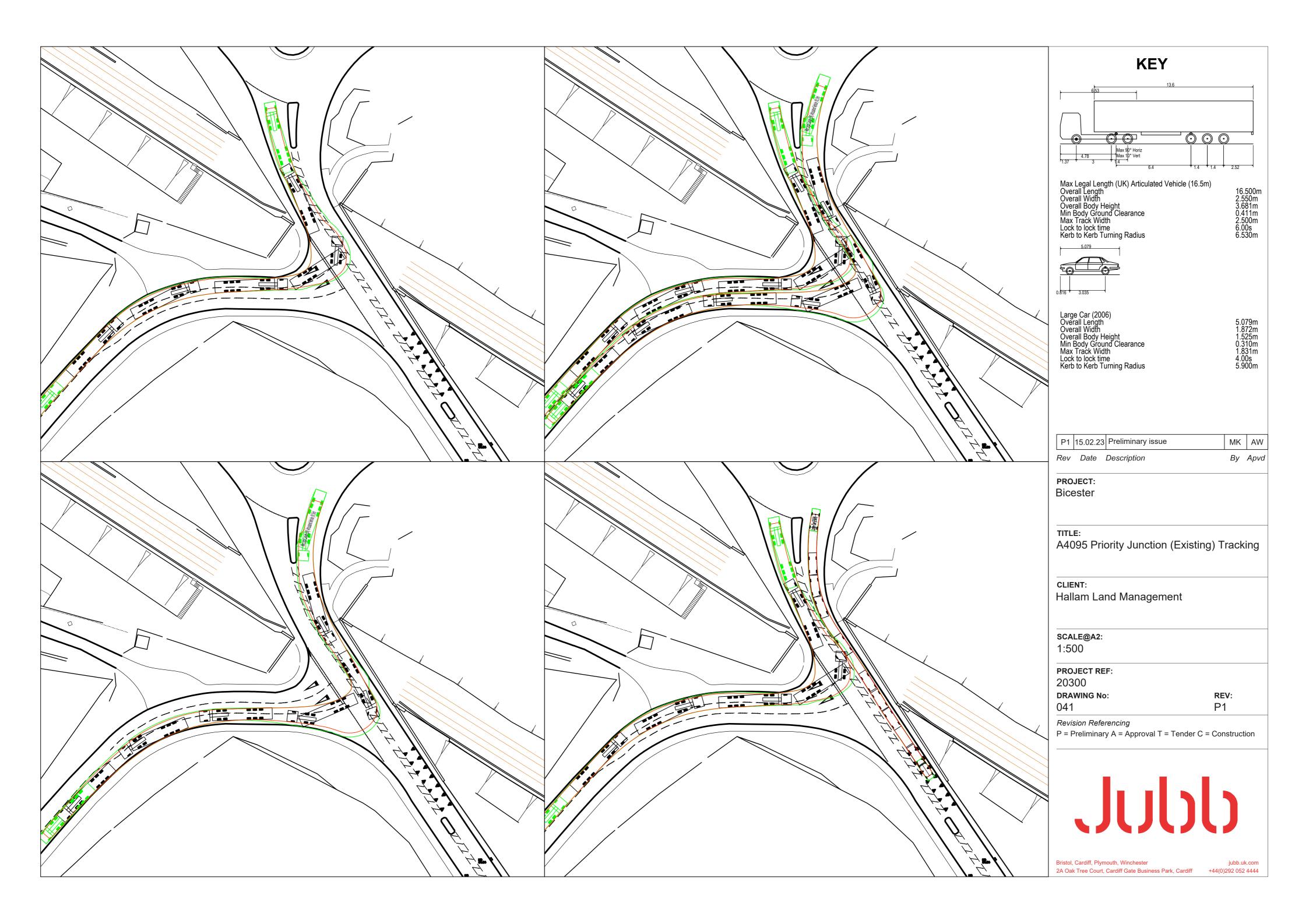








Appendix C Existing Junction Vehicle Tracking



Appendix D Junctions 10 Output Report – Existing Junctions



Junctions 10

ARCADY 10 - Roundabout Module PICADY 10 - Priority Intersection Module

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Filename: A4095_Bucknell Road Junction Network (Existing)_Strategic Model Scenarios_2022.08.12 (Linked).j10

Path: S:\PROJECT FOLDER\20300 Bicester\Calculations\Transport\Junctions 10

Report generation date: 12/08/2022 10:25:34

»Linked - Strategic Model Base + Committed, AM »Linked - Strategic Model Base + Committed, PM

Summary of junction performance

		AM		PM				
	Queue (PCU)	Delay (s)	RFC	Queue (PCU)	Delay (s)	RFC		
	Linked [Lane	Simulation] - Stra	rategic Model Base + Committed				
Junction 1 - Arm A	1.1	14.74		0.7	11.40			
Junction 1 - Arm B	184.6	622.83		193.5	638.13			
Junction 1 - Arm C	1.2	4.68		1.2	4.50			
Junction 2 - Arm A	0.2	0.68		0.1	0.47			
Junction 2 - Arm B	53.4	291.99		232.6	972.07			
Junction 2 - Arm C	3.1	16.51		3.0	14.67			

There are warnings associated with one or more model runs - see the 'Data Errors and Warnings' tables for each Analysis or Demand Set.

Values shown are the highest values encountered over all time segments. Delay is the maximum value of average delay per arriving vehicle. Arm and junction delays are averages for all movements, including movements with zero delay.

File summary

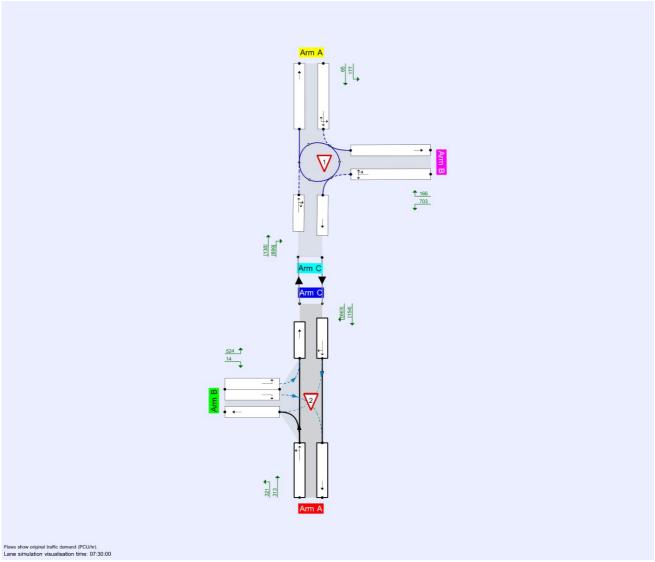
File Description

Title	A4095 / Bucknell Road Junction Network (Existing)
Location	Bicester, Oxfordshire
Site number	
Date	11/02/2022
Version	
Status	
Identifier	
Client	Hallam Land Management Ltd
Jobnumber	20300
Enumerator	JUBB\MattDavies
Description	

Units

ı	Distance units	Speed units	Traffic units input	Traffic units results	Flow units	Average delay units	Total delay units	Rate of delay units
ı	m	kph	PCU	PCU	perHour	s	-Min	perMin





The junction diagram reflects the last run of Junctions.

Analysis Options

Vehicle length (m)	Calculate Queue Percentiles	Calculate detailed queueing delay	Show lane queues in feet / metres	Show all PICADY stream intercepts	Calculate residual capacity	RFC Threshold	Average Delay threshold (s)	Queue threshold (PCU)	Use iterations with HCM roundabouts	Max number of iterations for roundabouts
5.75						0.85	36.00	20.00		500

Lane Simulation options

Criteria type	Stop criteria (%)	Stop criteria time (s)	Stop criteria number of trials	Random seed	Results refresh speed (s)	Individual vehicle animation number of trials	Average animation capture interval (s)	Use quick response	Do flow sampling	Suppress automatic lane creation	Last run random seed	Last run number of trials	Last run time taken (s)
Delay	1.00	100000	100000	-1	3	1	60	✓			1916309826	190	16.12

Demand Set Summary

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)	Run automatically
D1	Strategic Model Base + Committed	AM	FLAT	07:30	08:30	60	15	✓
D2	Strategic Model Base + Committed	PM	FLAT	16:15	17:15	60	15	✓



Analysis Set Details

ID	Name	Use Lane Simulation	Include in report	Network flow scaling factor (%)	Network capacity scaling factor (%)
A2	Linked	✓	✓	100.000	100.000



Linked - Strategic Model Base + Committed, AM

Data Errors and Warnings

Severity	Area Item		Description
Last Run	Lane Simulation	Junction 2 - Arm B - Lane Simulation	Arm B: Queue at end of modelled period is greater than 10 PCU. Delay is likely to have been underestimated.
Last Run	Lane Simulation	Junction 1 - Arm B - Lane Simulation	Arm B: Queue at end of modelled period is greater than 10 PCU. Delay is likely to have been underestimated.
Info	Lane Simulation	A2 - Linked [Lane Simulation]	This analysis set uses Lane Simulation mode. For detailed information on this mode, please see the User Guide.

Junction Network

Junctions

Junction	Name	Junction type	Arm A Direction	Arm B Direction	Arm C Direction	Use circulating lanes	Arm order	Junction Delay (s)	Junction LOS
1	Bucknell Road / A4095 Roundabout	Standard Roundabout					A, B, C	288.72	F
2	Bucknell Road / A4095 Priority T-Junction	T-Junction	Two-way	Two-way	Two-way			93.00	F

Junction Network

Driving side	Lighting	Network delay (s)	Network LOS	
Left	Normal/unknown	193.97	F	

Arms

Arms

Junction	Arm	Name	Description	No give-way line	Arm type
	Α	Bucknell Road (North)			
1	В	A4095 (East)			
	С	Bucknell Road (South)			
	Α	Bucknell Road (South)			Major
2	В	A4095 Howes Lane (West)			Minor
	C	Bucknell Road (North)			Major

Roundabout Geometry

Junction	Arm	V - Approach road half-width (m)	E - Entry width (m)	l' - Effective flare length (m)	R - Entry radius (m)	D - Inscribed circle diameter (m)	PHI - Conflict (entry) angle (deg)	Entry only	Exit only
	Α	2.97	9.00	9.2	67.9	30.0	25.0		
1	В	3.70	6.00	12.5	17.6	30.0	11.0		
	С	3.50	6.80	5.7	30.7	30.0	48.0		

Major Arm Geometry

Junc	tion	Arm	Width of carriageway (m)	Has kerbed central reserve	Has right-turn storage	Visibility for right turn (m)	Blocks?	Blocking queue (PCU)	Vehicles causing blocking (%)
2	2	С	7.10			250.0	✓	0.00	100

Geometries for Arm C are measured opposite Arm B. Geometries for Arm A (if relevant) are measured opposite Arm D.

Minor Arm Geometry

Junction	Arm	Minor arm type	Lane Width (Left) (m)	Vidth (Left) (m) Lane Width (Right) (m)		Visibility to right (m)	
2	В	Two lanes	3.76	3.60	27	38	



Slope / Intercept / Capacity

Roundabout Slope and Intercept used in model

Junction	Arm	Final slope	Final intercept (PCU/hr)		
	Α	0.646	1565		
1	В	0.666	1652		
	С	0.572	1347		

The slope and intercept shown above include any corrections and adjustments.

Priority Intersection Slopes and Intercepts

Junction	Stream	Intercept (PCU/hr)	Slope for A-B	Slope for A-C	Slope for C-A	Slope for C-B
	B-A	535	0.091	0.229	0.144	0.327
2	B-C	697	0.104	0.263	-	-
	С-В	719	0.265	0.265	-	-

The slopes and intercepts shown above include custom intercept adjustments only.

Lane Simulation: Arm options

Junction	Arm	Lane capacity source	Traffic considering secondary lanes (%)		
	Α	Evenly split	10.00		
1	В	Evenly split	10.00		
	С	Evenly split	10.00		
	Α		10.00		
2	В		10.00		
	O		10.00		

Lanes

Lailes													
Junction	Arm	Side	Lane level	Lane	Destination arms	Has limited storage	Storage (PCU)	Has bottleneck	Has obstruction	Minimum capacity (PCU/hr)	Maximum capacity (PCU/hr)	Signalised	
	Α	Entry	1	1	A, B, C		Infinity			0	99999		
	A	Exit	1	1			Infinity						
1	В	Entry	1	1	A, B, C		Infinity			0	99999		
!	В	Exit	1	1			Infinity						
	С	Entry	1	1	A, B, C	✓	2.90			0	99999		
	١	Exit	1	1		✓	3.20						
		Entry	1	1	B, C		Infinity			0	99999		
	Α	Exit	1	1			Infinity						
		F4	4	1	С		Infinity			0	99999		
2	В	Entry	1	2	Α		Infinity			0	99999		
		Exit	1	1			Infinity						
	С	Entry	1	1	A, B	✓	3.20			0	99999		
	٦	Exit	1	1		✓	2.90						

Entry Lane slope and intercept

Junction	Arm	Side	Lane level	Lane	Final slope	Final intercept (PCU/hr)
	Α	Entry	1	1	0.646	1565
1	В	Entry	1	1	0.666	1652
	С	Entry	1	1	0.572	1347

Streams may be combined, in which case capacity will be adjusted.

Values are shown for the first time segment only; they may differ for subsequent time segments.



Summary of Entry Lane allowed movements

Junction	Arm	Arm Lane Level		Destination arm		
	L	Level		Α	В	С
	Α	1	1		✓	✓
2	В	4	1			✓
2	В	'	2	✓		
	С	1	1	✓	✓	

Summary of Entry Lane allowed movements

Junction	ction Arm Lane Lane		Des	stina arm	tion	
	Level		Α	В	С	
	Α	1	1	✓	✓	✓
1	В	1	1	✓	✓	✓
	С	1	1	✓	✓	✓

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)	Run automatically
D1	Strategic Model Base + Committed	AM	FLAT	07:30	08:30	60	15	✓

Vehicle mix varies over turn	Vehicle mix varies over entry	Vehicle mix source	PCU Factor for a HV (PCU)
√	✓	HV Percentages	2.00

Linked Arm Data

Junction	Arm	Feeding Junction	Feeding Arm	Link Type	Flow source	Uniform flow (PCU/hr)	Flow multiplier (%)	Internal storage space (PCU)
1	С	2	С	Queue limited	Normal	0	100.00	10.00
2	С	1	С	Queue limited	Normal	0	100.00	10.00

Demand overview (Traffic)

Junction	Arm	Linked arm	Profile type	Use O-D data	Average Demand (PCU/hr)	Scaling Factor (%)
	Α		FLAT	✓	242	100.000
1	В		FLAT	✓	869	100.000
	С	✓				
	Α		FLAT	✓	634	100.000
2	В		FLAT	✓	538	100.000
	С	✓				

Origin-Destination Data

Demand (PCU/hr)

Junction 2

		•		,			
		То					
			Α	В	С		
		Α	0	321	313		
	From	В	14	0	524		
		С	154	663	0		

Proportions

	То					
		Α	В	С		
	Α	0.00	0.51	0.49		
From	В	0.03	0.00	0.97		
	O	0.19	0.81	0.00		



Demand (PCU/hr)

Junction 1

	То					
		Α	В	С		
F	Α	0	177	65		
From	В	166	0	703		
	O	138	699	0		

Proportions

	То					
From		Α	В	С		
	Α	0.00	0.73	0.27		
	В	0.19	0.00	0.81		
	C	0.16	0.84	0.00		

Vehicle Mix

Heavy Vehicle Percentages

Junction 2

	То				
		Α	В	С	
	Α	0	2	0	
From	В	0	0	20	
	O	0	18	2	

Average PCU Per Veh

	То						
		Α	В	С			
From	Α	1.000	1.020	1.000			
	В	1.000	1.000	1.200			
	С	1.000	1.180	1.020			

Heavy Vehicle Percentages

Junction 1

		T	·o	
		Α	В	C
	Α	0	10	5
From	В	0	0	14
	O	11	12	0

Average PCU Per Veh

			То	
		Α	В	С
	Α	1.000	1.100	1.050
From	В	1.000	1.000	1.140
	С	1.110	1.120	1.000

Detailed Demand Data

Demand for each time segment

Time Segment	Junction	Arm	Demand (PCU/hr)	Demand in PCU (PCU/hr)					
		Α	242	242					
	1	В	869	869					
07:30-07:45		С	837	837					
07:30-07:45		Α	634	634					
	2	В	538	538					
		C	817	817					
		Α	242	242					
	1	В	869	869					
07:45-08:00		C	837	837					
07.43-00.00		Α	634	634					
	2	В	538	538					
		С	817	817					
		Α	242	242					
	1	В	869	869					
08:00-08:15		С	837	837					
00.00-00.13	2	Α	634	634					
		В	538	538					
		С	817	817					
	1	1	1	1	1		Α	242	242
						В	869	869	
08:15-08:30		С	837	837					
		Α	634	634					
	2	В	538	538					
		C	817	817					



Results

Results Summary for whole modelled period

Junction	Arm	Max Delay (s)	Max Queue (PCU)	Max LOS	Average Demand (PCU/hr)	Total Junction Arrivals (PCU)
	Α	14.74	1.1	В	243	243
1	В	622.83	184.6	F	875	875
	С	4.68	1.2	А	795	795
	Α	0.68	0.2	А	638	638
2	В		53.4	F	535	535
	U	16.51	3.1	С	622	622

Main Results for each time segment

07:30 - 07:45

Junction	Arm	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Circulating flow (PCU/hr)	Throughput (PCU/hr)	Average throughput (PCU/hr)	Throughput (exit side) (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
	Α	252	63	666	251	242	254	0.0	1.1	12.657	В
1	В	876	219	66	686	682	850	0.0	47.1	131.186	F
	С	792	198	128	792	763	623	0.0	1.1	4.679	А
	Α	640	160		641	639	137	0.0	0.1	0.670	Α
2	В	533	133		480	461	832	0.0	19.6	95.782	F
	С	629	157		629	602	780	0.0	3.0	16.043	С

07:45 - 08:00

Junction	Arm	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Circulating flow (PCU/hr)	Throughput (PCU/hr)	Average throughput (PCU/hr)	Throughput (exit side) (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
	Α	246	61	656	248	242	262	1.1	0.8	13.513	В
1	В	877	219	63	679	693	841	47.1	89.8	358.179	F
	С	784	196	130	787	793	611	1.1	1.1	4.623	Α
	Α	636	159		636	640	130	0.1	0.1	0.683	Α
2	В	531	133		478	482	822	19.6	32.3	199.576	F
	С	616	154		617	630	780	3.0	3.0	16.457	С

08:00 - 08:15

Junction	Arm	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Circulating flow (PCU/hr)	Throughput (PCU/hr)	Average throughput (PCU/hr)	Throughput (exit side) (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
	Α	240	60	675	238	242	258	0.8	1.0	14.742	В
1	В	872	218	63	666	682	849	89.8	136.9	603.275	F
	С	806	201	126	807	802	605	1.1	1.0	4.581	А
	Α	644	161		645	641	131	0.1	0.1	0.630	А
2	В	539	135		502	496	832	32.3	44.0	281.870	F
	С	615	154		616	629	800	3.0	3.0	16.443	С



08:15 - 08:30

Junction	Arm	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Circulating flow (PCU/hr)	Throughput (PCU/hr)	Average throughput (PCU/hr)	Throughput (exit side) (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
	Α	235	59	669	232	243	258	1.0	1.1	13.768	В
1	В	877	219	59	689	681	842	136.9	184.6	622.829	F
	С	799	200	128	799	797	617	1.0	1.2	4.595	А
	Α	633	158		632	630	132	0.1	0.2	0.654	Α
2	В	538	134		494	497	827	44.0	53.4	291.991	F
	С	628	157		627	623	794	3.0	3.1	16.508	С

Lane Results

Lane Level notation: Lane Level 1 is always closest to the junction.

Lanes: Main Results for each time segment

07:30 - 07:45

Junction	Arm	Side	Lane level	Lane	Destination arms	Total Demand (PCU/hr)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Average throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
	Α	Entry	1	1	A, B, C	252	1134	0.222	251	242	0.0	1.1	12.657	В
	^	Exit	1	1		254			254	251	0.0	0.0	0.000	A
1	В	Entry	1	1	A, B, C	876	1608	0.545	686	682	0.0	47.1	131.186	F
,	В	Exit	1	1		850			850	819	0.0	0.0	0.000	А
	С	Entry	1	1	A, B, C	792	1274	0.622	792	763	0.0	1.1	4.679	А
	١	Exit	1	1		624			623	608	0.0	2.3	12.158	В
	Α	Entry	1	1	B, C	640			641	639	0.0	0.1	0.670	А
	A	Exit	1	1		137			137	130	0.0	0.0	0.000	А
		Entry	1	1	С	519			466	447	0.0	19.6	98.394	F
2	В	Entry	'	2	А	14			14	14	0.0	0.0	15.416	С
		Exit	1	1		832			832	810	0.0	0.0	0.000	А
	С	Entry	1	1	A, B	629			629	602	0.0	3.0	16.043	С
	ر	Exit	1	1		780			780	760	0.0	0.4	1.878	Α

07:45 - 08:00

Junction	Arm	Side	Lane level	Lane	Destination arms	Total Demand (PCU/hr)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Average throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
		Entry	1	1	A, B, C	246	1141	0.216	248	242	1.1	0.8	13.513	В
	Α	Exit	1	1		262			262	263	0.0	0.0	0.000	А
	В	Entry	1	1	A, B, C	877	1610	0.545	679	693	47.1	89.8	358.179	F
1	В	Exit	1	1		841			841	841	0.0	0.0	0.000	Α
	С	Entry	1	1	A, B, C	784	1273	0.616	787	793	1.1	1.1	4.623	Α
	١	Exit	1	1		612			611	625	2.3	2.3	13.251	В
		Entry	1	1	B, C	636			636	640	0.1	0.1	0.683	А
	Α	Exit	1	1		130			130	134	0.0	0.0	0.000	А
		Entr.	_	1	С	517			465	468	19.6	32.3	205.377	F
2	В	Entry	1	2	А	14			14	14	0.0	0.1	17.428	С
		Exit	1	1		822			822	830	0.0	0.0	0.000	Α
	С	Entry	1	1	A, B	616			617	630	3.0	3.0	16.457	С
	٦	Exit	1	1		779			780	788	0.4	0.4	1.901	А



08:00 - 08:15

Junction	Arm	Side	Lane level	Lane	Destination arms	Total Demand (PCU/hr)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Average throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
	Α	Entry	1	1	A, B, C	240	1129	0.213	238	242	0.8	1.0	14.742	В
	A	Exit	1	1		258			258	262	0.0	0.0	0.000	А
1	В	Entry	1	1	A, B, C	872	1610	0.541	666	682	89.8	136.9	603.275	F
	ь	Exit	1	1		849			849	846	0.0	0.0	0.000	А
	С	Entry	1	1	A, B, C	806	1275	0.632	807	802	1.1	1.0	4.581	А
)	Exit	1	1		603			605	619	2.3	2.2	13.557	В
	Α	Entry	1	1	B, C	644			645	641	0.1	0.1	0.630	А
	^	Exit	1	1		131			131	132	0.0	0.0	0.000	А
		Entry	1	1	С	525			487	482	32.3	43.9	290.399	F
2	В	Lilliy	'	2	Α	15			15	14	0.1	0.0	15.048	O
		Exit	1	1		832			832	837	0.0	0.0	0.000	A
	С	Entry	1	1	A, B	615			616	629	3.0	3.0	16.443	С
	١	Exit	1	1		800			800	796	0.4	0.3	1.854	А

08:15 - 08:30

Junction	Arm	Side	Lane level	Lane	Destination arms	Total Demand (PCU/hr)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Average throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
		Entry	1	1	A, B, C	235	1132	0.207	232	243	1.0	1.1	13.768	В
	Α	Exit	1	1		258			258	258	0.0	0.0	0.000	Α
	В	Entry	1	1	A, B, C	877	1612	0.544	689	681	136.9	184.6	622.829	F
1	В	Exit	1	1		842			842	847	0.0	0.0	0.000	Α
	С	Entry	1	1	A, B, C	799	1274	0.627	799	797	1.0	1.2	4.595	А
	١	Exit	1	1		620			617	614	2.2	2.5	13.479	В
		Entry	1	1	B, C	633			632	630	0.1	0.2	0.654	А
	Α	Exit	1	1		132			132	130	0.0	0.0	0.000	А
		F		1	С	523			479	484	43.9	53.4	303.872	F
2	В	Entry	1	2	А	15			15	14	0.0	0.1	15.713	С
		Exit	1	1		827			827	825	0.0	0.0	0.000	А
	С	Entry	1	1	A, B	628			627	623	3.0	3.1	16.508	С
	٦	Exit	1	1		795			794	795	0.3	0.5	1.825	А



Lane movements: Main Results for each time segment

07:30 - 07:45

Junction	Arm	Side	Lane level	Lane	To Arm	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Simulation max flow (PCU/hr)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Average throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
					Α	0	0	0	0	0.000	0	0	0.0	0.0	0.000	Α
	Α	Entry	1	1	В	185	46	1565	1137	0.162	184	177	0.0	0.6	9.912	Α
					С	67	17	1565	1136	0.059	66	65	0.0	0.5	19.742	С
					Α	161	40	1652	1607	0.100	128	129	0.0	8.6	127.488	F
1	В	Entry	1	1	В	0	0	0	0	0.000	0	0	0.0	0.0	0.000	А
					С	715	179	1652	1607	0.445	558	553	0.0	38.5	132.164	F
					Α	127	32	1347	1274	0.100	126	121	0.0	0.3	4.704	А
	С	Entry	1	1	В	665	166	1347	1274	0.522	666	642	0.0	8.0	4.674	А
					С	0	0	0	0	0.000	0	0	0.0	0.0	0.000	А
					Α	0	0	0	0	0.000	0	0	0.0	0.0	0.000	
	Α	Entry	1	1	В	326	81	-	-	-	326	324	0.0	0.0	0.472	Α
					С	315	79	1	1	-	315	315	0.0	0.1	0.868	А
					Α	0	0	0	0	0.000	0	0	0.0	0.0	0.000	Α
				1	В	0	0	0	0	0.000	0	0	0.0	0.0	0.000	
2	В	Entry	1		С	519	130	697	574	0.904	466	447	0.0	19.6	98.394	F
_			•		Α	14	4	527	246	0.058	14	14	0.0	0.0	15.416	С
				2	В	0	0	0	0	0.000	0	0	0.0	0.0	0.000	
					С	0	0	0	0	0.000	0	0	0.0	0.0	0.000	А
					Α	122	30	-	-	-	122	116	0.0	0.3	11.252	В
	С	Entry	1	1	В	508	127	719	548	0.926	506	486	0.0	2.7	17.381	С
					С	0	0	0	0	0.000	0	0	0.0	0.0	0.000	

07:45 - 08:00

Junction	Arm	Side	Lane level	Lane	To Arm	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Simulation max flow (PCU/hr)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Average throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
					Α	0	0	0	0	0.000	0	0	0.0	0.0	0.000	А
	Α	Entry	1	1	В	183	46	1565	1142	0.160	185	179	0.6	0.5	10.883	В
					С	63	16	1565	1144	0.055	63	63	0.5	0.3	20.628	С
					Α	163	41	1652	1610	0.101	130	132	8.6	16.5	356.293	F
1	В	Entry	1	1	В	0	0	0	0	0.000	0	0	0.0	0.0	0.000	Α
					С	714	179	1652	1610	0.444	549	562	38.5	73.3	358.678	F
					Α	131	33	1347	1273	0.103	131	132	0.3	0.2	4.534	Α
	С	Entry	1	1	В	653	163	1347	1273	0.513	656	662	0.8	0.9	4.640	А
					С	0	0	0	0	0.000	0	0	0.0	0.0	0.000	А
					Α	0	0	0	0	0.000	0	0	0.0	0.0	0.000	
	Α	Entry	1	1	В	322	80	-	-	-	321	321	0.0	0.1	0.454	А
					С	315	79	-	-	-	315	319	0.1	0.1	0.909	А
					Α	0	0	0	0	0.000	0	0	0.0	0.0	0.000	А
				1	В	0	0	0	0	0.000	0	0	0.0	0.0	0.000	
2	В	Entry	1		С	517	129	697	575	0.900	465	468	19.6	32.3	205.377	F
		Entry	'		Α	14	3	524	241	0.057	14	14	0.0	0.1	17.428	С
				2	В	0	0	0	0	0.000	0	0	0.0	0.0	0.000	
					С	0	0	0	0	0.000	0	0	0.0	0.0	0.000	Α
					Α	116	29	-	-	-	116	120	0.3	0.5	12.017	В
	С	Entry	1	1	В	500	125	719	550	0.910	501	509	2.7	2.5	17.697	С
					С	0	0	0	0	0.000	0	0	0.0	0.0	0.000	



08:00 - 08:15

Junction	Arm	Side	Lane level	Lane	To Arm	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Simulation max flow (PCU/hr)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Average throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
					Α	0	0	0	0	0.000	0	0	0.0	0.0	0.000	Α
	Α	Entry	1	1	В	175	44	1565	1129	0.155	174	178	0.5	0.5	11.691	В
					С	65	16	1565	1129	0.057	63	64	0.3	0.4	22.689	С
					Α	165	41	1652	1611	0.102	126	128	16.5	25.8	599.790	F
1	В	Entry	1	1	В	0	0	0	0	0.000	0	0	0.0	0.0	0.000	Α
					С	707	177	1652	1610	0.439	540	554	73.3	111.1	604.204	F
					Α	131	33	1347	1276	0.103	132	135	0.2	0.1	4.526	А
	С	Entry	1	1	В	674	169	1347	1275	0.529	675	668	0.9	0.9	4.592	А
					С	0	0	0	0	0.000	0	0	0.0	0.0	0.000	А
					Α	0	0	0	0	0.000	0	0	0.0	0.0	0.000	
	Α	Entry	1	1	В	331	83	-	-	-	332	327	0.1	0.0	0.428	А
					С	313	78	-	-	-	313	314	0.1	0.1	0.836	А
					Α	0	0	0	0	0.000	0	0	0.0	0.0	0.000	А
				1	В	0	0	0	0	0.000	0	0	0.0	0.0	0.000	
2					С	525	131	697	575	0.912	487	482	32.3	43.9	290.399	F
2	В	Entry	1		Α	15	4	530	244	0.061	15	14	0.1	0.0	15.048	С
				2	В	0	0	0	0	0.000	0	0	0.0	0.0	0.000	
					С	0	0	0	0	0.000	0	0	0.0	0.0	0.000	А
					Α	116	29	-	-	-	116	118	0.5	0.3	11.824	В
	С	Entry	1	1	В	499	125	719	547	0.912	500	511	2.5	2.6	17.705	С
					С	0	0	0	0	0.000	0	0	0.0	0.0	0.000	

08:15 - 08:30

Junction	Arm	Side	Lane level	Lane	To Arm	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Simulation max flow (PCU/hr)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Average throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
					Α	0	0	0	0	0.000	0	0	0.0	0.0	0.000	А
	Α	Entry	1	1	В	175	44	1565	1130	0.155	172	180	0.5	0.6	10.992	В
					С	60	15	1565	1131	0.053	59	62	0.4	0.5	21.421	С
					Α	168	42	1652	1611	0.104	128	129	25.8	35.1	619.130	F
1	В	Entry	1	1	В	0	0	0	0	0.000	0	0	0.0	0.0	0.000	Α
					С	709	177	1652	1612	0.440	560	553	111.1	149.5	623.784	F
					Α	130	32	1347	1275	0.102	129	129	0.1	0.2	4.622	Α
	С	Entry	1	1	В	669	167	1347	1274	0.525	669	667	0.9	1.0	4.590	Α
					С	0	0	0	0	0.000	0	0	0.0	0.0	0.000	Α
					Α	0	0	0	0	0.000	0	0	0.0	0.0	0.000	
	Α	Entry	1	1	В	317	79	ı	-	ı	317	318	0.0	0.1	0.442	Α
					С	316	79		-	1	316	312	0.1	0.1	0.865	Α
					Α	0	0	0	0	0.000	0	0	0.0	0.0	0.000	Α
				1	В	0	0	0	0	0.000	0	0	0.0	0.0	0.000	
2	В	Entry	1		С	523	131	697	572	0.915	479	484	43.9	53.4	303.872	F
2	В	Entry	'		Α	15	4	515	241	0.062	15	14	0.0	0.1	15.713	С
				2	В	0	0	0	0	0.000	0	0	0.0	0.0	0.000	
					С	0	0	0	0	0.000	0	0	0.0	0.0	0.000	Α
					Α	117	29	-	-	-	117	116	0.3	0.4	11.955	В
	С	Entry	1	1	В	511	128	719	551	0.929	510	506	2.6	2.7	17.754	С
					С	0	0	0	0	0.000	0	0	0.0	0.0	0.000	



Linked - Strategic Model Base + Committed, PM

Data Errors and Warnings

Severity	Area	Item	Description
Last Run	Lane Simulation	Junction 2 - Arm B - Lane Simulation	Arm B: Queue at end of modelled period is greater than 10 PCU. Delay is likely to have been underestimated.
Last Run	Lane Simulation	Junction 1 - Arm B - Lane Simulation	Arm B: Queue at end of modelled period is greater than 10 PCU. Delay is likely to have been underestimated.
Info	Lane Simulation	A2 - Linked [Lane Simulation]	This analysis set uses Lane Simulation mode. For detailed information on this mode, please see the User Guide.

Junction Network

Junctions

Junction	Name	Junction type	Arm A Direction	Arm B Direction	Arm C Direction	Use circulating lanes	Arm order	Junction Delay (s)	Junction LOS
1	Bucknell Road / A4095 Roundabout	Standard Roundabout					A, B, C	301.44	F
2	Bucknell Road / A4095 Priority T-Junction	T-Junction	Two-way	Two-way	Two-way			363.58	F

Junction Network

Driving side	Lighting	Network delay (s)	Network LOS
Left	Normal/unknown	332.77	F

Traffic Demand

Demand Set Details

ID	Scenario name	Time Period name	Traffic profile type	Start time (HH:mm)	Finish time (HH:mm)	Time period length (min)	Time segment length (min)	Run automatically
D2	Strategic Model Base + Committed	PM	FLAT	16:15	17:15	60	15	✓

Vehicle mix varies over turn	Vehicle mix varies over entry	Vehicle mix source	PCU Factor for a HV (PCU)
✓	✓	HV Percentages	2.00

Linked Arm Data

Junction	Arm	Feeding Junction	Feeding Arm	Link Type	Flow source	Uniform flow (PCU/hr)	Flow multiplier (%)	Internal storage space (PCU)
1	С	2	С	Queue limited	Normal	0	100.00	10.00
2	С	1	С	Queue limited	Normal	0	100.00	10.00

Demand overview (Traffic)

Junction	Arm	Linked arm	Profile type	Use O-D data	Average Demand (PCU/hr)	Scaling Factor (%)
	Α		FLAT	✓	219	100.000
1	В		FLAT	✓	924	100.000
	С	✓				
	Α		FLAT	✓	556	100.000
2	В		FLAT	✓	748	100.000
	С	✓				

Origin-Destination Data



Demand (PCU/hr)

Junction 2

		Т	·o	
		Α	В	С
F	Α	0	238	318
From	В	11	0	737
	U	272	619	0

Proportions

	То							
From		Α	В	С				
	Α	0.00	0.43	0.57				
	В	0.01	0.00	0.99				
	С	0.31	0.69	0.00				

Demand (PCU/hr)

Junction 1

	То						
From		Α	В	C			
	Α	0	158	61			
	В	94	0	830			
	С	139	916	0			

Proportions

	То								
		Α	В	O					
From	Α	0.00	0.72	0.28					
	В	0.10	0.00	0.90					
	С	0.13	0.87	0.00					

Vehicle Mix

Heavy Vehicle Percentages

Junction 2

	То						
		Α	В	С			
	Α	0	2	0			
From	В	17	0	8			
	С	0	5	0			

Average PCU Per Veh

		То							
		Α	В	С					
	Α	1.000	1.020	1.000					
From	В	1.170	1.000	1.080					
	С	1.000	1.050	1.000					

Heavy Vehicle Percentages

Junction 1

	То						
		Α	В	С			
	Α	0	10	5			
From	В	38	0	3			
	С	3	6	0			

Average PCU Per Veh

		То							
		Α	В	С					
	Α	1.000	1.100	1.050					
From	В	1.380	1.000	1.030					
	С	1.030	1.060	1.000					



Detailed Demand Data

Demand for each time segment

Time Segment	Junction	Arm	Demand (PCU/hr)	Demand in PCU (PCU/hr)
		Α	219	219
	1	В	924	924
16:15-16:30		С	1055	1055
16:15-16:30		Α	556	556
	2	В	748	748
		U	891	891
		Α	219	219
	1	В	924	924
16:30-16:45		U	1055	1055
16:30-16:45	2	Α	556	556
		В	748	748
		С	891	891
	1	Α	219	219
		В	924	924
16:45-17:00		С	1055	1055
16.45-17.00		Α	556	556
	2	В	748	748
		C	891	891
		Α	219	219
	1	В	924	924
17:00-17:15		С	1055	1055
17:00-17:15		Α	556	556
	2	В	748	748
		O	891	891

Results

Results Summary for whole modelled period

Junction	Arm	Max Delay (s)	Max Queue (PCU)	Max LOS	Average Demand (PCU/hr)	Total Junction Arrivals (PCU)
	A 11.40		0.7	В	221	221
1	В	638.13	193.5	F	926	926
	С	4.50	1.2	А	834	834
	Α	0.47	0.1	А	557	557
2	В	972.07	232.6	F	742	742
	С	14.67	3.0	В	715	715

Main Results for each time segment

16:15 - 16:30

Junction	Arm	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Circulating flow (PCU/hr)	Throughput (PCU/hr)	Average throughput (PCU/hr)	Throughput (exit side) (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
	Α	217	54	723	216	214	183	0.0	0.7	9.687	А
1	В	917	229	55	735	723	884	0.0	47.9	125.793	F
	С	833	208	72	834	821	713	0.0	1.0	4.472	Α
	Α	563	141		564	563	223	0.0	0.0	0.397	А
2	В	738	185		515	504	744	0.0	59.6	226.125	F
	С	714	179		712	687	823	0.0	3.0	14.255	В



16:30 - 16:45

Junction	Arm	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Circulating flow (PCU/hr)	Throughput (PCU/hr)	Average throughput (PCU/hr)	Throughput (exit side) (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
	Α	227	57	720	226	222	180	0.7	0.7	11.308	В
1	В	908	227	62	714	723	884	47.9	96.4	362.649	F
	С	830	208	71	829	829	707	1.0	1.2	4.501	Α
	Α	559	140		559	559	229	0.0	0.0	0.472	Α
2	В	741	185		512	517	727	59.6	118.9	637.069	F
	С	708	177		709	711	824	3.0	2.9	14.675	В

16:45 - 17:00

Junction	Arm	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Circulating flow (PCU/hr)	Throughput (PCU/hr)	Average throughput (PCU/hr)	Throughput (exit side) (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
	Α	228	57	730	227	222	183	0.7	0.6	11.404	В
1	В	932	233	63	720	724	894	96.4	145.8	609.247	F
	С	838	209	77	836	825	709	1.2	1.0	4.472	Α
	Α	556	139		557	547	231	0.0	0.0	0.431	Α
2	В	740	185		527	518	732	118.9	175.1	972.073	F
	С	713	178		714	717	835	2.9	2.9	14.483	В

17:00 - 17:15

Junction	Arm	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Circulating flow (PCU/hr)	Throughput (PCU/hr)	Average throughput (PCU/hr)	Throughput (exit side) (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
	Α	211	53	721	213	220	191	0.6	0.6	10.823	В
1	В	946	236	61	742	727	873	145.8	193.5	638.128	F
	С	835	209	78	834	838	726	1.0	1.1	4.382	Α
	Α	550	137		549	553	233	0.0	0.1	0.452	Α
2	В	749	187		528	522	742	175.1	232.6	760.308	F
	С	724	181		723	714	824	2.9	3.0	14.500	В

Lane Results

Lane Level notation: Lane Level 1 is always closest to the junction.

Lanes: Main Results for each time segment

16:15 - 16:30

Junction	Arm	Side	Lane level	Lane	Destination arms	Total Demand (PCU/hr)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Average throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
	A	Entry	1	1	A, B, C	217	1098	0.198	216	214	0.0	0.7	9.687	А
	^	Exit	1	1		183			183	181	0.0	0.0	0.000	А
1	В	Entry	1	1	A, B, C	917	1616	0.568	735	723	0.0	47.9	125.793	F
'		Exit	1	1		884			884	870	0.0	0.0	0.000	А
	С	Entry	1	1	A, B, C	833	1306	0.638	834	821	0.0	1.0	4.472	А
		Exit	1	1		717			713	698	0.0	2.3	10.296	В
	A	Entry	1	1	B, C	563			564	563	0.0	0.0	0.397	А
	^	Exit	1	1		223			223	219	0.0	0.0	0.000	А
		Entry	1	1	С	727			504	493	0.0	59.5	228.933	F
2	В	Entry	'	2	А	11			11	10	0.0	0.1	17.779	С
		Exit	1	1		744			744	719	0.0	0.0	0.000	А
	С	Entry	1	1	A, B	714			712	687	0.0	3.0	14.255	В
	"	Exit	1	1		822			823	815	0.0	0.2	1.554	А



16:30 - 16:45

Junction	Arm	Side	Lane level	Lane	Destination arms	Total Demand (PCU/hr)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Average throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
	А	Entry	1	1	A, B, C	227	1100	0.206	226	222	0.7	0.7	11.308	В
	A	Exit	1	1		180			180	183	0.0	0.0	0.000	А
	В	Entry	1	1	A, B, C	908	1610	0.564	714	723	47.9	96.4	362.649	F
'	B	Exit	1	1		884			884	880	0.0	0.0	0.000	А
	_	Entry	1	1	A, B, C	830	1307	0.635	829	829	1.0	1.2	4.501	А
	C E	Exit	1	1		705			707	710	2.3	2.2	11.428	В
		Entry	1	1	B, C	559			559	559	0.0	0.0	0.472	А
	A	Exit	1	1		229			229	226	0.0	0.0	0.000	А
		Entry	1	1	С	729			500	506	59.5	118.8	645.701	F
2	В	Entry	'	2	А	12			11	11	0.1	0.1	17.132	С
		Exit	1	1		727			727	737	0.0	0.0	0.000	А
	С	Entry	1	1	A, B	708			709	711	3.0	2.9	14.675	В
	٦	Exit	1	1		824			824	824	0.2	0.4	1.637	А

16:45 - 17:00

Junction	Arm	Side	Lane level	Lane	Destination arms	Total Demand (PCU/hr)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Average throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
		Entry	1	1	A, B, C	228	1093	0.209	227	222	0.7	0.6	11.404	В
	Α	Exit	1	1		183			183	176	0.0	0.0	0.000	Α
1	В	Entry	1	1	A, B, C	932	1610	0.579	720	724	96.4	145.8	609.247	F
'	ь	Exit	1	1		894			894	881	0.0	0.0	0.000	Α
	C	Entry	1	1	A, B, C	838	1304	0.643	836	825	1.2	1.0	4.472	Α
	C	Exit	1	1		707			709	715	2.2	2.0	11.419	В
	Α	Entry	1	1	B, C	556			557	547	0.0	0.0	0.431	Α
	A	Exit	1	1		231			231	232	0.0	0.0	0.000	Α
		Entry	1	1	С	730			518	507	118.8	175.0	987.008	F
2	В	Entry	'	2	А	10			10	12	0.1	0.1	21.300	С
		Exit	1	1		732			732	730	0.0	0.0	0.000	Α
	C	Entry	1	1	A, B	713			714	717	2.9	2.9	14.483	В
	C	Exit	1	1		835			835	821	0.4	0.3	1.619	Α

17:00 - 17:15

Junction	Arm	Side	Lane level	Lane	Destination arms	Total Demand (PCU/hr)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Average throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
		Entry	1	1	A, B, C	211	1099	0.192	213	220	0.6	0.6	10.823	В
	A	Exit	1	1		191			191	184	0.0	0.0	0.000	А
4	В	Entry	1	1	A, B, C	946	1612	0.587	742	727	145.8	193.5	638.128	F
1	В	Exit	1	1		873			873	887	0.0	0.0	0.000	А
	С	Entry	1	1	A, B, C	835	1303	0.641	834	838	1.0	1.1	4.382	А
	"	Exit	1	1		724		Color Colo	В					
	_	Entry	1	1	B, C	550			549	553	0.0	0.1	0.452	А
	A	Exit	1	1		233			233	229	0.0	0.0	0.000	А
		F4		1	С	739			516	511	175.0	232.6	847.459	F
2	В	Entry	1	2	А	11			11	11	0.1	0.0	15.715	С
		Exit	1	1		742			742	733	0.0	0.0	0.000	А
	С	Entry	1	1	A, B	724			723	714	2.9	3.0	14.500	В
	'	Exit	1	1		824			824	827	0.3	0.3	1.557	А



Lane movements: Main Results for each time segment

16:15 - 16:30

Junction	Arm	Side	Lane level	Lane	To Arm	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Simulation max flow (PCU/hr)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Average throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
					Α	0	0	0	0	0.000	0	0	0.0	0.0	0.000	А
	Α	Entry	1	1	В	161	40	1565	1096	0.147	161	159	0.0	0.3	7.622	Α
					С	56	14	1565	1092	0.051	55	56	0.0	0.3	15.194	С
					Α	92	23	1652	1614	0.057	72	71	0.0	4.7	121.911	F
1	В	Entry	1	1	В	0	0	0	0	0.000	0	0	0.0	0.0	0.000	А
					С	825	206	1652	1615	0.510	662	652	0.0	43.2	126.113	F
					Α	110	28	1347	1307	0.084	111	110	0.0	0.1	4.403	А
	С	Entry	1	1	В	722	181	1347	1306	0.553	723	711	0.0	0.8	4.483	А
					С	0	0	0	0	0.000	0	0	0.0	0.0	0.000	А
					Α	0	0	0	0	0.000	0	0	0.0	0.0	0.000	
	Α	Entry	1	1	В	245	61	-	-	-	245	240	0.0	0.0	0.240	А
					С	318	80	-	-	-	319	323	0.0	0.0	0.512	А
					Α	0	0	0	0	0.000	0	0	0.0	0.0	0.000	А
				1	В	0	0	0	0	0.000	0	0	0.0	0.0	0.000	
2	В	Entry	1		С	727	182	697	579	1.256	504	493	0.0	59.5	228.933	F
_	"	Liiii y			A	11	3	472	224	0.049	11	10	0.0	0.1	17.779	С
				2	В	0	0	0	0	0.000	0	0	0.0	0.0	0.000	
					С	0	0	0	0	0.000	0	0	0.0	0.0	0.000	А
					Α	212	53	-	-	-	213	208	0.0	0.7	10.817	В
	С	Entry	1	1	В	502	125	719	569	0.882	499	479	0.0	2.3	15.815	С
					С	0	0	0	0	0.000	0	0	0.0	0.0	0.000	

16:30 - 16:45

Junction	Arm	Side	Lane level	Lane	To Arm	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Simulation max flow (PCU/hr)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Average throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
					Α	0	0	0	0	0.000	0	0	0.0	0.0	0.000	Α
	Α	Entry	1	1	В	166	41	1565	1103	0.150	164	161	0.3	0.5	8.925	А
					С	61	15	1565	1103	0.056	62	61	0.3	0.2	17.395	С
					Α	83	21	1652	1611	0.052	71	74	4.7	9.1	350.243	F
1	В	Entry	1	1	В	0	0	0	0	0.000	0	0	0.0	0.0	0.000	А
					С	825	206	1652	1610	0.512	643	649	43.2	87.3	363.701	F
					Α	109	27	1347	1304	0.083	109	110	0.1	0.1	4.520	А
	С	Entry	1	1	В	722	181	1347	1307	0.553	720	719	0.8	1.1	4.498	А
					С	0	0	0	0	0.000	0	0	0.0	0.0	0.000	А
					Α	0	0	0	0	0.000	0	0	0.0	0.0	0.000	
	Α	Entry	1	1	В	235	59	-	-	-	235	241	0.0	0.0	0.314	А
					С	324	81	-	-	-	324	319	0.0	0.0	0.590	А
					Α	0	0	0	0	0.000	0	0	0.0	0.0	0.000	А
				1	В	0	0	0	0	0.000	0	0	0.0	0.0	0.000	
2	В	Entry	1		С	729	182	697	580	1.257	500	506	59.5	118.8	645.701	F
2		Entry	'		Α	12	3	493	220	0.054	11	11	0.1	0.1	17.132	С
				2	В	0	0	0	0	0.000	0	0	0.0	0.0	0.000	
					С	0	0	0	0	0.000	0	0	0.0	0.0	0.000	А
					Α	217	54	-	-	-	218	215	0.7	0.6	11.567	В
	С	Entry	1	1	В	491	123	719	570	0.860	491	496	2.3	2.3	16.082	С
					С	0	0	0	0	0.000	0	0	0.0	0.0	0.000	



16:45 - 17:00

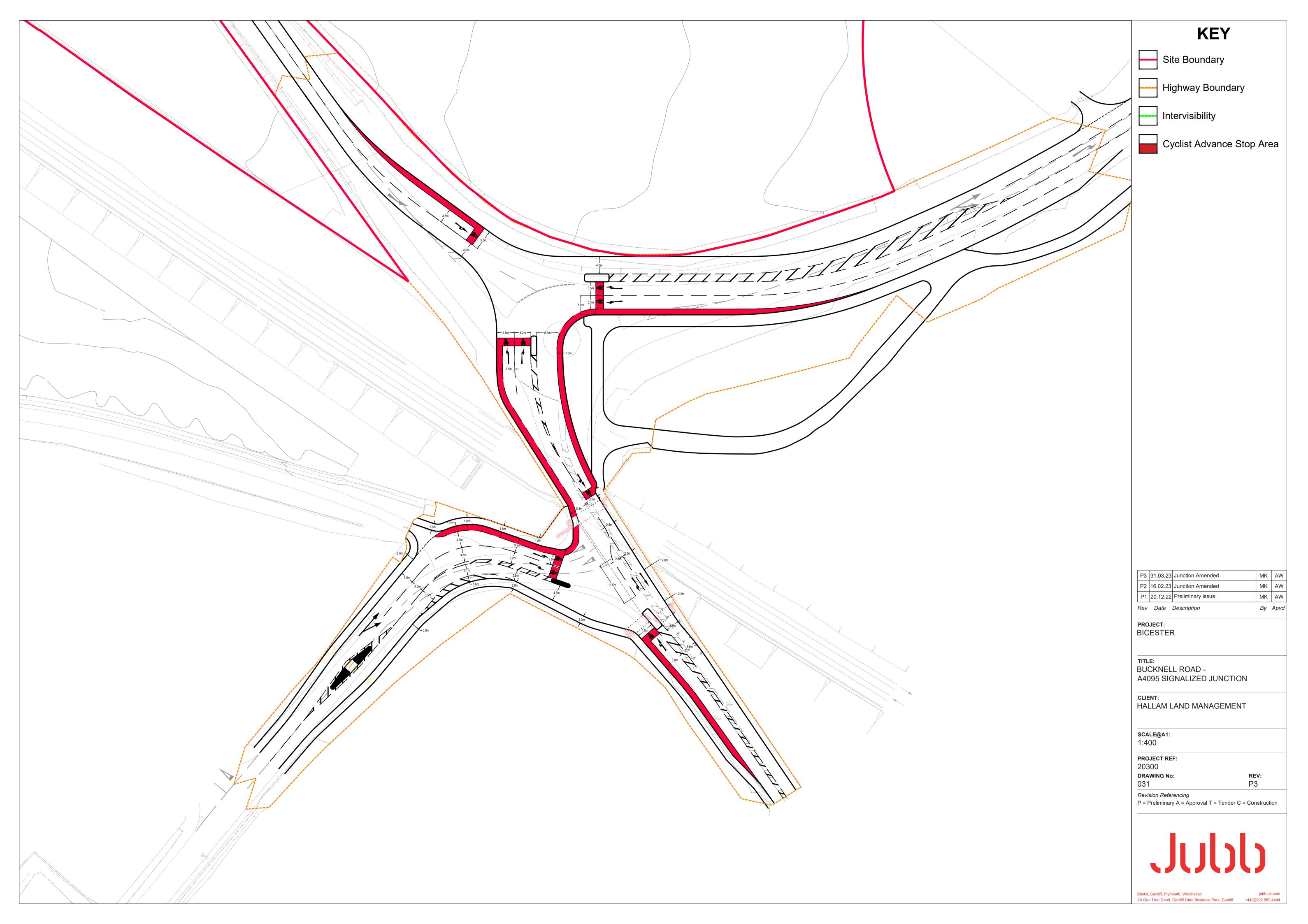
Junction	Arm	Side	Lane level	Lane	To Arm	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Simulation max flow (PCU/hr)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Average throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
					Α	0	0	0	0	0.000	0	0	0.0	0.0	0.000	А
	Α	Entry	1	1	В	166	41	1565	1093	0.151	164	160	0.5	0.4	8.622	Α
					С	63	16	1565	1095	0.058	63	62	0.2	0.2	18.233	С
					Α	99	25	1652	1609	0.061	77	72	9.1	14.9	612.775	F
1	В	Entry	1	1	В	0	0	0	0	0.000	0	0	0.0	0.0	0.000	А
					С	833	208	1652	1610	0.517	644	653	87.3	130.9	608.946	F
					Α	107	27	1347	1303	0.082	107	104	0.1	0.2	4.304	А
	С	Entry	1	1	В	731	183	1347	1304	0.560	730	721	1.1	0.9	4.497	А
					С	0	0	0	0	0.000	0	0	0.0	0.0	0.000	А
					A	0	0	0	0	0.000	0	0	0.0	0.0	0.000	
	Α	Entry	1	1	В	240	60	-	-	-	240	233	0.0	0.0	0.220	А
					С	317	79	-	-	-	317	314	0.0	0.0	0.583	А
					Α_	0	0	0	0	0.000	0	0	0.0	0.0	0.000	А
				1	В	0	0	0	0	0.000	0	0	0.0	0.0	0.000	
2	В	Entry	1		С	730	182	697	577	1.264	518	507	118.8	175.0	987.008	F
_	"	Lilliy			Α_	10	3	472	218	0.047	10	12	0.1	0.1	21.300	С
				2	В	0	0	0	0	0.000	0	0	0.0	0.0	0.000	
					С	0	0	0	0	0.000	0	0	0.0	0.0	0.000	Α
					Α	221	55	-	-	-	222	220	0.6	0.7	11.318	В
	С	Entry	1	1	В	492	123	719	571	0.861	492	497	2.3	2.3	15.960	С
					С	0	0	0	0	0.000	0	0	0.0	0.0	0.000	

17:00 - 17:15

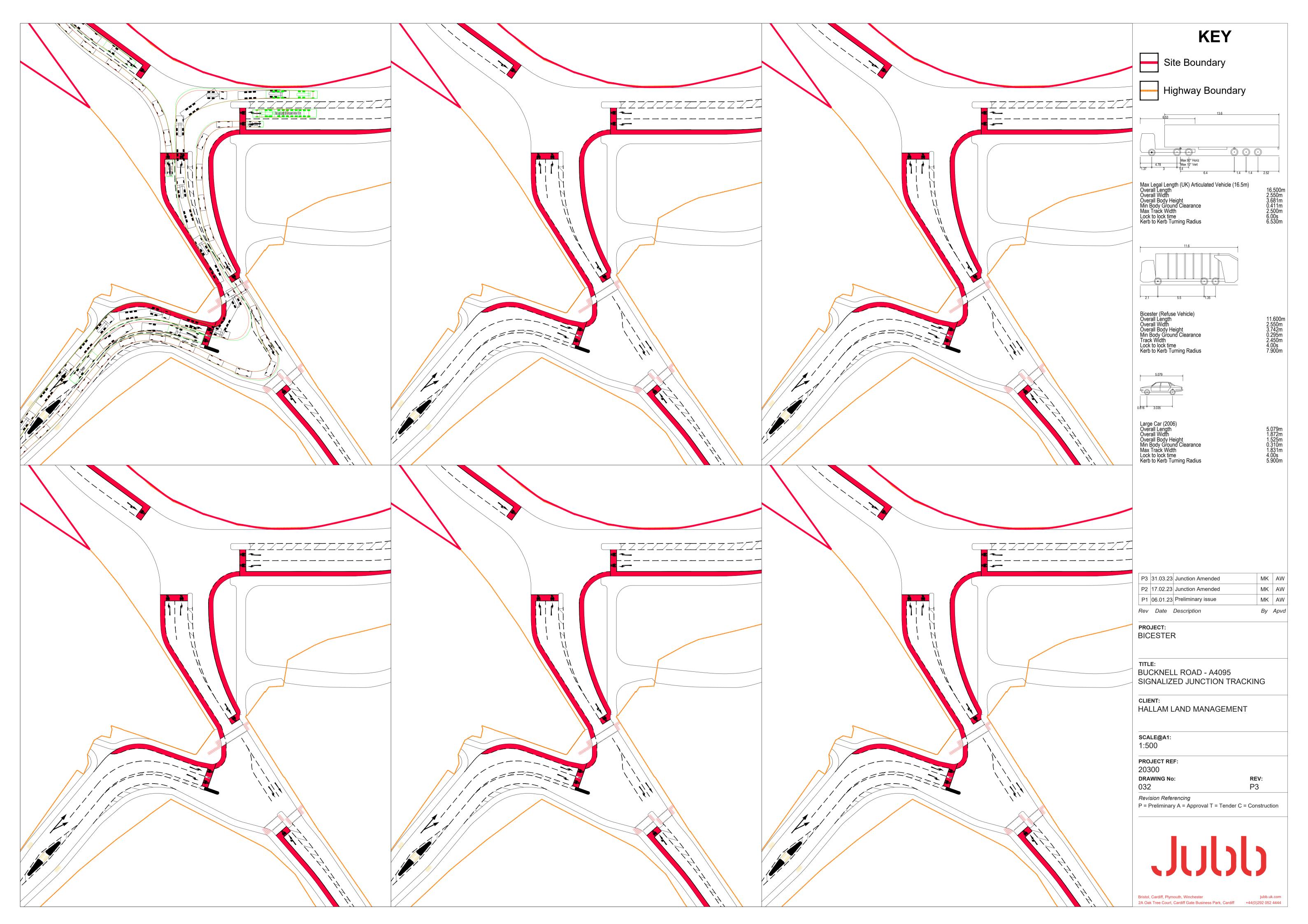
Junction	Arm	Side	Lane level	Lane	To Arm	Total Demand (PCU/hr)	Junction Arrivals (PCU)	Simulation max flow (PCU/hr)	Capacity (PCU/hr)	RFC	Throughput (PCU/hr)	Average throughput (PCU/hr)	Start queue (PCU)	End queue (PCU)	Delay (s)	Unsignalised level of service
					Α	0	0	0	0	0.000	0	0	0.0	0.0	0.000	А
	Α	Entry	1	1	В	152	38	1565	1098	0.139	152	159	0.4	0.4	8.413	А
					С	59	15	1565	1099	0.054	61	60	0.2	0.2	16.920	С
					Α	95	24	1652	1611	0.059	78	74	14.9	18.9	633.211	F
1	В	Entry	1	1	В	0	0	0	0	0.000	0	0	0.0	0.0	0.000	А
					С	851	213	1652	1611	0.528	664	653	130.9	174.7	638.517	F
					Α	112	28	1347	1304	0.086	112	110	0.2	0.1	4.369	Α
	С	Entry	1	1	В	723	181	1347	1303	0.555	721	728	0.9	0.9	4.384	А
					С	0	0	0	0	0.000	0	0	0.0	0.0	0.000	А
					Α	0	0	0	0	0.000	0	0	0.0	0.0	0.000	
	A	Entry	1	1	В	241	60	-	-	-	241	237	0.0	0.0	0.256	А
					С	308	77	-	-	-	308	316	0.0	0.0	0.596	А
					Α	0	0	0	0	0.000	0	0	0.0	0.0	0.000	А
				1	В	0	0	0	0	0.000	0	0	0.0	0.0	0.000	
2	В	Entry	1		С	739	185	697	588	1.256	516	511	175.0	232.6	847.459	F
_		Linkiy	'		Α	11	3	498	235	0.046	11	11	0.1	0.0	15.715	С
				2	В	0	0	0	0	0.000	0	0	0.0	0.0	0.000	
					С	0	0	0	0	0.000	0	0	0.0	0.0	0.000	А
					Α	222	55	-	-	-	221	218	0.7	0.6	11.393	В
	С	Entry	1	1	В	502	126	719	573	0.876	502	496	2.3	2.4	15.941	С
					С	0	0	0	0	0.000	0	0	0.0	0.0	0.000	

(

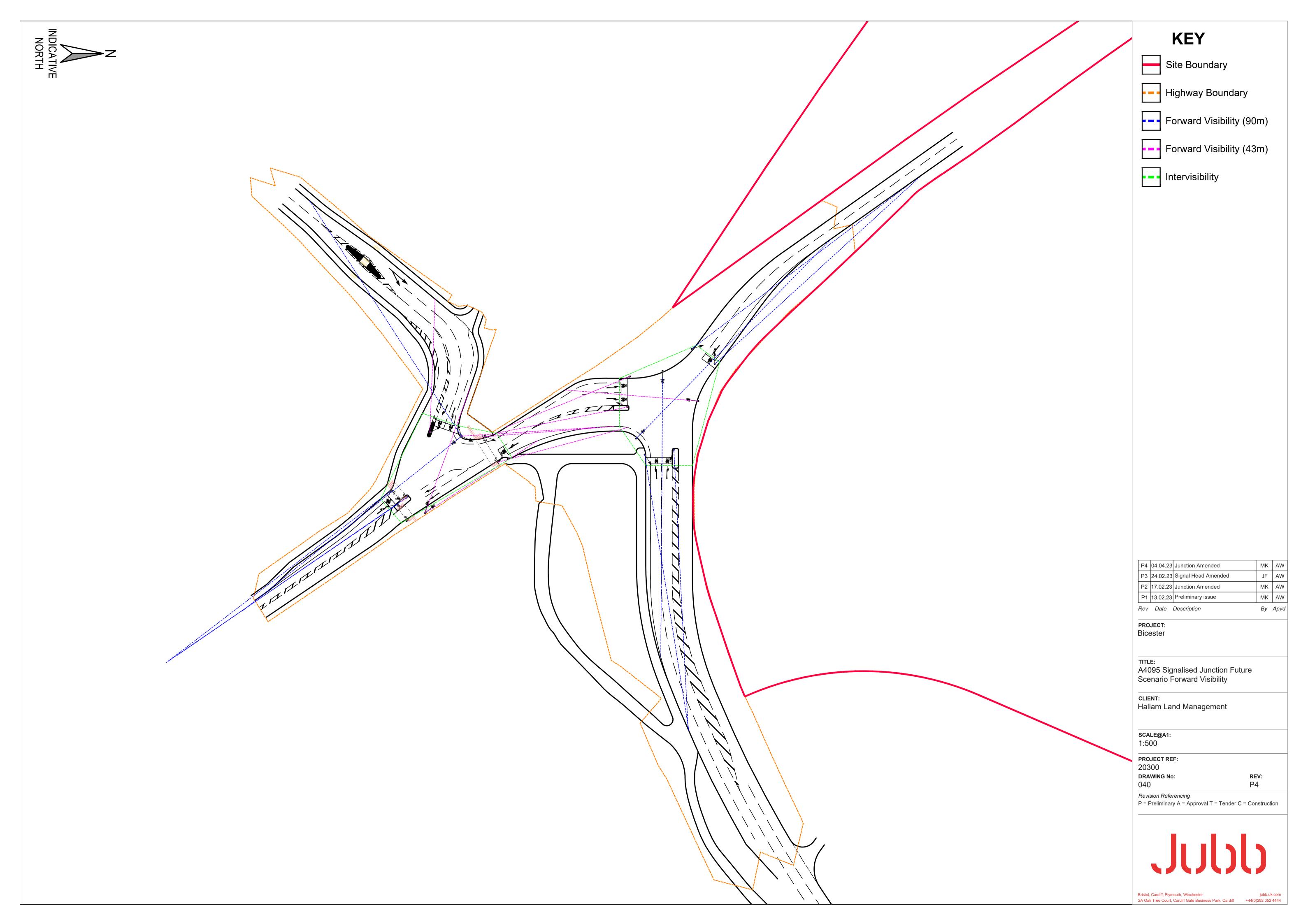
Appendix E Proposed Signalisation Interim Mitigation Scheme



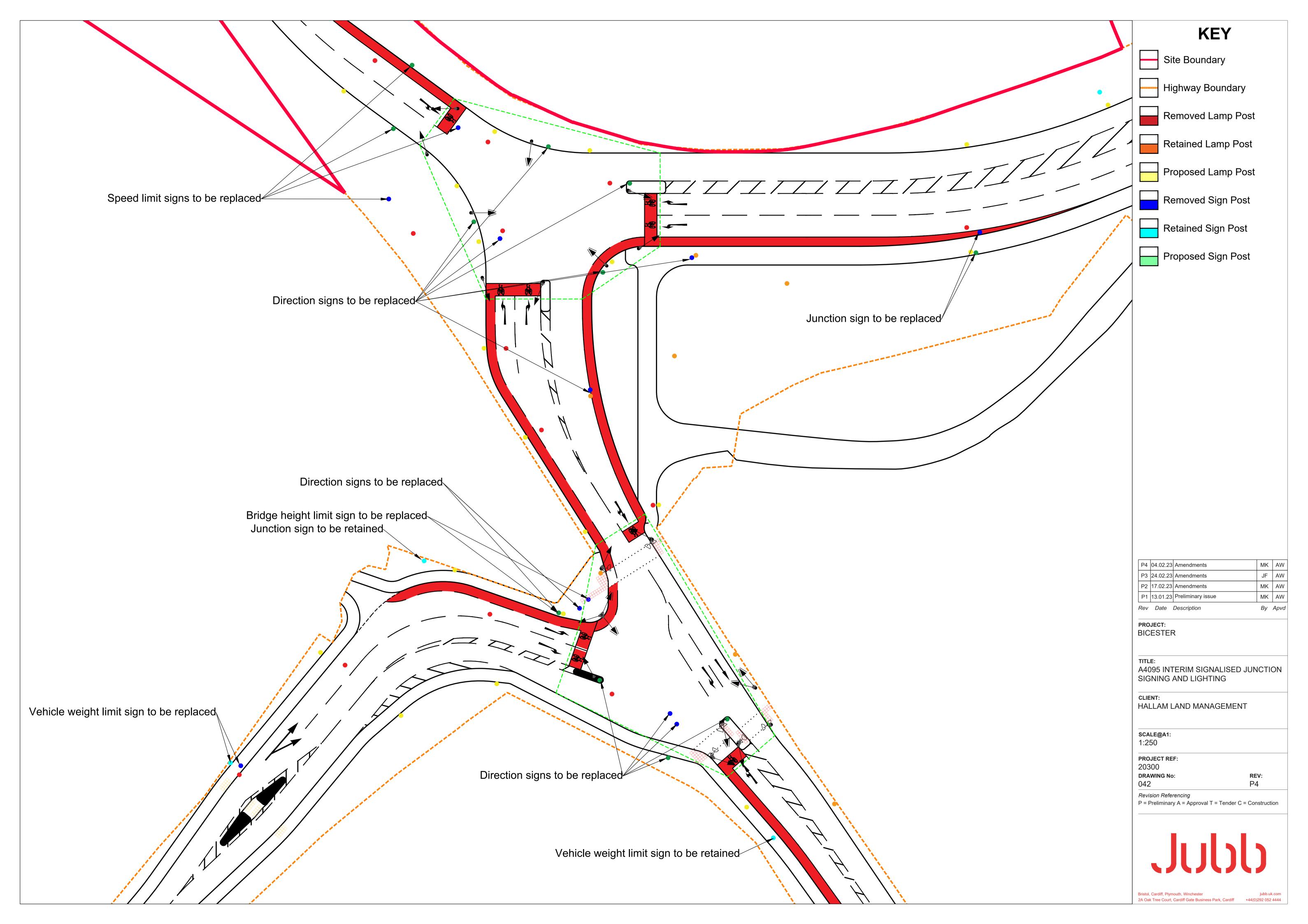
Appendix F Proposed Signalisation Interim Mitigation Scheme Vehicle Tracking



Appendix G Proposed Signalisation Interim Mitigation Scheme Visibility to Signal Heads



Appendix H Proposed Signalisation Interim Mitigation Scheme Signing and Street Lighting Positions

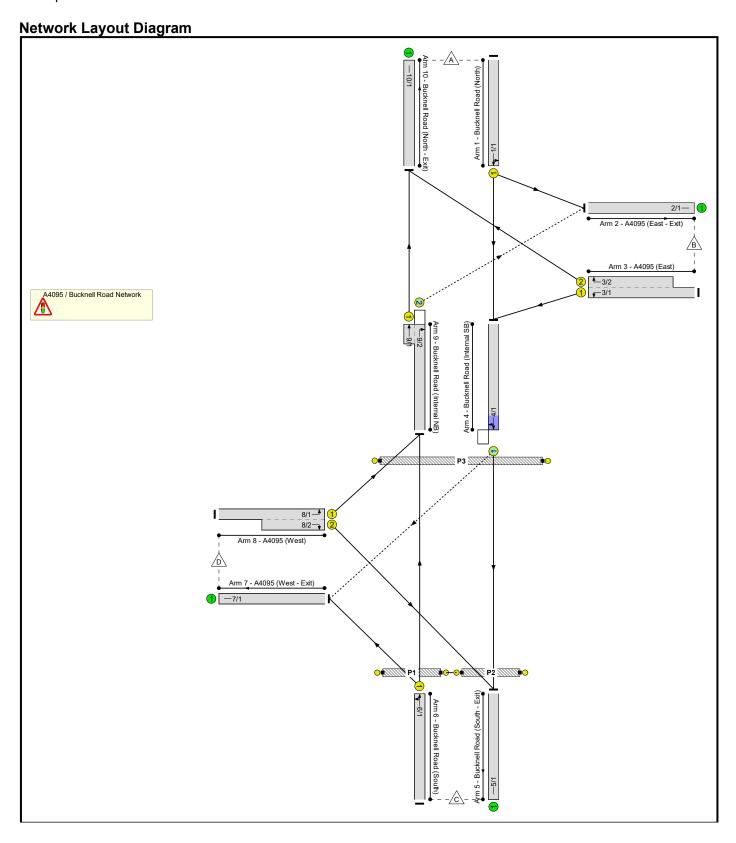


Appendix I LINSIG Output Report – Proposed Interim Junction 2026

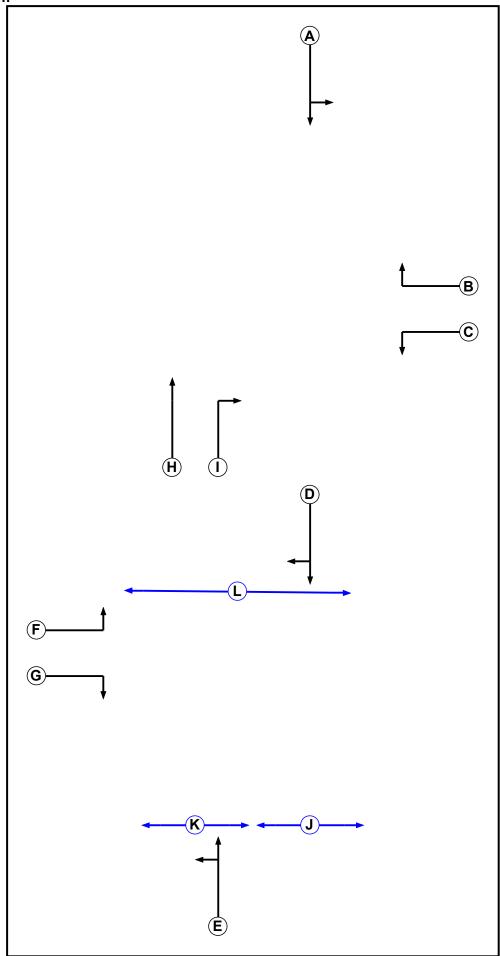
Full Input Data And Results Full Input Data And Results

User and Project Details

Project:	North West Bicester - Land North East of the Marylebone-Birmingham Railway Line
Title:	A4095 / Bucknell Road
Location:	Bicester, Oxfordshire
Client:	Hallam Land Management Ltd
Additional detail:	Based on Drawing No. 20300-031-P1.
File name:	A4095_Bucknell Road Junction Network (Pre-Link Road Layout)_30-03 ACSL with b scenario.lsg3x
Author:	MD / SR
Company:	Jubb
Address:	Suite B, Ground Floor West, St James Court, St James Parade, Bristol, BS1 3LH



Phase Diagram



Phase Input Data

Phase Name	Phase Type	Stage Stream	Assoc. Phase	Street Min	Cont Min
Α	Traffic	1		7	7
В	Traffic	1		7	7
С	Traffic	1		7	7
D	Traffic	2		7	7
Е	Traffic	2		7	7
F	Traffic	2		7	7
G	Traffic	2		7	7
Н	Traffic	1		7	7
I	Traffic	1		7	7
J	Pedestrian	2		5	5
K	Pedestrian	2		5	5
L	Pedestrian	2		5	5

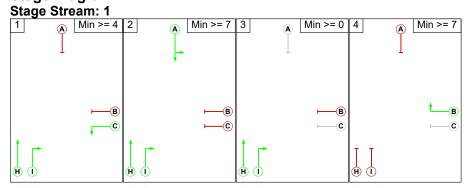
Phase Intergreens Matrix

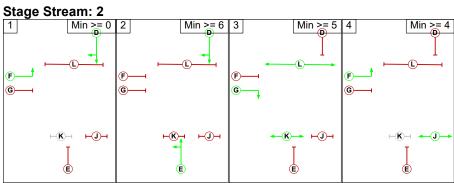
mase miter	<u>J</u>												
					Sta	artir	ıg F	has	se				
		Α	В	С	D	Е	F	G	Н	I	J	K	L
	Α		5	6	-	-	-	-	-	-	-	-	1
	В	5		-	-	-	-	-	5	5	-	-	-
	С	5	-		-	-	-	-	-	-	-	_	-
	D	-	-	-		-	-	5	-	-	8	-	5
	Е	_	-	-	-		7	5	-	-	-	5	8
Terminating Phase	F	_	_	-	_	5		-	-	-	-	_	6
	G	_	_	-	5	7	-		-	-	8	_	-
	Н	_	5	_	_	_	-	-		-	-	_	-
	ı	_	5	-	_	-	-	-	-		-	_	-
	J	_	-	_	6	-	-	6	-	-		-	-
	K	-	-	-	-	6	-	-	-	-	-		-
	L	_	-	-	9	9	9	-	-	-	-	-	

Phases in Stage

Stream	Stage No.	Phases in Stage
1	1	СНІ
1	2	АНІ
1	3	ні
1	4	В
2	1	DF
2	2	DE
2	3	GKL
2	4	FJ

Stage Diagram





Phase Delays

Stage Stream: 1

Term. Stage	Start Stage	Phase	Туре	Value	Cont value						
	There are no Phase Delays defined										

Stage Stream: 2

Term. Stage	Start Stage	Phase	Туре	Value	Cont value
	There are no	Phase D	elays d	efined	

Prohibited Stage Change

Stage Stream: 1

		То	Sta	age	
		1	2	3	4
	1		5	0	5
From Stage	2	6		0	5
)	3	2	2		5
	4	5	5	5	

Stage Stream: 2

tage on cann =									
	To Stage								
		1	2	3	4				
	1		5	6	8				
From Stage	2	7		8	8				
9-	3	9	9		9				
	4	6	6	6					

Full Input Data And Results **Give-Way Lane Input Data**

Junction: A4095 / Bucknell Road Network												
Lane	Movement	Max Flow when Giving Way (PCU/Hr)	Min Flow when Giving Way (PCU/Hr)	Opposing Lane	Opp. Lane Coeff.	Opp. Mvmnts.	Right Turn Storage (PCU)	Non-Blocking Storage (PCU)	DTE	Right Turn Move up (s)	Max Turns in Intergreen (PCU)	
4/1 (Bucknell Road (Internal SB))	7/1 (Right)	1439	0	6/1	1.09	All	2.00	2.00	0.50	2	2.00	
9/2 (Bucknell Road (Internal NB))	2/1 (Right)	1439	0	1/1	1.09	All	2.00	-	0.50	2	2.00	

Lane Input Data

Junction: A4		ucknell R	Road Ne	twork									
Lane	Lane Type	Phases	Start Disp.	End Disp.	Physical Length (PCU)	Sat Flow Type	Def User Saturation Flow (PCU/Hr)	Lane Width (m)	Gradient	Nearside Lane	Turns	Turning Radius (m)	
1/1 (Bucknell Road	U	А	2	3	60.0	Geom	-	3.80	0.00	Y	Arm 2 Left	35.00	
(North))											Arm 4 Ahead	29.00	
2/1 (A4095 (East - Exit))	U		2	3	60.0	Inf	-	-	-	-	-	-	
3/1 (A4095 (East))	U	С	2	3	60.0	Geom	-	3.50	0.00	Y	Arm 4 Left	12.00	
3/2 (A4095 (East))	U	В	2	3	22.3	Geom	-	3.50	0.00	Y	Arm 10 Right	37.00	
4/1 (Bucknell									0.00	0.00		Arm 5 Ahead	Inf
Road (Internal SB))	Ο	D	2	3	8.0	Geom	-	3.00	0.00	Y	Arm 7 Right	10.00	
5/1 (Bucknell Road (South - Exit))	U		2	3	60.0	Inf	-	-	-	-	-	-	
6/1 (Bucknell	U	E	2	3	60.0	Geom		3.20	0.00	Y	Arm 7 Left	30.00	
Road (South))	U		2	3	00.0	Geom		3.20	0.00	1	Arm 9 Ahead	Inf	
7/1 (A4095 (West - Exit))	U		2	3	60.0	Inf	-	-	-	-	-	-	
8/1 (A4095 (West))	U	F	2	3	60.0	Geom	-	3.75	0.00	Y	Arm 9 Left	7.00	
8/2 (A4095 (West))	U	G	2	3	8.9	Geom	-	3.20	0.00	Y	Arm 5 Right	25.00	
9/1 (Bucknell Road (Internal NB))	U	Н	2	3	2.8	Geom	-	4.40	0.00	Y	Arm 10 Ahead	27.00	
9/2 (Bucknell Road (Internal NB))	0	I	2	3	8.2	Geom	-	4.40	0.00	Y	Arm 2 Right	14.00	
10/1 (Bucknell Road (North - Exit))	U		2	3	60.0	Inf	-	-	-	-	-	-	

Traffic Flow Groups

Flow Group	Start Time	End Time	Duration	Formula
1: 'Strategic Model Hawkfield 2026 Development 1a - AM'	08:00	09:00	01:00	
2: 'Strategic Model Hawkfield 2026 Development 1a - PM'	17:00	18:00	01:00	
3: 'Strategic Model Hawkfield 2026 Development 2a - AM'	08:00	09:00	01:00	
4: 'Strategic Model Hawkfield 2026 Development 2a - PM'	17:00	18:00	01:00	
5: 'Strategic Model Hawkfield 2026 Development 1b - AM'	08:00	09:00	01:00	
6: 'Strategic Model Hawkfield 2026 Development 1b - PM'	17:00	18:00	01:00	
7: 'Strategic Model Hawkfield 2026 Development 2b - AM'	08:00	09:00	01:00	
8: 'Strategic Model Hawkfield 2026 Development 2b - PM'	17:00	18:00	01:00	

Scenario 1: 'Strategic Model Hawkfield Development 1a - AM' (FG1: 'Strategic Model Hawkfield 2026 Development 1a - AM', Plan 1: 'Network Control Plan 1')
Traffic Flows, Desired

Desired Flow:

	Destination										
		Α	В	С	D	Tot.					
	Α	0	163	13	55	231					
Origin	В	144	0	139	580	863					
Origin	С	56	236	0	325	617					
	D	91	379	13	0	483					
	Tot.	291	778	165	960	2194					

Traffic Lane Flows

raffic Lane Flows									
Lane	Scenario 1: Strategic Model Hawkfield Development 1a - AM								
Junction: A4095 /	Bucknell Road Network								
1/1	231								
2/1	778								
3/1 (with short)	863(In) 719(Out)								
3/2 (short)	144								
4/1	787								
5/1	165								
6/1	617								
7/1	960								
8/1 (with short)	483(In) 470(Out)								
8/2 (short)	13								
9/1 (short)	147								
9/2 (with short)	762(In) 615(Out)								
10/1	291								

Lane Saturation Flows

Junction: A4095 / Bucknell Roa	d Netwo	ork						
Lane	Lane Width (m)	Gradient	Nearside Lane	Allowed Turns	Turning Radius (m)	Turning Prop.	Sat Flow (PCU/Hr)	Flared Sat Flow (PCU/Hr)
1/1	2.00	0.00	Y	Arm 2 Left	35.00	70.6 %	1000	4000
(Bucknell Road (North))	3.80	0.00	Y	Arm 4 Ahead	29.00	29.4 %	1908	1908
2/1 (A4095 (East - Exit) Lane 1)			Infinite S	aturation Flow			Inf	Inf
3/1 (A4095 (East))	3.50	0.00	Υ	Arm 4 Left	12.00	100.0 %	1747	1747
3/2 (A4095 (East))	3.50	0.00	Υ	Arm 10 Right	37.00	100.0 %	1888	1888
4/1	2.00	0.00		Arm 5 Ahead	Inf	19.3 %	4700	1700
(Bucknell Road (Internal SB))	3.00	0.00	Y	Arm 7 Right	10.00	80.7 %	1708	1708
5/1 (Bucknell Road (South - Exit) Lane 1)		'	'	Inf	Inf			
6/1		0.00	.,	Arm 7 Left	30.00	52.7 %	1005	1885
(Bucknell Road (South))	3.20		Y	Arm 9 Ahead	Inf	47.3 %	1885	
7/1 (A4095 (West - Exit) Lane 1)			Infinite S	aturation Flow			Inf	Inf
8/1 (A4095 (West))	3.75	0.00	Υ	Arm 9 Left	7.00	100.0 %	1639	1639
8/2 (A4095 (West))	3.20	0.00	Y	Arm 5 Right	25.00	100.0 %	1825	1825
9/1 (Bucknell Road (Internal NB))	4.40	0.00	Y	Arm 10 Ahead	27.00	100.0 %	1947	1947
9/2 (Bucknell Road (Internal NB))	4.40	0.00	Y	Arm 2 Right	14.00	100.0 %	1856	1856
10/1 (Bucknell Road (North - Exit) Lane 1)		,	,	Inf	Inf			

Scenario 2: 'Strategic Model Hawkfield Development 1a - PM' (FG2: 'Strategic Model Hawkfield 2026 Development 1a - PM', Plan 1: 'Network Control Plan 1')
Traffic Flows, Desired

Desired Flow:

	Destination										
		Α	В	С	D	Tot.					
	Α	0	154	26	53	233					
Origin	В	84	0	261	544	889					
Origin	С	43	283	0	230	556					
	D	96	630	17	0	743					
	Tot.	223	1067	304	827	2421					

Traffic Lane Flows

raffic Lane Flows	
Lane	Scenario 2: Strategic Model Hawkfield Development 1a - PM
Junction: A4095 / Bucknell Road Network	
1/1	233
2/1	1067
3/1 (with short)	889(In) 805(Out)
3/2 (short)	84
4/1	884
5/1	304
6/1	556
7/1	827
8/1 (with short)	743(In) 726(Out)
8/2 (short)	17
9/1 (short)	139
9/2 (with short)	1052(In) 913(Out)
10/1	223

Lane Saturation Flows

Junction: A4095 / Bucknell Road Network								
Lane	Lane Width (m)	Gradient	Nearside Lane	Allowed Turns	Turning Radius (m)	Turning Prop.	Sat Flow (PCU/Hr)	Flared Sat Flow (PCU/Hr)
1/1	2.00	0.00	Y	Arm 2 Left	35.00	66.1 %	1000	1000
(Bucknell Road (North))	3.80	0.00	Y	Arm 4 Ahead	29.00	33.9 %	1908	1908
2/1 (A4095 (East - Exit) Lane 1)			Infinite S	aturation Flow			Inf	Inf
3/1 (A4095 (East))	3.50	0.00	Υ	Arm 4 Left	12.00	100.0 %	1747	1747
3/2 (A4095 (East))	3.50	0.00	Υ	Arm 10 Right	37.00	100.0 %	1888	1888
4/1	2.00	0.00	V	Arm 5 Ahead	Inf	32.5 %	1739	1739
(Bucknell Road (Internal SB))	3.00	0.00	Y	Arm 7 Right	10.00	67.5 %		1739
5/1 (Bucknell Road (South - Exit) Lane 1)		Infinite Saturation Flow					Inf	Inf
6/1			.,	Arm 7 Left	30.00	41.4 %	1896	4000
(Bucknell Road (South))	3.20	0.00	Y	Arm 9 Ahead	Inf	58.6 %		1896
7/1 (A4095 (West - Exit) Lane 1)			Infinite S	aturation Flow			Inf	Inf
8/1 (A4095 (West))	3.75	0.00	Υ	Arm 9 Left	7.00	100.0 %	1639	1639
8/2 (A4095 (West))	3.20	0.00	Y	Arm 5 Right	25.00	100.0 %	1825	1825
9/1 (Bucknell Road (Internal NB))	4.40	0.00	Y	Arm 10 Ahead	27.00	100.0 %	1947	1947
9/2 (Bucknell Road (Internal NB))	4.40	0.00	Υ	Arm 2 Right	14.00	100.0 %	1856	1856
10/1 (Bucknell Road (North - Exit) Lane 1)		Infinite Saturation Flow					Inf	Inf

Scenario 3: 'Strategic Model Hawkfield Development 2a - AM' (FG3: 'Strategic Model Hawkfield 2026 Development 2a - AM', Plan 1: 'Network Control Plan 1')
Traffic Flows, Desired

Desired Flow:

	Destination							
		Α	В	С	D	Tot.		
	Α	0	161	13	52	226		
Origin	В	152	0	149	582	883		
Oligili	С	56	236	0	326	618		
	D	90	378	13	0	481		
	Tot.	298	775	175	960	2208		

Traffic Lane Flows

raffic Lane Flows					
Lane	Scenario 3: Strategic Model Hawkfield Development 2a - AM				
Junction: A4095 /	Bucknell Road Network				
1/1	226				
2/1	775				
3/1 (with short)	883(In) 731(Out)				
3/2 (short)	152				
4/1	796				
5/1	175				
6/1	618				
7/1	960				
8/1 (with short)	481(ln) 468(Out)				
8/2 (short)	13				
9/1 (short)	146				
9/2 (with short)	760(In) 614(Out)				
10/1	298				

Lane Saturation Flows

Junction: A4095 / Bucknell Road Network								
Lane	Lane Width (m)	Gradient	Nearside Lane	Allowed Turns	Turning Radius (m)	Turning Prop.	Sat Flow (PCU/Hr)	Flared Sat Flow (PCU/Hr)
1/1	2.00	0.00	Y	Arm 2 Left	35.00	71.2 %	1000	1009
(Bucknell Road (North))	3.80	0.00	Y	Arm 4 Ahead	29.00	28.8 %	1908	1908
2/1 (A4095 (East - Exit) Lane 1)			Infinite S	aturation Flow			Inf	Inf
3/1 (A4095 (East))	3.50	0.00	Υ	Arm 4 Left	12.00	100.0 %	1747	1747
3/2 (A4095 (East))	3.50	0.00	Υ	Arm 10 Right	37.00	100.0 %	1888	1888
4/1	2.00	0.00	V	Arm 5 Ahead	Inf	20.4 %	1711	4744
(Bucknell Road (Internal SB))	3.00	0.00	Y	Arm 7 Right	10.00	79.6 %	1711	1711
5/1 (Bucknell Road (South - Exit) Lane 1)		Infinite Saturation Flow					Inf	Inf
6/1			.,	Arm 7 Left	30.00	52.8 %	1885	
(Bucknell Road (South))	3.20	0.00	Y	Arm 9 Ahead	Inf	47.2 %		1885
7/1 (A4095 (West - Exit) Lane 1)		,	Infinite S	aturation Flow			Inf	Inf
8/1 (A4095 (West))	3.75	0.00	Υ	Arm 9 Left	7.00	100.0 %	1639	1639
8/2 (A4095 (West))	3.20	0.00	Υ	Arm 5 Right	25.00	100.0 %	1825	1825
9/1 (Bucknell Road (Internal NB))	4.40	0.00	Υ	Arm 10 Ahead	27.00	100.0 %	1947	1947
9/2 (Bucknell Road (Internal NB))	4.40	0.00	Y	Arm 2 Right	14.00	100.0 %	1856	1856
10/1 (Bucknell Road (North - Exit) Lane 1)		Infinite Saturation Flow					Inf	Inf

Scenario 4: 'Strategic Model Hawkfield Development 2a - PM' (FG4: 'Strategic Model Hawkfield 2026 Development 2a - PM', Plan 1: 'Network Control Plan 1')
Traffic Flows, Desired

Desired Flow:

	P							
	Destination							
		Α	В	С	D	Tot.		
	Α	0	164	29	63	256		
Origin	В	85	0	252	544	881		
Origin	С	43	293	0	192	528		
	D	92	636	15	0	743		
	Tot.	220	1093	296	799	2408		

Traffic Lane Flows

rattic Lane Flows						
Lane	Scenario 4: Strategic Model Hawkfield Development 2a - PM					
Junction: A4095 /	Bucknell Road Network					
1/1	256					
2/1	1093					
3/1 (with short)	881(In) 796(Out)					
3/2 (short)	85					
4/1	888					
5/1	296					
6/1	528					
7/1	799					
8/1 (with short)	743(In) 728(Out)					
8/2 (short)	15					
9/1 (short)	135					
9/2 (with short)	1064(In) 929(Out)					
10/1	220					

Lane Saturation Flows

Junction: A4095 / Bucknell Road Network								
Lane	Lane Width (m)	Gradient	Nearside Lane	Allowed Turns	Turning Radius (m)	Turning Prop.	Sat Flow (PCU/Hr)	Flared Sat Flow (PCU/Hr)
1/1	2.00	0.00	Y	Arm 2 Left	35.00	64.1 %	1007	1007
(Bucknell Road (North))	3.80	0.00	Y	Arm 4 Ahead	29.00	35.9 %	1907	1907
2/1 (A4095 (East - Exit) Lane 1)			Infinite S	aturation Flow			Inf	Inf
3/1 (A4095 (East))	3.50	0.00	Y	Arm 4 Left	12.00	100.0 %	1747	1747
3/2 (A4095 (East))	3.50	0.00	Υ	Arm 10 Right	37.00	100.0 %	1888	1888
4/1	2.00	0.00	V	Arm 5 Ahead	Inf	31.6 %	- 1737	1737
(Bucknell Road (Internal SB))	3.00	0.00	Y	Arm 7 Right	10.00	68.4 %		1/3/
5/1 (Bucknell Road (South - Exit) Lane 1)		Infinite Saturation Flow					Inf	Inf
6/1			.,	Arm 7 Left	30.00	36.4 %	1900	4000
(Bucknell Road (South))	3.20	0.00	Y	Arm 9 Ahead	Inf	63.6 %		1900
7/1 (A4095 (West - Exit) Lane 1)			Infinite S	aturation Flow			Inf	Inf
8/1 (A4095 (West))	3.75	0.00	Υ	Arm 9 Left	7.00	100.0 %	1639	1639
8/2 (A4095 (West))	3.20	0.00	Y	Arm 5 Right	25.00	100.0 %	1825	1825
9/1 (Bucknell Road (Internal NB))	4.40	0.00	Y	Arm 10 Ahead	27.00	100.0 %	1947	1947
9/2 (Bucknell Road (Internal NB))	4.40	0.00	Υ	Arm 2 Right	14.00	100.0 %	1856	1856
10/1 (Bucknell Road (North - Exit) Lane 1)		Infinite Saturation Flow					Inf	Inf

Scenario 5: 'Strategic Model Hawkfield Development 1b - AM' (FG5: 'Strategic Model Hawkfield 2026 Development 1b - AM', Plan 1: 'Network Control Plan 1')
Traffic Flows, Desired

Desired Flow:

	Destination							
		Α	В	С	D	Tot.		
	Α	0	171	13	56	240		
Origin	В	143	0	128	571	842		
Origin	С	58	245	0	322	625		
	D	87	365	17	0	469		
	Tot.	288	781	158	949	2176		

Traffic Lane Flows

raffic Lane Flows						
Lane	Scenario 5: Strategic Model Hawkfield Development 1b - AM					
Junction: A4095 /	Bucknell Road Network					
1/1	240					
2/1	781					
3/1 (with short)	842(In) 699(Out)					
3/2 (short)	143					
4/1	768					
5/1	158					
6/1	625					
7/1	949					
8/1 (with short)	469(In) 452(Out)					
8/2 (short)	17					
9/1 (short)	145					
9/2 (with short)	755(In) 610(Out)					
10/1	288					

Lane Saturation Flows

Junction: A4095 / Bucknell Road Network								
Lane	Lane Width (m)	Gradient	Nearside Lane	Allowed Turns	Turning Radius (m)	Turning Prop.	Sat Flow (PCU/Hr)	Flared Sat Flow (PCU/Hr)
1/1	2.00	0.00	Y	Arm 2 Left	35.00	71.3 %	1000	1009
(Bucknell Road (North))	3.80	0.00	Y	Arm 4 Ahead	29.00	28.7 %	1908	1908
2/1 (A4095 (East - Exit) Lane 1)			Infinite S	aturation Flow			Inf	Inf
3/1 (A4095 (East))	3.50	0.00	Υ	Arm 4 Left	12.00	100.0 %	1747	1747
3/2 (A4095 (East))	3.50	0.00	Υ	Arm 10 Right	37.00	100.0 %	1888	1888
4/1	2.00	0.00	V	Arm 5 Ahead	Inf	18.4 %	1700	1706
(Bucknell Road (Internal SB))	3.00	0.00	Y	Arm 7 Right	10.00	81.6 %	1706	1706
5/1 (Bucknell Road (South - Exit) Lane 1)		Infinite Saturation Flow					Inf	Inf
6/1			.,	Arm 7 Left	30.00	51.5 %	1886	4000
(Bucknell Road (South))	3.20	0.00	Y	Arm 9 Ahead	Inf	48.5 %		1886
7/1 (A4095 (West - Exit) Lane 1)			Infinite S	aturation Flow			Inf	Inf
8/1 (A4095 (West))	3.75	0.00	Υ	Arm 9 Left	7.00	100.0 %	1639	1639
8/2 (A4095 (West))	3.20	0.00	Y	Arm 5 Right	25.00	100.0 %	1825	1825
9/1 (Bucknell Road (Internal NB))	4.40	0.00	Y	Arm 10 Ahead	27.00	100.0 %	1947	1947
9/2 (Bucknell Road (Internal NB))	4.40	0.00	Υ	Arm 2 Right	14.00	100.0 %	1856	1856
10/1 (Bucknell Road (North - Exit) Lane 1)		Infinite Saturation Flow					Inf	Inf

Scenario 6: 'Strategic Model Hawkfield Development 1b - PM' (FG6: 'Strategic Model Hawkfield 2026 Development 1b - PM', Plan 1: 'Network Control Plan 1')
Traffic Flows, Desired

Desired Flow:

	Destination							
		Α	В	С	D	Tot.		
	Α	0	157	23	48	228		
Origin	В	83	0	253	542	878		
Origin	С	40	275	0	241	556		
	D	92	635	19	0	746		
	Tot.	215	1067	295	831	2408		

Traffic Lane Flows

raffic Lane Flows					
Lane	Scenario 6: Strategic Model Hawkfield Development 1b - PM				
Junction: A4095 /	Bucknell Road Network				
1/1	228				
2/1	1067				
3/1 (with short)	878(In) 795(Out)				
3/2 (short)	83				
4/1	866				
5/1	295				
6/1	556				
7/1	831				
8/1 (with short)	746(In) 727(Out)				
8/2 (short)	19				
9/1 (short)	132				
9/2 (with short)	1042(In) 910(Out)				
10/1	215				

Lane Saturation Flows

Junction: A4095 / Bucknell Roa	d Netwo	ork							
Lane	Lane Width (m)	Gradient	Nearside Lane	Allowed Turns	Turning Radius (m)	Turning Prop.	Sat Flow (PCU/Hr)	Flared Sat Flow (PCU/Hr)	
1/1	2.00	0.00	Y	Arm 2 Left	35.00	68.9 %	1000	4000	
(Bucknell Road (North))	3.80	0.00	Y	Arm 4 Ahead	29.00	31.1 %	1908	1908	
2/1 (A4095 (East - Exit) Lane 1)			Inf	Inf					
3/1 (A4095 (East))	3.50	0.00	Υ	Arm 4 Left	12.00	100.0 %	1747	1747	
3/2 (A4095 (East))	3.50	0.00	Υ	Arm 10 Right	37.00	100.0 %	1888	1888	
4/1	2.00	0.00	Y	Arm 5 Ahead	Inf	31.9 %	1707	1737	
(Bucknell Road (Internal SB))	3.00	0.00	ř	Arm 7 Right	10.00	68.1 %	1737		
5/1 (Bucknell Road (South - Exit) Lane 1)		'	Inf	Inf					
6/1			.,	Arm 7 Left	30.00	43.3 %		4004	
(Bucknell Road (South))	3.20	0.00	Y	Arm 9 Ahead	Inf	56.7 %	1894	1894	
7/1 (A4095 (West - Exit) Lane 1)			Infinite S	aturation Flow			Inf	Inf	
8/1 (A4095 (West))	3.75	0.00	Υ	Arm 9 Left	7.00	100.0 %	1639	1639	
8/2 (A4095 (West))	3.20	0.00	Y	Arm 5 Right	25.00	100.0 %	1825	1825	
9/1 (Bucknell Road (Internal NB))	4.40	0.00	Y	Arm 10 Ahead	27.00	100.0 %	1947	1947	
9/2 (Bucknell Road (Internal NB))	4.40	0.00	Y	Arm 2 Right	14.00	100.0 %	1856	1856	
10/1 (Bucknell Road (North - Exit) Lane 1)		,	,	Inf	Inf				

Scenario 7: 'Strategic Model Hawkfield Development 2b - AM' (FG7: 'Strategic Model Hawkfield 2026 Development 2b - AM', Plan 1: 'Network Control Plan 1') Traffic Flows, Desired

Desired Flow:

	Destination										
		Α	В	С	D	Tot.					
	Α	0	172	12	52	236					
Origin	В	149	0	134	575	858					
Origin	С	58	246	0	322	626					
	D	86	364	15	0	465					
	Tot.	293	782	161	949	2185					

Traffic Lane Flows

raffic Lane Flows									
Lane	Scenario 7: Strategic Model Hawkfield Development 2b - AM								
Junction: A4095 /	Bucknell Road Network								
1/1	236								
2/1	782								
3/1 (with short)	858(In) 709(Out)								
3/2 (short)	149								
4/1	773								
5/1	161								
6/1	626								
7/1	949								
8/1 (with short)	465(In) 450(Out)								
8/2 (short)	15								
9/1 (short)	144								
9/2 (with short)	754(In) 610(Out)								
10/1	293								

Lane Saturation Flows

Junction: A4095 / Bucknell Roa	d Netwo	ork						
Lane	Lane Width (m)	Gradient	Nearside Lane	Allowed Turns	Turning Radius (m)	Turning Prop.	Sat Flow (PCU/Hr)	Flared Sat Flow (PCU/Hr)
1/1	2.00	0.00	Y	Arm 2 Left	35.00	72.9 %	1000	1909
(Bucknell Road (North))	3.80	0.00	Y	Arm 4 Ahead	29.00	27.1 %	1909	1909
2/1 (A4095 (East - Exit) Lane 1)			Inf	Inf				
3/1 (A4095 (East))	3.50	0.00	Υ	Arm 4 Left	12.00	100.0 %	1747	1747
3/2 (A4095 (East))	3.50	0.00	Υ	Arm 10 Right	37.00	100.0 %	1888	1888
4/1	2.00	0.00	Y	Arm 5 Ahead	Inf	18.9 %	4707	1707
(Bucknell Road (Internal SB))	3.00	0.00	'	Arm 7 Right	10.00	81.1 %	1707	
5/1 (Bucknell Road (South - Exit) Lane 1)		'	Inf	Inf				
6/1		0.00	.,	Arm 7 Left	30.00	51.4 %	1000	4000
(Bucknell Road (South))	3.20	0.00	Y	Arm 9 Ahead	Inf	48.6 %	1886	1886
7/1 (A4095 (West - Exit) Lane 1)			Infinite S	aturation Flow			Inf	Inf
8/1 (A4095 (West))	3.75	0.00	Υ	Arm 9 Left	7.00	100.0 %	1639	1639
8/2 (A4095 (West))	3.20	0.00	Y	Arm 5 Right	25.00	100.0 %	1825	1825
9/1 (Bucknell Road (Internal NB))	4.40	0.00	Y	Arm 10 Ahead	27.00	100.0 %	1947	1947
9/2 (Bucknell Road (Internal NB))	4.40	0.00	Y	Arm 2 Right	14.00	100.0 %	1856	1856
10/1 (Bucknell Road (North - Exit) Lane 1)		,	,	Inf	Inf			

Scenario 8: 'Strategic Model Hawkfield Development 2b - PM' (FG8: 'Strategic Model Hawkfield 2026 Development 2b - PM', Plan 1: 'Network Control Plan 1')
Traffic Flows, Desired

Desired Flow:

	Destination										
		Α	В	С	D	Tot.					
	Α	0	159	26	57	242					
Origin	В	84	0	247	538	869					
Origin	С	39	284	0	208	531					
	D	89	640	22	0	751					
	Tot.	212	1083	295	803	2393					

Traffic Lane Flows

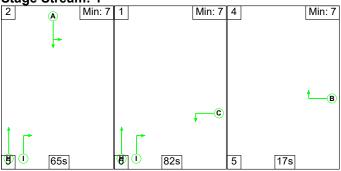
raffic Lane Flo	ws					
Lane	Scenario 8: Strategic Model Hawkfield Development 2b - PM					
Junction: A4095 /	Bucknell Road Network					
1/1	242					
2/1	1083					
3/1 (with short)	869(In) 785(Out)					
3/2 (short)	84					
4/1	868					
5/1	295					
6/1	531					
7/1	803					
8/1 (with short)	751(ln) 729(Out)					
8/2 (short)	22					
9/1 (short)	128					
9/2 (with short)	1052(In) 924(Out)					
10/1	212					

Lane Saturation Flows

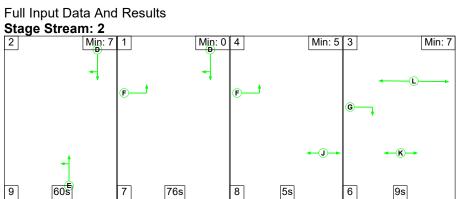
Lane	Lane Width (m)	Gradient	Nearside Lane	Allowed Turns	Turning Radius (m)	Turning Prop.	Sat Flow (PCU/Hr)	Flared Sat Flow (PCU/Hr)	
1/1	2.00	0.00		Arm 2 Left	35.00	65.7 %	4007	4007	
(Bucknell Road (North))	3.80	0.00	Y	Arm 4 Ahead	29.00	34.3 %	1907	1907	
2/1 (A4095 (East - Exit) Lane 1)			Inf	Inf					
3/1 (A4095 (East))	3.50	0.00	Y	Arm 4 Left	12.00	100.0 %	1747	1747	
3/2 (A4095 (East))	3.50	0.00	Y	Arm 10 Right	37.00	100.0 %	1888	1888	
4/1	3.00	0.00	Υ	Arm 5 Ahead	Inf	31.5 %	1736	1736	
(Bucknell Road (Internal SB))	3.00	0.00	T	Arm 7 Right	10.00	68.5 %	1730		
5/1 (Bucknell Road (South - Exit) Lane 1)			Inf	Inf					
6/1	0.00		.,	Arm 7 Left	30.00	39.2 %	1000	4000	
(Bucknell Road (South))	3.20	0.00	Y	Arm 9 Ahead	Inf	60.8 %	1898	1898	
7/1 (A4095 (West - Exit) Lane 1)			Infinite S	aturation Flow			Inf	Inf	
8/1 (A4095 (West))	3.75	0.00	Y	Arm 9 Left	7.00	100.0 %	1639	1639	
8/2 (A4095 (West))	3.20	0.00	Y	Arm 5 Right	25.00	100.0 %	1825	1825	
9/1 (Bucknell Road (Internal NB))	4.40	0.00	Y	Arm 10 Ahead	27.00	100.0 %	1947	1947	
9/2 (Bucknell Road (Internal NB))	4.40	0.00	Y	Arm 2 Right	14.00	100.0 %	1856	1856	
10/1 (Bucknell Road (North - Exit) Lane 1)			Inf	Inf					

Scenario 1: 'Strategic Model Hawkfield Development 1a - AM' (FG1: 'Strategic Model Hawkfield 2026 Development 1a - AM', Plan 1: 'Network Control Plan 1')
Stage Sequence Diagram

Stage Stream: 1







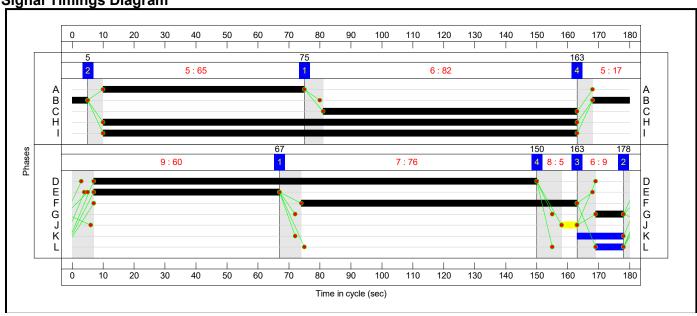
Stage Timings Stage Stream: 1

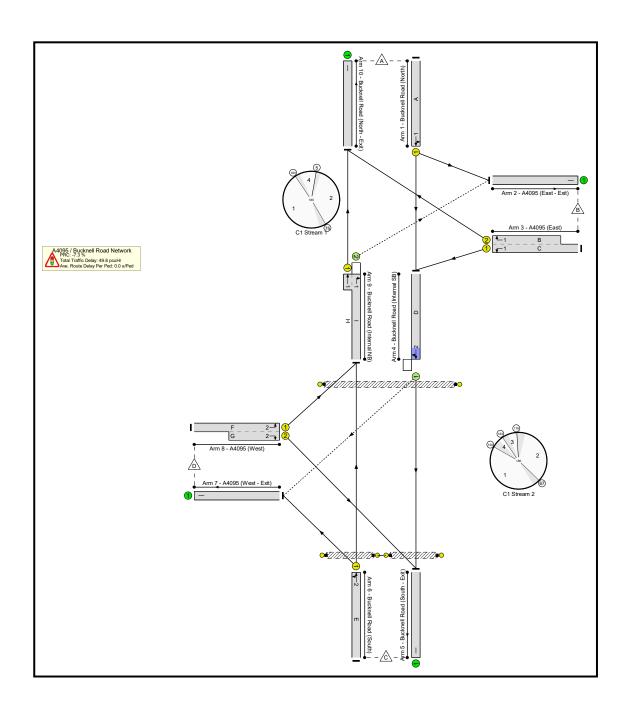
Stage	2	1	4
Duration	65	82	17
Change Point	5	75	163

Stage Stream: 2

J										
Stage	2	1	4	3						
Duration	60	76	5	9						
Change Point	178	67	150	163						







Network Results

Item	Lane Description	Lane Type	Controller Stream	Position In Filtered Route	Full Phase	Arrow Phase	Num Greens	Total Green (s)	Arrow Green (s)	Demand Flow (pcu)	Sat Flow (pcu/Hr)	Capacity (pcu)	Deg Sat (%)
Network: A4095 / Bucknell Road	-	-	N/A	-	-		-	-	-	-	-	-	96.6%
A4095 / Bucknell Road Network	-	-	N/A	-	-		-	-	-	-	-	-	96.6%
1/1	Bucknell Road (North) Left Ahead	U	1	N/A	А		1	65	-	231	1908	700	33.0%
2/1	A4095 (East - Exit)	U	N/A	N/A	-		-	-	-	778	Inf	Inf	0.0%
3/1+3/2	A4095 (East) Left Right	U	1	N/A	СВ		1	82:17	-	863	1747:1888	755+155	95.2 : 93.1%
4/1	Bucknell Road (Internal SB) Ahead Right	0	2	N/A	D		1	143	-	787	1708	828	95.1%
5/1	Bucknell Road (South - Exit)	U	N/A	N/A	-		-	-	-	165	Inf	Inf	0.0%
6/1	Bucknell Road (South) Left Ahead	U	2	N/A	E		1	60	-	617	1885	639	96.6%
7/1	A4095 (West - Exit)	U	N/A	N/A	-		-	-	-	960	Inf	Inf	0.0%
8/1+8/2	A4095 (West) Right Left	U	2	N/A	FG		1	89:9	-	483	1639:1825	810+22	58.0 : 58.0%
9/2+9/1	Bucknell Road (Internal NB) Right Ahead	O+U	1	N/A	ΙH		1	153	-	762	1856:1947	1130+270	54.4 : 54.4%
10/1	Bucknell Road (North - Exit)	U	N/A	N/A	-		-	-	-	291	Inf	Inf	0.0%
Ped Link: P1	Pedestrians Crossing Bucknell Road Northbound	-	2	-	К		1	15	-	0	-	0	0.0%
Ped Link: P2	Pedestrians Crossing Bucknell Road Southbound	-	2	-	J		1	5	-	0	-	0	0.0%

Ped Link: P3	Unnamed Ped Link	-	2	-	L		1	9	-	0	-	0	0.0%
ltem	Arriving (pcu)	Leaving (pcu)	Turners In Gaps (pcu)	Turners When Unopposed (pcu)	Turners In Intergreen (pcu)	Uniform Delay (pcuHr)	Rand + Oversat Delay (pcuHr)	Storage Area Uniform Delay (pcuHr)	Total Delay (pcuHr)	Av. Delay Per PCU (s/pcu)	Max. Back of Uniform Queue (pcu)	Rand + Oversat Queue (pcu)	Mean Max Queue (pcu)
Network: A4095 / Bucknell Road	-	-	262	970	17	32.8	16.1	0.9	49.8	-	-	-	-
A4095 / Bucknell Road Network	-	-	262	970	17	32.8	16.1	0.9	49.8	-	-	-	-
1/1	231	231	-	-	-	2.6	0.2	-	2.9	44.9	8.3	0.2	8.5
2/1	778	778	-	-	-	0.0	0.0	-	0.0	0.0	0.0	0.0	0.0
3/1+3/2	863	863	-	-	-	12.1	7.1	-	19.2 (14.9+4.3)	80.2 (74.5:108.5)	36.5	7.1	43.6
4/1	787	787	13	609	13	3.4	0.0	0.7	4.1	18.6	9.8	0.0	9.8
5/1	165	165	-	-	-	0.0	0.0	-	0.0	0.0	0.0	0.0	0.0
6/1	617	617	-	-	-	10.0	8.1	-	18.1	105.8	30.2	8.1	38.3
7/1	960	960	-	-	-	0.0	0.0	-	0.0	0.0	0.0	0.0	0.0
8/1+8/2	483	483	-	-	-	4.4	0.7	-	5.1 (4.8+0.3)	38.0 (36.7:86.1)	16.5	0.7	17.2
9/2+9/1	762	762	249	361	4	0.2	0.0	0.2	0.4 (0.4+0.0)	1.9 (2.2:0.6)	2.8	0.0	2.8
10/1	291	291	-	-	-	0.0	0.0	-	0.0	0.0	0.0	0.0	0.0
Ped Link: P1	0	0	-	-	-	-	-	-	-	-	-	-	-
Ped Link: P2	0	0	-	-	-	-	-	-	-	-	-	-	-
Ped Link: P3	0	0	-	-	-	-	-	-	-	-	-	-	-

C1 Stream: 1 PRC for Signalled Lanes (%):
C1 Stream: 2 PRC for Signalled Lanes (%):
PRC Over All Lanes (%):

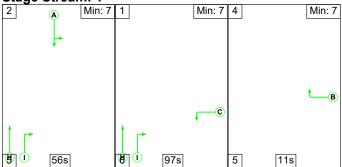
-5.8 -7.3 -7.3 Total Delay for Signalled Lanes (pcuHr): Total Delay for Signalled Lanes (pcuHr): Total Delay Over All Lanes(pcuHr): 22.50 Cycle Time (s): 180 27.31 Cycle Time (s): 180 49.81

Scenario 2: 'Strategic Model Hawkfield Development 1a - PM' (FG2: 'Strategic Model Hawkfield 2026

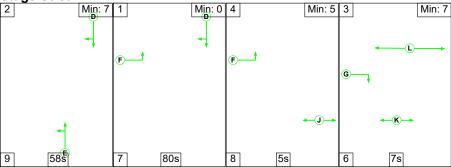
Development 1a - PM', Plan 1: 'Network Control Plan 1')

Stage Sequence Diagram

Stage Stream: 1



Stage Stream: 2



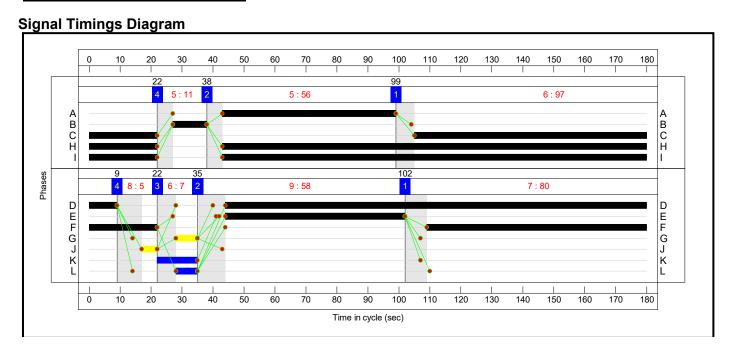
Stage Timings

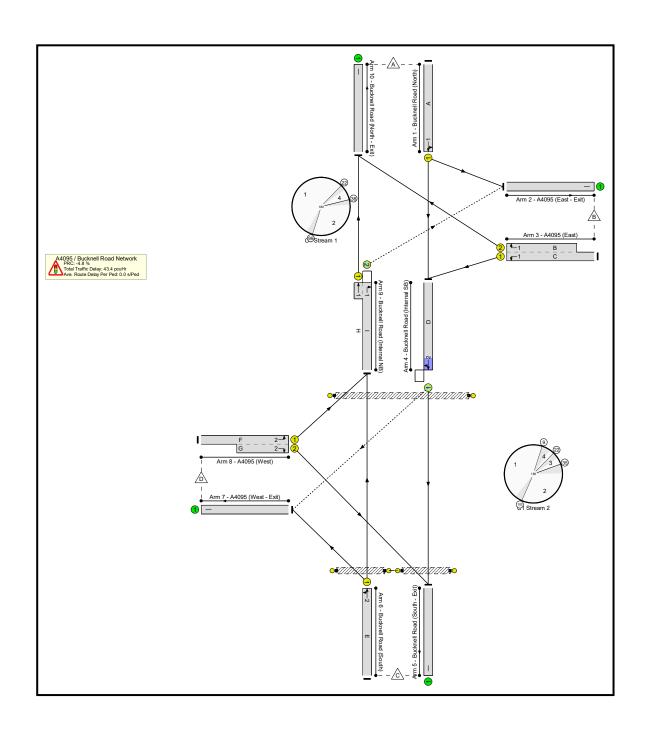
Stage Stream: 1

Stage	2	1	4	
Duration	56	97	11	
Change Point	38	99	22	

Stage Stream: 2

Stage	2	1	4	3
Duration	58	80	5	7
Change Point	35	102	9	22





Network Results

Item	Lane Description	Lane Type	Controller Stream	Position In Filtered Route	Full Phase	Arrow Phase	Num Greens	Total Green (s)	Arrow Green (s)	Demand Flow (pcu)	Sat Flow (pcu/Hr)	Capacity (pcu)	Deg Sat (%)
Network: A4095 / Bucknell Road	-	-	N/A	-	-		-	-	-	-	-	-	94.3%
A4095 / Bucknell Road Network	-	-	N/A	-	-		-	-	-	-	-	-	94.3%
1/1	Bucknell Road (North) Left Ahead	U	1	N/A	А		1	56	-	233	1908	604	38.6%
2/1	A4095 (East - Exit)	U	N/A	N/A	-		-	-	-	1067	Inf	Inf	0.0%
3/1+3/2	A4095 (East) Left Right	U	1	N/A	СВ		1	97:11	-	889	1747:1888	913+95	88.2 : 88.2%
4/1	Bucknell Road (Internal SB) Ahead Right	0	2	N/A	D		1	145	-	884	1739	938	94.3%
5/1	Bucknell Road (South - Exit)	U	N/A	N/A	-		-	-	-	304	Inf	Inf	0.0%
6/1	Bucknell Road (South) Left Ahead	U	2	N/A	E		1	58	-	556	1896	621	89.5%
7/1	A4095 (West - Exit)	U	N/A	N/A	-		-	-	-	827	Inf	Inf	0.0%
8/1+8/2	A4095 (West) Right Left	U	2	N/A	FG		1	93:7	-	743	1639:1825	848+20	85.6 : 85.6%
9/2+9/1	Bucknell Road (Internal NB) Right Ahead	O+U	1	N/A	IН		1	159	-	1052	1856:1947	1249+190	73.1 : 73.1%
10/1	Bucknell Road (North - Exit)	U	N/A	N/A	-		-	-	-	223	Inf	Inf	0.0%
Ped Link: P1	Pedestrians Crossing Bucknell Road Northbound	-	2	-	К		1	13	-	0	-	0	0.0%
Ped Link: P2	Pedestrians Crossing Bucknell Road Southbound	-	2	-	J		1	5	-	0	-	0	0.0%

Ped Link: P3	Unnamed Ped Link	-	2	-	L		1	7	-	0	-	0	0.0%
ltem	Arriving (pcu)	Leaving (pcu)	Turners In Gaps (pcu)	Turners When Unopposed (pcu)	Turners In Intergreen (pcu)	Uniform Delay (pcuHr)	Rand + Oversat Delay (pcuHr)	Storage Area Uniform Delay (pcuHr)	Total Delay (pcuHr)	Av. Delay Per PCU (s/pcu)	Max. Back of Uniform Queue (pcu)	Rand + Oversat Queue (pcu)	Mean Max Queue (pcu)
Network: A4095 / Bucknell Road	-	-	302	1195	13	31.8	10.5	1.0	43.4	-	-	-	-
A4095 / Bucknell Road Network	-	-	302	1195	13	31.8	10.5	1.0	43.4	-	-	-	-
1/1	233	233	-	-	-	3.1	0.3	-	3.4	52.7	9.1	0.3	9.4
2/1	1067	1067	-	-	-	0.0	0.0	-	0.0	0.0	0.0	0.0	0.0
3/1+3/2	889	889	-	-	-	9.7	3.5	-	13.2 (11.0+2.2)	53.6 (49.1:96.4)	35.9	3.5	39.5
4/1	884	884	40	551	6	2.0	0.0	0.6	2.6	10.8	5.5	0.0	5.5
5/1	304	304	-	-	-	0.0	0.0	-	0.0	0.0	0.0	0.0	0.0
6/1	556	556	-	-	-	8.9	3.8	-	12.7	82.2	26.4	3.8	30.2
7/1	827	827	-	-	-	0.0	0.0	-	0.0	0.0	0.0	0.0	0.0
8/1+8/2	743	743	-	-	-	7.8	2.9	-	10.7 (10.2+0.5)	51.8 (50.7:96.8)	31.8	2.9	34.6
9/2+9/1	1052	1052	262	644	7	0.3	0.0	0.4	0.7 (0.7+0.0)	2.5 (2.8:0.5)	9.1	0.0	9.1
10/1	223	223	-	-	-	0.0	0.0	-	0.0	0.0	0.0	0.0	0.0
Ped Link: P1	0	0	-	-	-	-	-	-	-	-	-	-	-
Ped Link: P2	0	0	-	-	-	-	-	-	-	-	-	-	-
Ped Link: P3	0	0	-	-	-	-	-	-	-	-	-	-	-

C1 Stream: 1 PRC for Signalled Lanes (%):
C1 Stream: 2 PRC for Signalled Lanes (%):
PRC Over All Lanes (%):

^{2.0} -4.8 -4.8

Total Delay for Signalled Lanes (pcuHr): Total Delay for Signalled Lanes (pcuHr): Total Delay Over All Lanes(pcuHr):

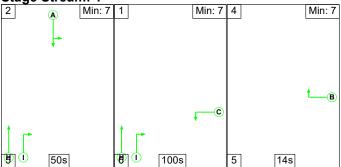
^{17.36} Cycle Time (s): 180 26.02 Cycle Time (s): 180 43.38

Scenario 3: 'Strategic Model Hawkfield Development 2a - AM' (FG3: 'Strategic Model Hawkfield 2026

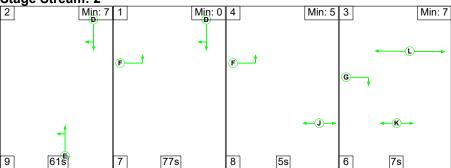
Development 2a - AM', Plan 1: 'Network Control Plan 1')

Stage Sequence Diagram

Stage Stream: 1



Stage Stream: 2



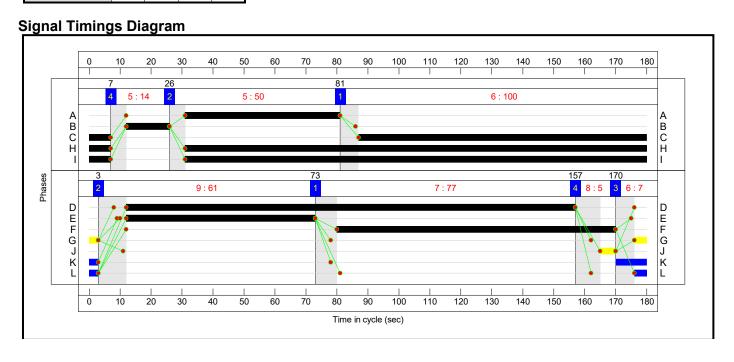
Stage Timings

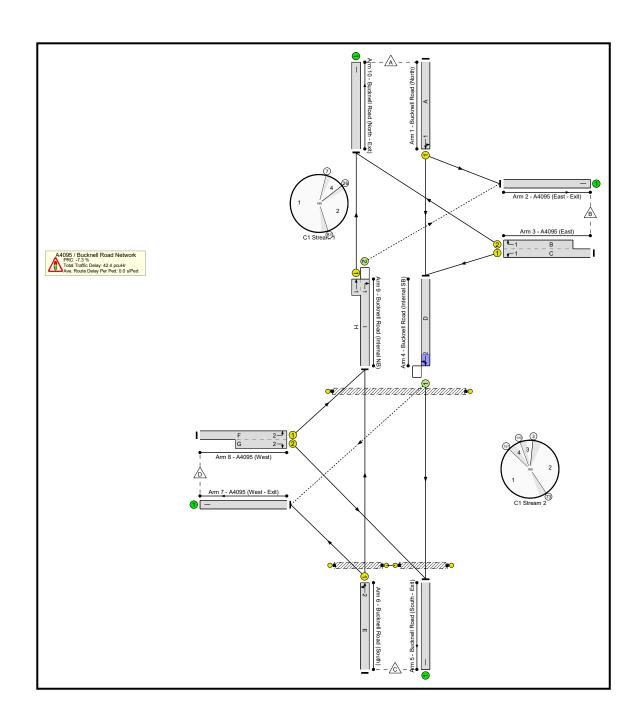
Stage Stream: 1

Stage	2	1	4
Duration	50	100	14
Change Point	26	81	7

Stage Stream: 2

Stage	2	1	4	3	
Duration	61	77	5	7	
Change Point	3	73	157	170	





Network Results

Item	Lane Description	Lane Type	Controller Stream	Position In Filtered Route	Full Phase	Arrow Phase	Num Greens	Total Green (s)	Arrow Green (s)	Demand Flow (pcu)	Sat Flow (pcu/Hr)	Capacity (pcu)	Deg Sat (%)
Network: A4095 / Bucknell Road	-	-	N/A	-	-		-	-	-	-	-	-	96.6%
A4095 / Bucknell Road Network	-	-	N/A	-	-		-	-	-	-	-	-	96.6%
1/1	Bucknell Road (North) Left Ahead	U	1	N/A	А		1	50	-	226	1908	541	41.8%
2/1	A4095 (East - Exit)	U	N/A	N/A	-		-	-	-	775	Inf	Inf	0.0%
3/1+3/2	A4095 (East) Left Right	U	1	N/A	СВ		1	100:14	-	883	1747:1888	900+157	81.3 : 96.6%
4/1	Bucknell Road (Internal SB) Ahead Right	0	2	N/A	D		1	145	-	796	1711	849	93.8%
5/1	Bucknell Road (South - Exit)	U	N/A	N/A	-		-	-	-	175	Inf	Inf	0.0%
6/1	Bucknell Road (South) Left Ahead	U	2	N/A	E		1	61	-	618	1885	649	95.2%
7/1	A4095 (West - Exit)	U	N/A	N/A	-		-	-	-	960	Inf	Inf	0.0%
8/1+8/2	A4095 (West) Right Left	U	2	N/A	FG		1	90:7	-	481	1639:1825	819+23	57.2 : 57.2%
9/2+9/1	Bucknell Road (Internal NB) Right Ahead	O+U	1	N/A	IН		1	156	-	760	1856:1947	1174+279	52.3 : 52.3%
10/1	Bucknell Road (North - Exit)	U	N/A	N/A	-		-	-	-	298	Inf	Inf	0.0%
Ped Link: P1	Pedestrians Crossing Bucknell Road Northbound	-	2	-	К		1	13	-	0	-	0	0.0%
Ped Link: P2	Pedestrians Crossing Bucknell Road Southbound	-	2	-	J		1	5	-	0	-	0	0.0%
Ped Link: P3	Unnamed Ped Link	-	2	-	L		1	7	-	0	-	0	0.0%

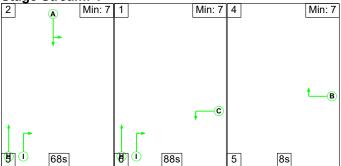
Item	Arriving (pcu)	Leaving (pcu)	Turners In Gaps (pcu)	Turners When Unopposed (pcu)	Turners In Intergreen (pcu)	Uniform Delay (pcuHr)	Rand + Oversat Delay (pcuHr)	Storage Area Uniform Delay (pcuHr)	Total Delay (pcuHr)	Av. Delay Per PCU (s/pcu)	Max. Back of Uniform Queue (pcu)	Rand + Oversat Queue (pcu)	Mean Max Queue (pcu)
Network: A4095 / Bucknell Road	-	-	239	1003	6	30.9	10.4	1.1	42.4	-	-	-	-
A4095 / Bucknell Road Network	-	-	239	1003	6	30.9	10.4	1.1	42.4	-	-	-	-
1/1	226	226	-	-	-	3.3	0.4	-	3.7	58.1	9.2	0.4	9.5
2/1	775	775	-	-	-	0.0	0.0	-	0.0	0.0	0.0	0.0	0.0
3/1+3/2	883	883	-	-	-	9.5	2.5	-	12.0 (8.1+3.9)	49.0 (39.9:92.3)	29.4	2.5	31.9
4/1	796	796	17	611	6	3.3	0.0	0.7	4.0	17.9	10.7	0.0	10.7
5/1	175	175	-	-	-	0.0	0.0	-	0.0	0.0	0.0	0.0	0.0
6/1	618	618	-	-	-	9.9	6.9	-	16.7	97.5	30.0	6.9	36.9
7/1	960	960	-	-	-	0.0	0.0	-	0.0	0.0	0.0	0.0	0.0
8/1+8/2	481	481	-	-	-	4.3	0.7	-	5.0 (4.7+0.3)	37.2 (35.8:87.8)	16.3	0.7	17.0
9/2+9/1	760	760	222	392	0	0.6	0.0	0.4	1.1 (1.0+0.0)	5.1 (6.1:1.0)	11.5	0.0	11.5
10/1	298	298	-	-	-	0.0	0.0	-	0.0	0.0	0.0	0.0	0.0
Ped Link: P1	0	0	-	-	-	-	-	-	-	-	-	-	-
Ped Link: P2	0	0	-	-	-	-	-	-	-	-	-	-	-
Ped Link: P3	0	0	-	-	-	-	-	-	-	-	-	-	-
		C1 Str	eam: 2 PRC for S	Lanes (%): Signalled Lanes (%): Over All Lanes (%):	-7.3 -5.8 -7.3	Total Delay	for Signalled La for Signalled La Delay Over All La	nes (pcuHr): nes (pcuHr):	16.73 Cy	/cle Time (s): 180 /cle Time (s): 180)		<u> </u>

Scenario 4: 'Strategic Model Hawkfield Development 2a - PM' (FG4: 'Strategic Model Hawkfield 2026

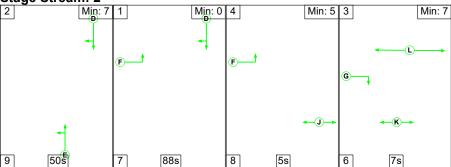
Development 2a - PM', Plan 1: 'Network Control Plan 1')

Stage Sequence Diagram

Stage Stream: 1



Stage Stream: 2



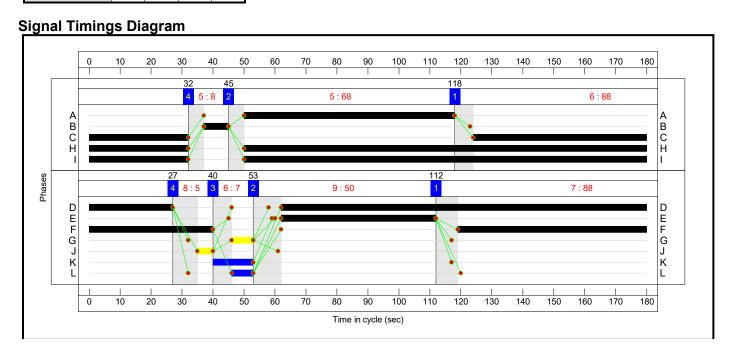
Stage Timings

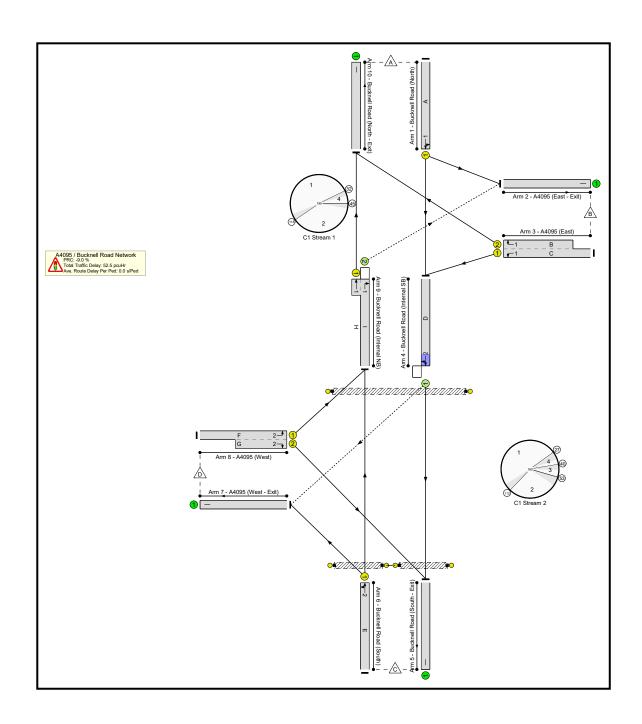
Stage Stream: 1

Stage	2	1	4
Duration	68	88	8
Change Point	45	118	32

Stage Stream: 2

Stage	2	1	4	3
Duration	50	88	5	7
Change Point	53	112	27	40



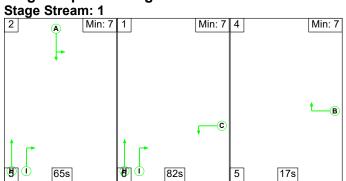


Network Results

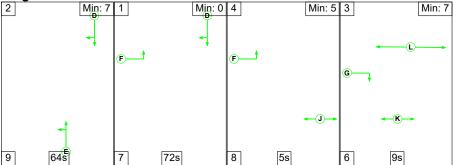
Item	Lane Description	Lane Type	Controller Stream	Position In Filtered Route	Full Phase	Arrow Phase	Num Greens	Total Green (s)	Arrow Green (s)	Demand Flow (pcu)	Sat Flow (pcu/Hr)	Capacity (pcu)	Deg Sat (%)
Network: A4095 / Bucknell Road	-	-	N/A	-	-		-	-	-	-	-	-	98.1%
A4095 / Bucknell Road Network	-	-	N/A	-	-		-	-	-	-	-	-	98.1%
1/1	Bucknell Road (North) Left Ahead	U	1	N/A	А		1	68	-	256	1907	731	35.0%
2/1	A4095 (East - Exit)	U	N/A	N/A	-		-	-	-	1093	Inf	Inf	0.0%
3/1+3/2	A4095 (East) Left Right	U	1	N/A	СВ		1	88:8	-	881	1747:1888	832+89	95.7 : 95.7%
4/1	Bucknell Road (Internal SB) Ahead Right	0	2	N/A	D		1	145	-	888	1737	946	93.8%
5/1	Bucknell Road (South - Exit)	U	N/A	N/A	-		-	-	-	296	Inf	Inf	0.0%
6/1	Bucknell Road (South) Left Ahead	U	2	N/A	E		1	50	-	528	1900	538	98.1%
7/1	A4095 (West - Exit)	U	N/A	N/A	-		-	-	-	799	Inf	Inf	0.0%
8/1+8/2	A4095 (West) Right Left	U	2	N/A	FG		1	101:7	-	743	1639:1825	921+19	79.0 : 79.0%
9/2+9/1	Bucknell Road (Internal NB) Right Ahead	O+U	1	N/A	ΙH		1	162	-	1064	1856:1947	1243+181	74.7 : 74.7%
10/1	Bucknell Road (North - Exit)	U	N/A	N/A	-		-	-	-	220	Inf	Inf	0.0%
Ped Link: P1	Pedestrians Crossing Bucknell Road Northbound	-	2	-	К		1	13	-	0	-	0	0.0%
Ped Link: P2	Pedestrians Crossing Bucknell Road Southbound	-	2	-	J		1	5	-	0	-	0	0.0%

Ped Link: P3	Unnamed Ped Link	-	2	-	L		1	7	-	0	-	0	0.0%
ltem	Arriving (pcu)	Leaving (pcu)	Turners In Gaps (pcu)	Turners When Unopposed (pcu)	Turners In Intergreen (pcu)	Uniform Delay (pcuHr)	Rand + Oversat Delay (pcuHr)	Storage Area Uniform Delay (pcuHr)	Total Delay (pcuHr)	Av. Delay Per PCU (s/pcu)	Max. Back of Uniform Queue (pcu)	Rand + Oversat Queue (pcu)	Mean Max Queue (pcu)
Network: A4095 / Bucknell Road	-	-	338	1179	19	32.4	19.2	1.0	52.5	-	-	-	-
A4095 / Bucknell Road Network	-	-	338	1179	19	32.4	19.2	1.0	52.5	-	-	-	-
1/1	256	256	-	-	-	2.8	0.3	-	3.1	43.3	9.1	0.3	9.4
2/1	1093	1093	-	-	-	0.0	0.0	-	0.0	0.0	0.0	0.0	0.0
3/1+3/2	881	881	-	-	-	11.4	7.9	-	19.3 (16.6+2.8)	79.0 (74.9:117.3)	39.5	7.9	47.4
4/1	888	888	5	590	12	2.0	0.0	0.6	2.5	10.2	8.0	0.0	8.0
5/1	296	296	-	-	-	0.0	0.0	-	0.0	0.0	0.0	0.0	0.0
6/1	528	528	-	-	-	9.4	9.2	-	18.6	126.7	26.1	9.2	35.3
7/1	799	799	-	-	-	0.0	0.0	-	0.0	0.0	0.0	0.0	0.0
8/1+8/2	743	743	-	-	-	6.5	1.9	-	8.3 (8.0+0.4)	40.4 (39.4:91.9)	28.7	1.9	30.6
9/2+9/1	1064	1064	333	589	7	0.3	0.0	0.4	0.7 (0.6+0.0)	2.2 (2.5:0.6)	3.5	0.0	3.5
10/1	220	220	-	-	-	0.0	0.0	-	0.0	0.0	0.0	0.0	0.0
Ped Link: P1	0	0	-	-	-	-	-	-	-	-	-	-	-
Ped Link: P2	0	0	-	-	-	-	-	-	-	-	-	-	-
Ped Link: P3	0	0	_	_	-	-	-	-	_	_	-	-	-

Scenario 5: 'Strategic Model Hawkfield Development 1b - AM' (FG5: 'Strategic Model Hawkfield 2026 Development 1b - AM', Plan 1: 'Network Control Plan 1') **Stage Sequence Diagram**



Stage Stream: 2



Stage Timings

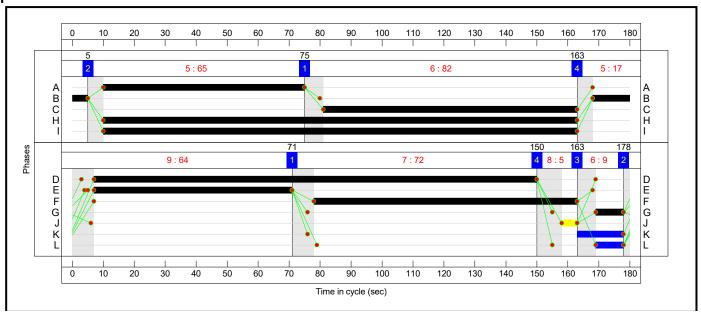
Stage Stream: 1

Stage	2	1	4
Duration	65	82	17
Change Point	5	75	163

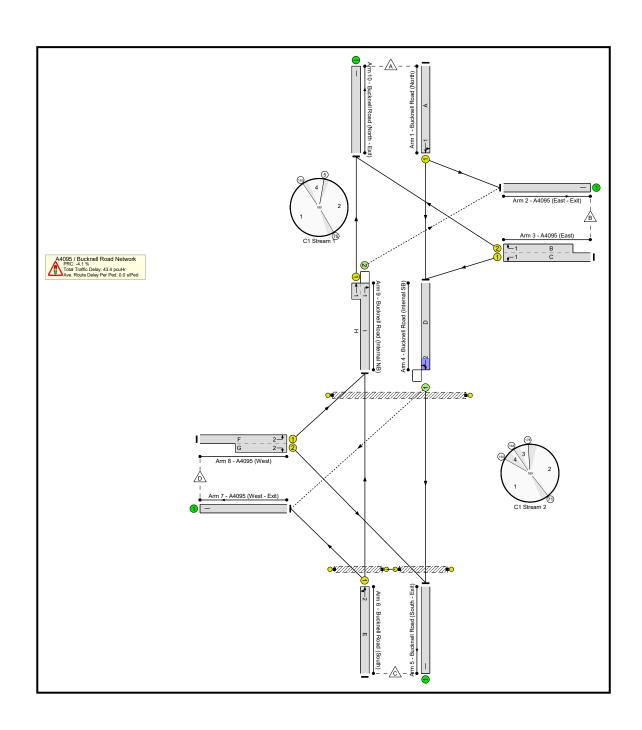
Stage Stream: 2

Stage	2	1	4	3
Duration	64	72	5	9
Change Point	178	71	150	163

Signal Timings Diagram



Network Layout Diagram



Item	Lane Description	Lane Type	Controller Stream	Position In Filtered Route	Full Phase	Arrow Phase	Num Greens	Total Green (s)	Arrow Green (s)	Demand Flow (pcu)	Sat Flow (pcu/Hr)	Capacity (pcu)	Deg Sat (%)
Network: A4095 / Bucknell Road	-	-	N/A	-	-		-	-	-	-	-	-	93.7%
A4095 / Bucknell Road Network	-	-	N/A	-	-		-	-	-	-	-	-	93.7%
1/1	Bucknell Road (North) Left Ahead	U	1	N/A	А		1	65	-	240	1908	700	34.3%
2/1	A4095 (East - Exit)	U	N/A	N/A	-		-	-	-	781	Inf	Inf	0.0%
3/1+3/2	A4095 (East) Left Right	U	1	N/A	СВ		1	82:17	-	842	1747:1888	754+161	92.7 : 88.6%
4/1	Bucknell Road (Internal SB) Ahead Right	0	2	N/A	D		1	143	-	768	1706	819	93.7%
5/1	Bucknell Road (South - Exit)	U	N/A	N/A	-		-	-	-	158	Inf	Inf	0.0%
6/1	Bucknell Road (South) Left Ahead	U	2	N/A	E		1	64	-	625	1886	681	91.8%
7/1	A4095 (West - Exit)	U	N/A	N/A	-		-	-	-	949	Inf	Inf	0.0%
8/1+8/2	A4095 (West) Right Left	U	2	N/A	FG		1	85:9	-	469	1639:1825	769+29	58.8 : 58.8%
9/2+9/1	Bucknell Road (Internal NB) Right Ahead	O+U	1	N/A	ΙH		1	153	-	755	1856:1947	1124+267	54.3 : 54.3%
10/1	Bucknell Road (North - Exit)	U	N/A	N/A	-		-	-	-	288	Inf	Inf	0.0%
Ped Link: P1	Pedestrians Crossing Bucknell Road Northbound	-	2	-	К		1	15	-	0	-	0	0.0%
Ped Link: P2	Pedestrians Crossing Bucknell Road Southbound	-	2	-	J		1	5	-	0	-	0	0.0%

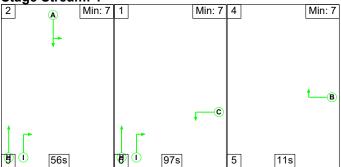
Ped Link: P3	Unnamed Ped Link	-	2	-	L		1	9	-	0	-	0	0.0%
ltem	Arriving (pcu)	Leaving (pcu)	Turners In Gaps (pcu)	Turners When Unopposed (pcu)	Turners In Intergreen (pcu)	Uniform Delay (pcuHr)	Rand + Oversat Delay (pcuHr)	Storage Area Uniform Delay (pcuHr)	Total Delay (pcuHr)	Av. Delay Per PCU (s/pcu)	Max. Back of Uniform Queue (pcu)	Rand + Oversat Queue (pcu)	Mean Max Queue (pcu)
Network: A4095 / Bucknell Road	-	-	286	934	17	31.6	10.8	1.0	43.4	-	-	-	-
A4095 / Bucknell Road Network	-	-	286	934	17	31.6	10.8	1.0	43.4	-	-	-	-
1/1	240	240	-	-	-	2.8	0.3	-	3.0	45.2	8.7	0.3	8.9
2/1	781	781	-	-	-	0.0	0.0	-	0.0	0.0	0.0	0.0	0.0
3/1+3/2	842	842	-	-	-	11.7	5.0	-	16.7 (12.7+4.0)	71.4 (65.5:100.4)	34.5	5.0	39.5
4/1	768	768	34	580	13	2.9	0.0	0.7	3.6	16.9	8.8	0.0	8.8
5/1	158	158	-	-	-	0.0	0.0	-	0.0	0.0	0.0	0.0	0.0
6/1	625	625	-	-	-	9.5	4.8	-	14.3	82.4	29.7	4.8	34.5
7/1	949	949	-	-	-	0.0	0.0	-	0.0	0.0	0.0	0.0	0.0
8/1+8/2	469	469	-	-	-	4.6	0.7	-	5.3 (4.9+0.4)	41.1 (39.3:86.6)	16.5	0.7	17.2
9/2+9/1	755	755	252	354	4	0.2	0.0	0.3	0.4 (0.4+0.0)	2.1 (2.4:0.6)	3.5	0.0	3.5
10/1	288	288	-	-	-	0.0	0.0	-	0.0	0.0	0.0	0.0	0.0
Ped Link: P1	0	0	-	-	-	-	-	-	-	-	-	-	-
Ped Link: P2	0	0	-	-	-	-	-	-	-	_	-	-	-
Ped Link: P3	0	0	-	_	-	_	-	-	_	_	-	_	_

Scenario 6: 'Strategic Model Hawkfield Development 1b - PM' (FG6: 'Strategic Model Hawkfield 2026

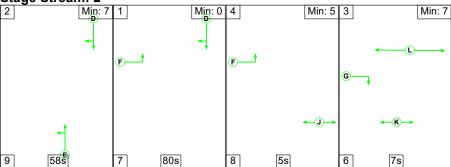
Development 1b - PM', Plan 1: 'Network Control Plan 1')

Stage Sequence Diagram

Stage Stream: 1



Stage Stream: 2



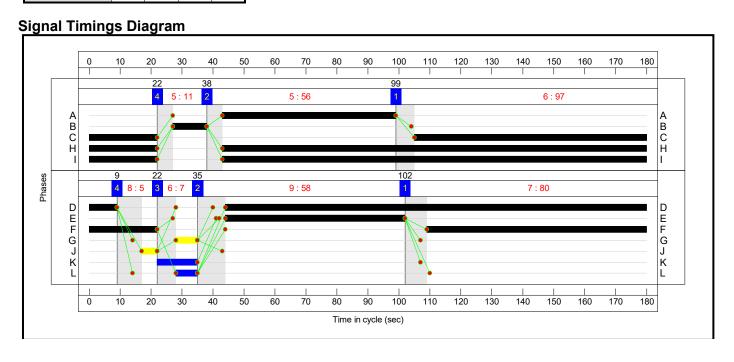
Stage Timings

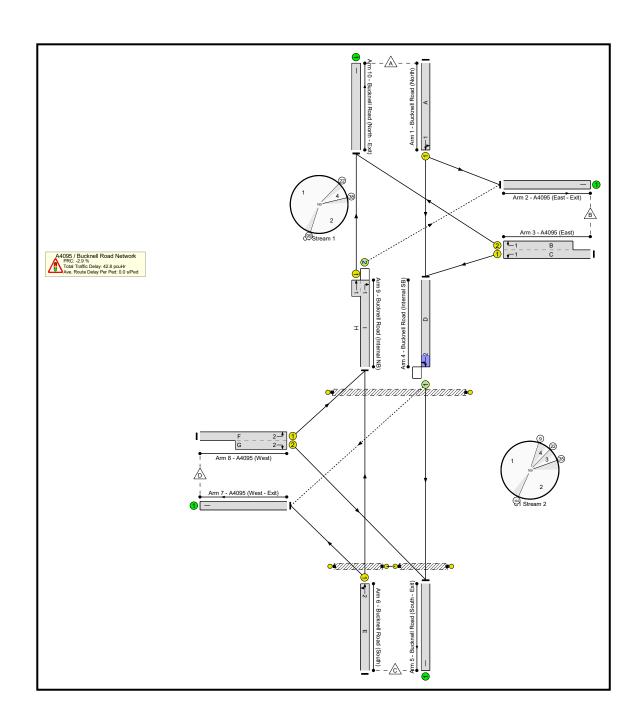
Stage Stream: 1

Stage	2	1	4
Duration	56	97	11
Change Point	38	99	22

Stage Stream: 2

Stage	2	1	4	3
Duration	58	80	5	7
Change Point	35	102	9	22





Item	Lane Description	Lane Type	Controller Stream	Position In Filtered Route	Full Phase	Arrow Phase	Num Greens	Total Green (s)	Arrow Green (s)	Demand Flow (pcu)	Sat Flow (pcu/Hr)	Capacity (pcu)	Deg Sat (%)
Network: A4095 / Bucknell Road	-	-	N/A	-	-		-	-	-	-	-	-	92.6%
A4095 / Bucknell Road Network	-	-	N/A	-	-		-	-	-	-	-	-	92.6%
1/1	Bucknell Road (North) Left Ahead	U	1	N/A	А		1	56	-	228	1908	604	37.7%
2/1	A4095 (East - Exit)	U	N/A	N/A	-		-	-	-	1067	Inf	Inf	0.0%
3/1+3/2	A4095 (East) Left Right	U	1	N/A	СВ		1	97:11	-	878	1747:1888	913+95	87.1 : 87.1%
4/1	Bucknell Road (Internal SB) Ahead Right	0	2	N/A	D		1	145	-	866	1737	935	92.6%
5/1	Bucknell Road (South - Exit)	U	N/A	N/A	-		-	-	-	295	Inf	Inf	0.0%
6/1	Bucknell Road (South) Left Ahead	U	2	N/A	E		1	58	-	556	1894	621	89.6%
7/1	A4095 (West - Exit)	U	N/A	N/A	-		-	-	-	831	Inf	Inf	0.0%
8/1+8/2	A4095 (West) Right Left	U	2	N/A	FG		1	93:7	-	746	1639:1825	846+22	85.9 : 85.9%
9/2+9/1	Bucknell Road (Internal NB) Right Ahead	O+U	1	N/A	IН		1	159	-	1042	1856:1947	1257+182	72.4 : 72.4%
10/1	Bucknell Road (North - Exit)	U	N/A	N/A	-		-	-	-	215	Inf	Inf	0.0%
Ped Link: P1	Pedestrians Crossing Bucknell Road Northbound	-	2	-	К		1	13	-	0	-	0	0.0%
Ped Link: P2	Pedestrians Crossing Bucknell Road Southbound	-	2	-	J		1	5	-	0	-	0	0.0%

Ped Link: P3	Unnamed Ped Link	-	2	-	L		1	7	-	0	-	0	0.0%
ltem	Arriving (pcu)	Leaving (pcu)	Turners In Gaps (pcu)	Turners When Unopposed (pcu)	Turners In Intergreen (pcu)	Uniform Delay (pcuHr)	Rand + Oversat Delay (pcuHr)	Storage Area Uniform Delay (pcuHr)	Total Delay (pcuHr)	Av. Delay Per PCU (s/pcu)	Max. Back of Uniform Queue (pcu)	Rand + Oversat Queue (pcu)	Mean Max Queue (pcu)
Network: A4095 / Bucknell Road	-	-	304	1183	13	31.5	10.3	1.0	42.8	-	-	-	-
A4095 / Bucknell Road Network	-	-	304	1183	13	31.5	10.3	1.0	42.8	-	-	-	-
1/1	228	228	-	-	-	3.0	0.3	-	3.3	52.5	8.8	0.3	9.1
2/1	1067	1067	-	-	-	0.0	0.0	-	0.0	0.0	0.0	0.0	0.0
3/1+3/2	878	878	-	-	-	9.5	3.2	-	12.7 (10.5+2.2)	52.1 (47.6:95.2)	35.0	3.2	38.
4/1	866	866	39	545	6	1.9	0.0	0.6	2.5	10.5	5.0	0.0	5.0
5/1	295	295	-	-	-	0.0	0.0	-	0.0	0.0	0.0	0.0	0.
6/1	556	556	-	-	-	8.9	3.8	-	12.7	82.4	26.4	3.8	30
7/1	831	831	-	-	-	0.0	0.0	-	0.0	0.0	0.0	0.0	0.
8/1+8/2	746	746	-	-	-	7.9	2.9	-	10.8 (10.3+0.5)	52.2 (51.0:97.1)	31.9	2.9	34.
9/2+9/1	1042	1042	265	638	7	0.3	0.0	0.4	0.6 (0.6+0.0)	2.2 (2.5:0.5)	7.5	0.0	7.
10/1	215	215	-	-	-	0.0	0.0	-	0.0	0.0	0.0	0.0	0.
Ped Link: P1	0	0	-	-	-	-	-	-	-	-	-	-	-
Ped Link: P2	0	0	-	-	-	-	-	-		-	-	-	-
Ped Link: P3	0	0	-	-	_	-	-	-	-	_	-	-	-

C1 Stream: 2 PRC for Signalled Lanes (%): PRC Over All Lanes (%):

-2.9 -2.9

Total Delay for Signalled Lanes (pcuHr): Total Delay Over All Lanes(pcuHr):

26.08 42.77

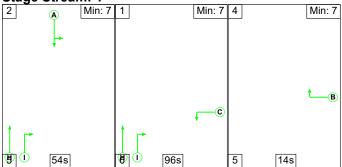
Cycle Time (s): 180

Scenario 7: 'Strategic Model Hawkfield Development 2b - AM' (FG7: 'Strategic Model Hawkfield 2026

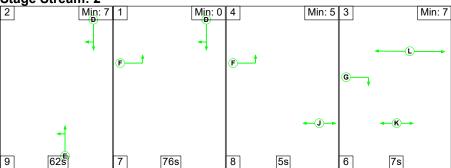
Development 2b - AM', Plan 1: 'Network Control Plan 1')

Stage Sequence Diagram

Stage Stream: 1



Stage Stream: 2



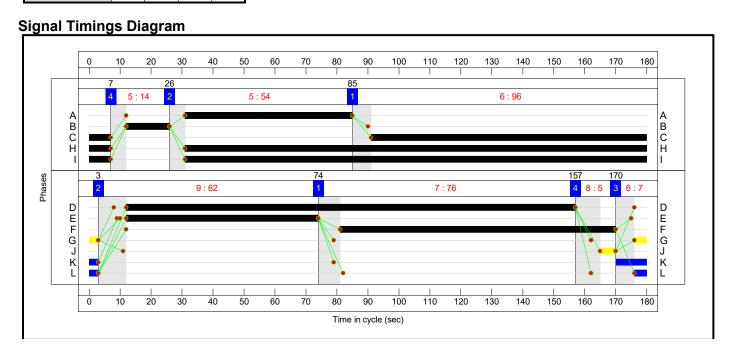
Stage Timings

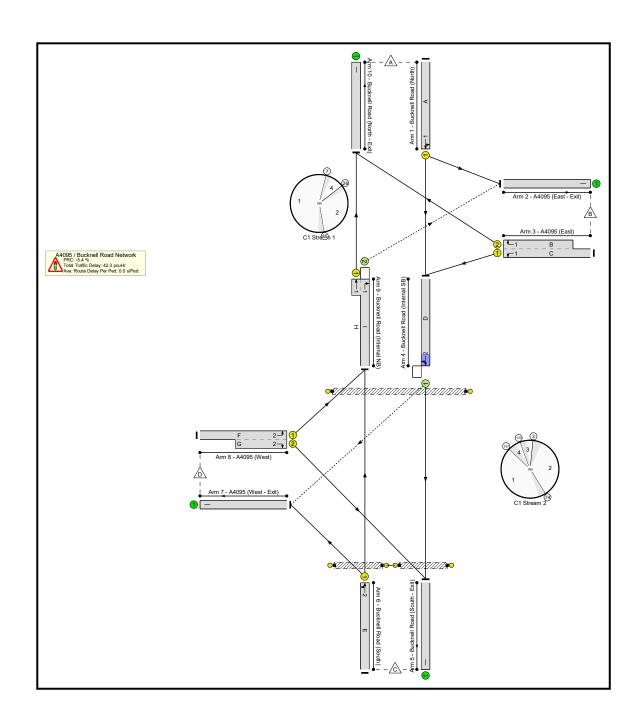
Stage Stream: 1

Stage	2	1	4
Duration	54	96	14
Change Point	26	85	7

Stage Stream: 2

Stage	2	1	4	3
Duration	62	76	5	7
Change Point	3	74	157	170





Item	Lane Description	Lane Type	Controller Stream	Position In Filtered Route	Full Phase	Arrow Phase	Num Greens	Total Green (s)	Arrow Green (s)	Demand Flow (pcu)	Sat Flow (pcu/Hr)	Capacity (pcu)	Deg Sat (%)
Network: A4095 / Bucknell Road	-	-	N/A	-	-		-	-	-	-	-	-	94.8%
A4095 / Bucknell Road Network	-	-	N/A	-	-		-	-	-	-	-	-	94.8%
1/1	Bucknell Road (North) Left Ahead	U	1	N/A	А		1	54	-	236	1909	583	40.5%
2/1	A4095 (East - Exit)	U	N/A	N/A	-		-	-	-	782	Inf	Inf	0.0%
3/1+3/2	A4095 (East) Left Right	U	1	N/A	СВ		1	96:14	-	858	1747:1888	866+157	81.8 : 94.7%
4/1	Bucknell Road (Internal SB) Ahead Right	0	2	N/A	D		1	145	-	773	1707	841	91.9%
5/1	Bucknell Road (South - Exit)	U	N/A	N/A	-		-	-	-	161	Inf	Inf	0.0%
6/1	Bucknell Road (South) Left Ahead	U	2	N/A	E		1	62	-	626	1886	660	94.8%
7/1	A4095 (West - Exit)	U	N/A	N/A	-		-	-	-	949	Inf	Inf	0.0%
8/1+8/2	A4095 (West) Right Left	U	2	N/A	F G		1	89:7	-	465	1639:1825	807+27	55.8 : 55.8%
9/2+9/1	Bucknell Road (Internal NB) Right Ahead	O+U	1	N/A	IН		1	156	-	754	1856:1947	1163+275	52.5 : 52.5%
10/1	Bucknell Road (North - Exit)	U	N/A	N/A	-		-	-	-	293	Inf	Inf	0.0%
Ped Link: P1	Pedestrians Crossing Bucknell Road Northbound	-	2	-	К		1	13	-	0	-	0	0.0%
Ped Link: P2	Pedestrians Crossing Bucknell Road Southbound	-	2	-	J		1	5	-	0	-	0	0.0%
Ped Link: P3	Unnamed Ped Link	-	2	-	L		1	7	-	0	-	0	0.0%

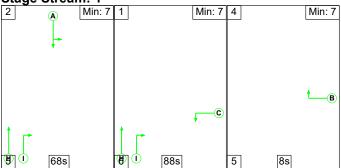
- 236 782 858 773	263 263 - -	963 963 -	11 11 -	31.0 31.0 3.2 0.0	10.1	1.2	42.3 42.3 3.6	-	-	-	-
236 782 858	-	-	-	3.2				-	-	-	-
782 858	-				0.3	-	3.6	E / 7			1
858		-	-	0.0				54.7	9.3	0.3	9.6
	-	_		0.0	0.0	-	0.0	0.0	0.0	0.0	0.0
773			-	9.8	2.5	-	12.3 (8.4+3.8)	51.5 (42.8:92.7)	29.2	2.5	31.7
113	20	596	11	3.2	0.0	0.7	3.9	18.1	10.4	0.0	10.4
161	-	-	-	0.0	0.0	-	0.0	0.0	0.0	0.0	0.0
626	-	-	-	9.9	6.6	-	16.5	95.0	30.4	6.6	37.0
949	-	-	-	0.0	0.0	-	0.0	0.0	0.0	0.0	0.0
465	-	-	-	4.2	0.6	-	4.9 (4.5+0.4)	37.6 (35.9:87.8)	15.6	0.6	16.2
754	243	367	0	0.7	0.0	0.4	1.2 (1.1+0.0)	5.7 (6.8:1.1)	12.1	0.0	12.1
293	-	-	-	0.0	0.0	-	0.0	0.0	0.0	0.0	0.0
0	-	-	-	-	-	-	-	-	-	-	-
0	-	-	-	-	-	-	-	-	-	-	-
0	-	-	-	-	-	-	-	-	-	-	-
	949 465 754 293 0 0 C1	949 - 465 - 754 243 293 - 0 - 0 - C1 Stream: 1 PRC for C1 Stream: 2 PRC for C1 Stream:	949	949	949 0.0 465 4.2 754 243 367 0 0.7 293 0.0 0 0.0 0	949 0.0 0.0 465 4.2 0.6 754 243 367 0 0.7 0.0 293 0.0 0.0 0	949 0.0 0.0 4.2 0.6 4.2 0.6 754 243 367 0 0.7 0.0 0.4 293 0.0 0.0 0	949 0.0 0.0 - 0.0 465 4.2 0.6 - 4.9 (4.5+0.4) 754 243 367 0 0.7 0.0 0.4 1.2 (1.1+0.0) 293 0.0 0.0 - 0.0 0	949 0.0 0.0 - 0.0 0.0 465 4.2 0.6 - 4.9 (35.9:87.8) 754 243 367 0 0.7 0.0 0.4 (1.1+0.0) 5.7 (6.8:1.1) 293 0.0 0.0 - 0.0 0.0 0	949 0.0 0.0 - 0.0 0.0 0.0 465 4.2 0.6 - 4.9 (4.5+0.4) (35.9:87.8) 15.6 754 243 367 0 0.7 0.0 0.4 (1.2 (1.1+0.0) 5.7 (6.8:1.1) 12.1 293 0.0 0.0 - 0.0 0.0 0.0 0	949 0.0 0.0 - 0.0 0.0 0.0 0.0 0.0 0.0

Scenario 8: 'Strategic Model Hawkfield Development 2b - PM' (FG8: 'Strategic Model Hawkfield 2026

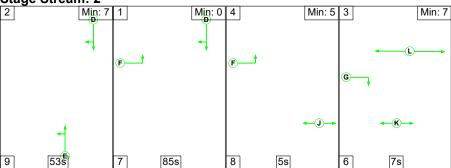
Development 2b - PM', Plan 1: 'Network Control Plan 1')

Stage Sequence Diagram

Stage Stream: 1



Stage Stream: 2



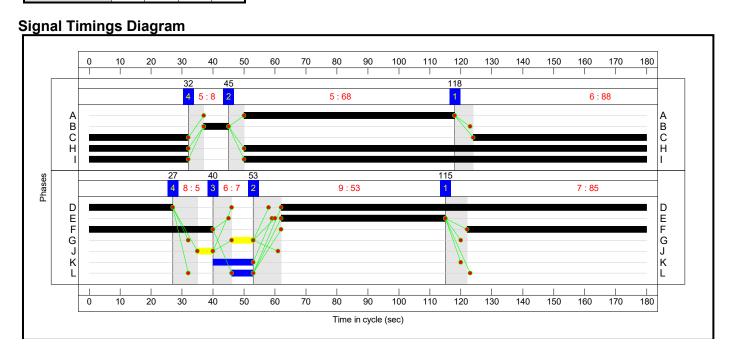
Stage Timings

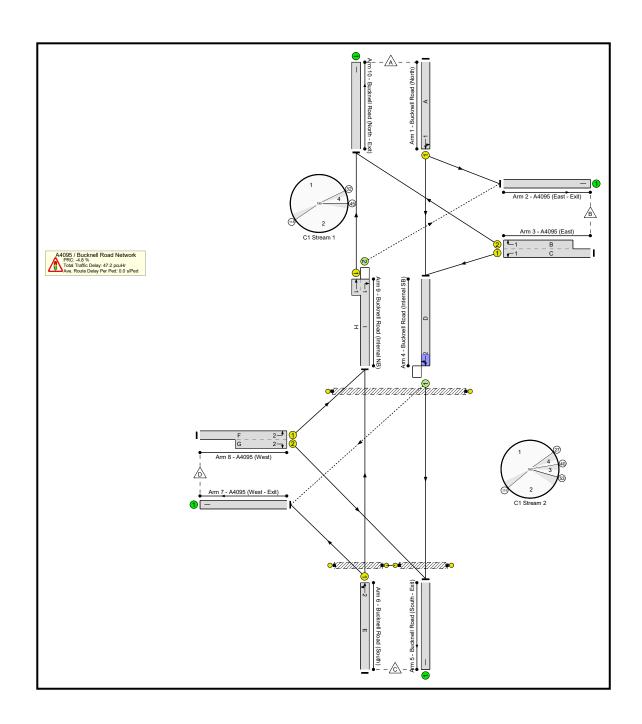
Stage Stream: 1

Stage	2	1	4
Duration	68	88	8
Change Point	45	118	32

Stage Stream: 2

		-	-	
Stage	2	1	4	3
Duration	53	85	5	7
Change Point	53	115	27	40





Item	Lane Description	Lane Type	Controller Stream	Position In Filtered Route	Full Phase	Arrow Phase	Num Greens	Total Green (s)	Arrow Green (s)	Demand Flow (pcu)	Sat Flow (pcu/Hr)	Capacity (pcu)	Deg Sat (%)
Network: A4095 / Bucknell Road	-	-	N/A	-	-		-	-	-	-	-	-	94.3%
A4095 / Bucknell Road Network	-	-	N/A	-	-		-	-	-	-	-	-	94.3%
1/1	Bucknell Road (North) Left Ahead	U	1	N/A	А		1	68	-	242	1907	731	33.1%
2/1	A4095 (East - Exit)	U	N/A	N/A	-		-	-	-	1083	Inf	Inf	0.0%
3/1+3/2	A4095 (East) Left Right	U	1	N/A	СВ		1	88:8	-	869	1747:1888	832+89	94.3 : 94.3%
4/1	Bucknell Road (Internal SB) Ahead Right	0	2	N/A	D		1	145	-	868	1736	955	90.9%
5/1	Bucknell Road (South - Exit)	U	N/A	N/A	-		-	-	-	295	Inf	Inf	0.0%
6/1	Bucknell Road (South) Left Ahead	U	2	N/A	E		1	53	-	531	1898	569	93.3%
7/1	A4095 (West - Exit)	U	N/A	N/A	-		-	-	-	803	Inf	Inf	0.0%
8/1+8/2	A4095 (West) Right Left	U	2	N/A	FG		1	98:7	-	751	1639:1825	888+27	82.1 : 82.1%
9/2+9/1	Bucknell Road (Internal NB) Right Ahead	O+U	1	N/A	IН		1	162	-	1052	1856:1947	1260+175	73.3 : 73.3%
10/1	Bucknell Road (North - Exit)	U	N/A	N/A	-		-	-	-	212	Inf	Inf	0.0%
Ped Link: P1	Pedestrians Crossing Bucknell Road Northbound	-	2	-	К		1	13	-	0	-	0	0.0%
Ped Link: P2	Pedestrians Crossing Bucknell Road Southbound	-	2	-	J		1	5	-	0	-	0	0.0%

Ped Link: P3	Unnamed Ped Link Arriving (pcu)	-	2	-	L		1	7	-	0	-	0	0.0%
		Leaving (pcu)	Turners In Gaps (pcu)	Turners When Unopposed (pcu)	Turners In Intergreen (pcu)	Uniform Delay (pcuHr)	Rand + Oversat Delay (pcuHr)	Storage Area Uniform Delay (pcuHr)	Total Delay (pcuHr)	Av. Delay Per PCU (s/pcu)	Max. Back of Uniform Queue (pcu)	Rand + Oversat Queue (pcu)	Mean Max Queue (pcu)
Network: A4095 / Bucknell Road	-	-	348	1152	19	31.8	14.5	0.8	47.2	-	-	-	-
A4095 / Bucknell Road Network	-	-	348	1152	19	31.8	14.5	0.8	47.2	-	-	-	-
1/1	242	242	-	-	-	2.6	0.2	-	2.9	42.9	8.5	0.2	8.8
2/1	1083	1083	-	-	-	0.0	0.0	-	0.0	0.0	0.0	0.0	0.0
3/1+3/2	869	869	-	-	-	11.2	6.6	-	17.8 (15.2+2.6)	73.7 (69.6:112.5)	38.4	6.6	45.1
4/1	868	868	24	559	12	1.5	0.0	0.6	2.1	8.8	7.0	0.0	7.0
5/1	295	295	-	-	-	0.0	0.0	-	0.0	0.0	0.0	0.0	0.0
6/1	531	531	-	-	-	9.0	5.4	-	14.4	97.8	25.7	5.4	31.1
7/1	803	803	-	-	-	0.0	0.0	-	0.0	0.0	0.0	0.0	0.0
8/1+8/2	751	751	-	-	-	7.2	2.2	-	9.4 (8.8+0.6)	45.1 (43.6:93.9)	30.2	2.2	32.4
9/2+9/1	1052	1052	324	593	7	0.3	0.0	0.3	0.5 (0.5+0.0)	1.8 (2.0:0.6)	2.7	0.0	2.7
10/1	212	212	-	-	-	0.0	0.0	-	0.0	0.0	0.0	0.0	0.0
Ped Link: P1	0	0	-	-	-	-	-	-	-	-	-	-	-
Ped Link: P2	0	0	-	-	-	-	-	-	-	-	-	-	-
Ped Link: P3	0	0	_	_	_	-	_	i -	_	1 -	_	_	_

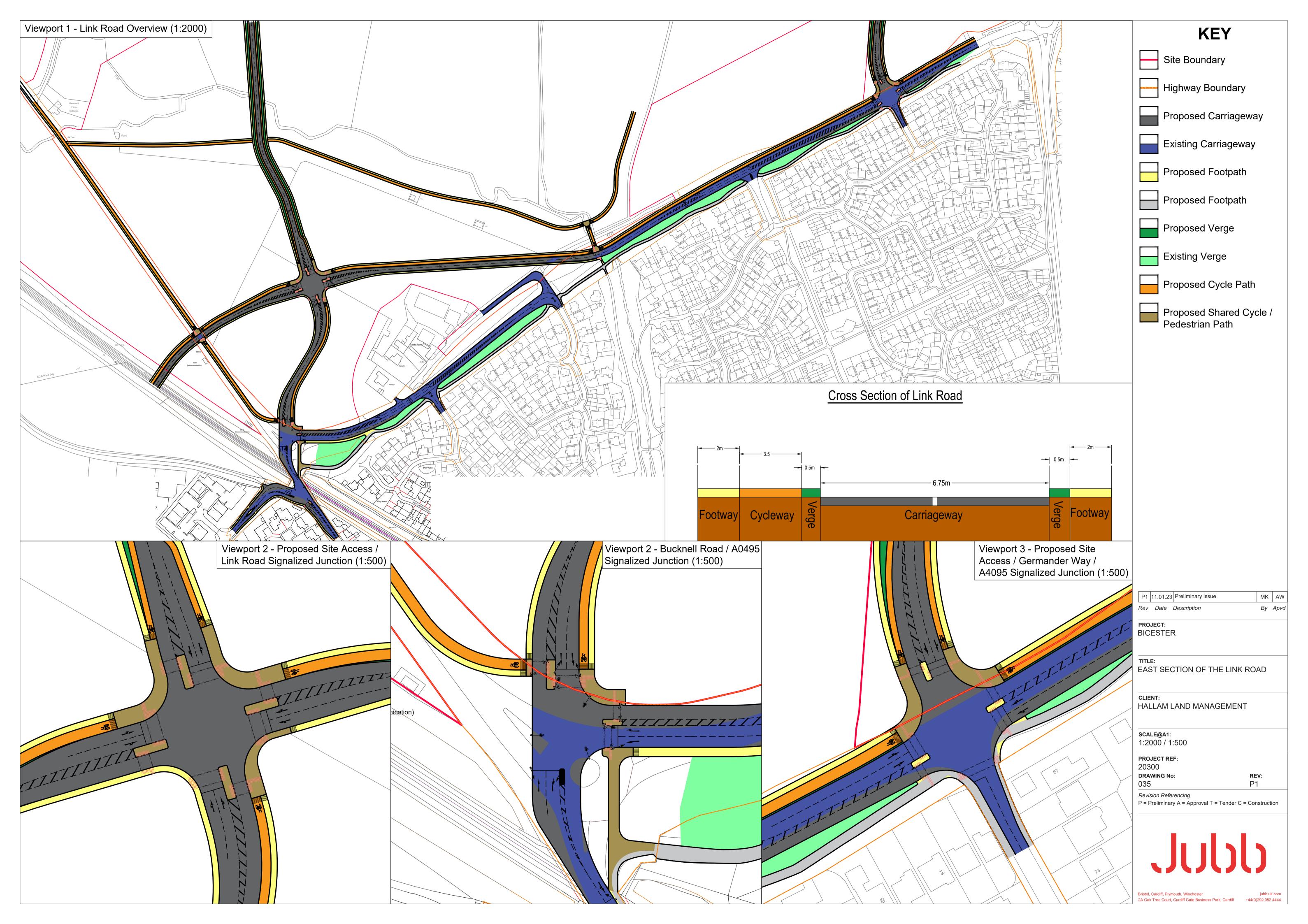
Stream: 2 PRC for Signalled Lanes (%): PRC Over All Lanes (%):

Total Delay for Signalled Lanes (pcuHr): Total Delay Over All Lanes(pcuHr):

^{25.95} 47.16 Cycle Time (s): 180

North West Bicester – Hawkwell Village 20300

Appendix J Strategic Link Road (East of the Railway Line) Proposed Scheme



North West Bicester – Hawkwell Village 20300

Appendix K – Bucknell Road Proposed Traffic Calming Scheme



