

ENVIRONMENT

Lone Star Land Ltd Land off Balmoral Avenue Banbury Sustainable Drainage Statement



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Birmingham Livery Place, 35 Livery Street, Colmore Susiness District, Birmingham, B3 2PB T: 0121 233 3322

> Cambridge 14.16 Pigh Street, Histon, Cambridge CB24 9JD T: 01223 235 173

Whitehall Waterfront, 2 Riverside Way, Leeds
LS1 4EH
T: 0113 233 8000

London 11 Borough High Street London, SE1 9SE T: 0207 407 3879

Manchester 11 Portland Street, Manchester, M1 3HU 0161 233 4260

Market Harborough 12a Woodcock House, Compass Point Market Harborough, Leicestershire, LE16 9HW T: 01858 455020

> Nottingham Waterfront House, Station Street, Nottingham NG2 3DQ T: 0115 924 1100

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P01	08.10.21	\$2	Ryan Davies BSc (Hons), MSc	Rowan Jobling BEng (Hons)	Catherine Thorpe BSc (Hons) MCInstCES



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1. INTRODUCTION

- 1.1 A Sustainable Drainage Statement (SDS) sets out the principles of drainage design for a development and summarises the reasoning behind the chosen design. This includes consideration of national and local guidance, justification of specific flow rates, volumes of attenuated storage, as well as the appropriate level of treatment to be provided to surface water runoff.
- 1.2 This SDS has been produced by BWB Consulting on behalf of Lone Star Land Ltd in respect of a site at Land off Balmoral Avenue, Banbury.
- 1.3 A Flood Risk Assessment (FRA) has been produced for the site (ref: BP2-BWB-ZZ-XX-RP-YE 0001 FRA) and this SDS accompanies this overarching document.
- 1.4 This SDS is intended to support an outline planning application and as such the level of detail included is commensurate and subject to the nature of the proposals. A proposed site development plan is included as **Appendix 1**.
- 1.5 The greenfield site is located to the western extent of Banbury, Oxfordshire approximately 2km west of the town centre. As shown in **Figure 1.1**, the site is split into the indicative site boundary and the wider site ownership. Although the client is in ownership of the entire site, development is limited to the indicative site boundary. The rest of this report will concern only the indicative site boundary.
- 1.6 The greenfield site is surrounded by residential development to the north, the B4035 (Broughton Road) to the south and greenfield agricultural land to the east and west. The parcel of greenfield land to the north-east has already been granted outline planning permission (ref: 20/01643/OUT) for the construction of up to 49 units. Further contextual information provided within **Table 1.1**.
- 1.7 The proposed development is understood to comprise the erection of 49 dwellings, public open space, and associated infrastructure.



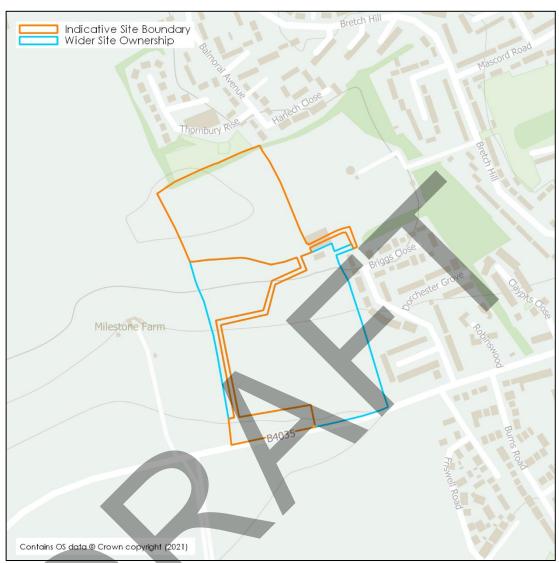


Figure 1.1: Site Location

Table 1.1: Site Details

Site Name	Land off Balmoral Avenue
Location	Banbury
NGR (approx.)	SP 4381 3982
Application Site Area (ha)	3.15 (Approx.)
Development Area (ha)	1.42 (Approx.)
Development Type	Residential
Lead Local Flood Authority	Oxfordshire County Council
Local Planning Authority	Cherwell District Council
Sewerage Undertaker	Thames Water



Sustainable Drainage Guidance

- 1.8 Sustainable Drainage Systems (SuDS) aim to reduce the impact of development by replicating the natural runoff regime in a sustainable, cost-effective manner, whilst protecting water quality and reducing pollution. The four key objectives of SuDS design are to achieve improvements in water quantity, water quality, amenity provision and biodiversity.
- 1.9 Oxfordshire County Council (OCC) have produced local SuDS guidance¹ which new development must abide by. The more pertinent requirements for an outline planning application are outlined below:
 - i. New development should restrict runoff rates to the equivalent greenfield runoff rate for events up to and included the 1 in 100-year event plus an allowance for climate change.
 - ii. An additional 10% allowance in impermeable surface area for urban creep should be made for all developments.
 - iii. Storage features must be designed to accommodate the 1 in 100-year event plus a 40% allowance for climate change.
 - iv. 300mm of freeboard must be provided for above ground storage features.
 - v. A minimum allowable discharge of 51/s does not apply.
 - vi. Consideration of the drainage hierarchy must be made in the selection of an appropriate outfall.
- 1.10 Alongside the local guidance, the Non-Statutory Technical Standards for Sustainable Drainage Systems² as published by DEFRA have been utilised to inform the strategy.
- 1.11 A 40% allowance for the potential implications of climate change has been applied, in accordance with local guidance and the Environment Agency's (EA) guidance³ (most recently updated in July 2021).

¹ Oxfordshire County Council Local Standards and Guidance for Surface Water Drainage on Major Development in Oxfordshire (Oxfordshire County Council, November 2018)

² 2015, DEFRA. Non-statutory technical standards for sustainable drainage systems

³Environment Agency, Flood risk assessments: climate change allowances: https://www.gov.uk/guidance/flood-risk-assessments-climate-change-allowances



2. EXISTING CONDITIONS

- 2.1 A topographical survey has been undertaken across the site and is included as **Appendix 2**. Across the main development site to the north, the survey demonstrates that levels fall relatively steeply towards the south and east, with maximum and minimum levels ranging between approximately 161.0 m Above Ordnance Datum (AOD) to 152.2m AOD. Between where the site boundary forms a corridor through the wider ownership towards the southern boundary levels fall steeply from 153.0m AOD to 129.1m AOD.
- 2.2 Mapping of bedrock geology provided by the British Geological Survey (BGS), demonstrates that the site is underlain by four different bedrock geologies: 'Chipping Norton Limestone Formation', 'Horsehay Sand Formation', 'Northampton Sand Formation' and predominately by 'Whitby Mudstone Formation', as shown in **Figure 2.1**. No superficial deposits are shown as being recorded at the site.

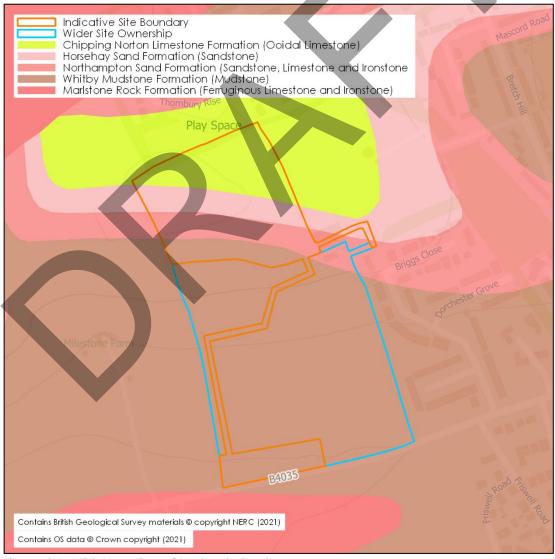


Figure 2.1: BGS Mapping of Bedrock Geology



- 2.3 Thames Water are the sewerage undertakers for the area. A copy of their asset plans has been included as **Appendix 3**. The records demonstrate the presence of a sewer network along Balmoral Avenue to the north and east. Both surface water and foul water sewers are shown, with pipe diameters for the former shown to range from 150-300mm, with diameters of the latter ranging from 100-225mm.
- 2.4 A ditch can also be seen adjacent to the southern boundary of the site, where Thames Water sewer records show 300mm surface water sewers outfalling into via a headwall.
- 2.5 Due to the site's existing greenfield condition, it is not expected to be positively drained. Infiltration is therefore expected to provide the primary means of surface water drainage for the site, which becomes overland flows when capacity of the soils is exceeded. Overland flows are expected to drain southwards in accordance with the site's topography before being received by the ditch along the southern boundary.
- 2.6 The site does not lie within a source Protection Zone.

Existing Runoff Rates

2.7 An assessment of the existing surface water runoff rates from the area proposed for development has been undertaken and is summarised within **Table 2.1**. This has been measured from the masterplan to comprise an area of 1.42ha. Calculations are included within **Appendix 4**.

The runoff rates have been estimated using the IH124 method, with appropriate prorated adjustments for a site of less than 50ha, as recommended in Interim Code of Practice for Sustainable Drainage⁴. This was undertaken within Micro Drainage, which makes the necessary adjustments for small sites automatically.

Table 2.1: Existing Runoff Rate from the Developable Area of the Site

Return Period (Yrs.)	Runoff Rate (I/s)
	4.0
Mean Annual Flow Rate (QBAR)	4.8
30	10.8
100	15.2

⁴ The National SUDS Working Group (2004), Interim Code of Practice for Sustainable Drainage



Existing Runoff Volume

- 2.8 An assessment of the existing surface water runoff rates from the area proposed for development has been made for a 1 in 100-year, 6-hour storm.
- 2.9 As the existing site is permeable, the runoff volume has been calculated using the Source Control module within Micro Drainage to be **383.03m³**, results are included within **Appendix 5**.





3. SURFACE WATER DRAINAGE STRATEGY

Drainage Hierarchy

- 3.1 The Planning Policy Guidance⁵ and the SuDS Manual⁶ identify that surface water runoff from a development should be disposed of as high up the following hierarchy as reasonably practicable:
 - i. into the ground (infiltration);
 - ii. to a surface water body;
 - iii. to a surface water sewer, highway drain, or another drainage system;
 - iv. to a combined sewer.
- 3.2 The aim of this is approach is to manage surface water runoff close to where it falls and mimic natural drainage as closely as possible.

<u>Infiltration</u>

- 3.3 Mapping of bedrock geology demonstrates that the majority of the site including the central and southern portion are underlain by mudstone of the Whitby Mudstone Formation.
- 3.4 Due to the topography of the site, the most practical location to locate an attenuation would be towards the south of the site. As this area being underlain by mudstone, soakaways are not considered to be a feasible option.

Discharge to a Watercourse

- 3.5 Thames Water sewer records demonstrate that 300mm surface water sewers outfall into a ditch along the southern boundary of the site, via a headwall
- 3.6 Given its proximity and its position at the low point of the site, this ditch is considered to be the most appropriate discharge location, with runoff rates being limited to the equivalent greenfield (QBAR) rate. The site is considered to naturally drain towards this feature when capacity of the soils no longer permit infiltration. As such, this is considered to mimic the existing drainage regime.
- 3.7 In order to perform works in the vicinity of an ordinary watercourse, it may be necessary to acquire Land Drainage Consent (Land Drainage Act, 1991) from the LLFA.

⁵ Planning Practice Guidance. http://planningguidance.planningportal.gov.uk/.

⁶ The SuDS Manual (C753). CIRIA 2015.



Peak Flow Control

- 3.8 In order to comply with the Non-Statutory Technical Standards for Sustainable Drainage Systems S2-S3⁷, runoff from greenfield developments should not exceed the equivalent greenfield rates for all events up to 100-year return period plus climate change event.
- 3.9 Local guidance requires the runoff rate from all new development on greenfield sites to be controlled at the greenfield (QBAR) rate during events up to and including the 1 in 100-year plus climate change event.
- 3.10 To comply with the peak flow control criterion, it is proposed to restrict all rainfall events to the equivalent greenfield QBAR rate up to the 1 in 100-year plus climate change event. This is summarised within **Table 3.1**.

Table 3.1: Existing & Proposed Runoff Rates

Return Period (Yr.)	Existing Runoff Rate (I/s)	Proposed Discharge Rate (I/s)
1	4.0	
QBAR	4.8	
30	10.8	4.8
100	15.2	
100 + 40%	\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	

Attenuated Storage

- 3.11 As the development proposals require a restricted runoff rate, it will be necessary to provide attenuated storage to balance the excess volume in a safe manner within the site.
- 3.12 The surface water storage should be located within the site in a position where it can receive runoff from the development and discharge from the site by gravity, and also in a position where it is hydraulically isolated from any fluvial floodplain or external surface water floodplain/ overland flow route that may be present in the site.
- 3.13 Sufficient storage for events up to the 1 in 100-year storm with an allowance for climate change should be provided. A 10% allowance should be applied to the current proposed impermeable area to allow for urban creep over the lifetime of the development.
- 3.14 After considering the site constraints and development aspirations it is suggested that the necessary surface water storage volume is found within a detention basin located at the lowest elevation of the site, between the proposed development and the outfall location.

⁷ 2015, DEFRA. Non-statutory technical standards for sustainable drainage systems



- 3.15 For the purposes of this outline assessment, it has been assumed that the basin will accommodate all of the necessary storage, but it may be possible to redistribute a portion of the storage within other drainage components during the detailed design of the development (e.g.: in the pipe network, swales, filter drains, etc).
- 3.16 A simulation has been run using Micro Drainage 'Source Control' to identify the necessary storage provision. A developable area of 1.42ha has been measured from the indicative site layout plan. It has been assumed that impermeable surfaces will form 65% of the developable area (1.42ha x 0.65 = 0.92ha).
- 3.17 Using a restriction of 4.81/s and applying a 10% allowance to the proposed impermeable area (0.92ha x 10% = 1.0ha), the volume of attenuated storage required for the development has been calculated for storm events up to the 100 year + 40% storm. The results are summarised in **Table 3.2** and calculations are included as **Appendix 6**.

Table 3.2: Outline Attenuated Storage Requirements

Rainfall Method	Critical Storm	Maximum Volume (m³)
FSR	720 min (Winter)	666.1
FEH	960 min (Winter)	774.2

- 3.18 The FEH rainfall distribution is shown to provide a more conservative estimate of the minimum attenuation storage required. Therefore, at this outline design stage, it is expected that a minimum of 774.2m³ of attenuated storage will be provided to cater for the maximum anticipated runoff volume for all storm durations up to the 1 in 100-year return period storm, including a 40% climate change allowance and future urban creep.
- 3.19 It is envisaged that the final required affenuated storage volume will be determined during the detailed design stage, once the development layout and drainage areas are fixed.

Runoff Volume Control

- 3.20 The Non-Statutory Technical Standards for Sustainable Drainage Systems \$4-\$68 states that where reasonably practical the runoff volume from a development for the 1 in 100-year 6-hour rainfall event should not exceed the runoff volume prior to development or redevelopment. Where it is not reasonably practicable to constrain the volume of runoff from a development at or below the existing volume, then the runoff must be discharged in a manner that does not adversely affect flood risk, i.e.:
 - i. The additional runoff volume resulting from the development (the 'long term storage volume') should be discharged separately from the site at a rate of 2 l/s/ha or less. Or,
 - ii. All the runoff volume from the development should be discharged at a rate equivalent to the mean annual flow rate (QBAR) rate under greenfield conditions or less. Or,

^{8 2015,} DEFRA. Non-statutory technical standards for sustainable drainage systems



- iii. All the runoff volume from the development should be discharged at a rate of 21/s/ha or less.
- 3.21 An estimate of the post-development runoff volume from the 1 in 100-year 6-hour storm can be derived from the Micro Drainage calculations, as provided within the storage calculations under the 360 min storm within **Appendix 6**. The existing and post-development runoff volumes for the developable areas of the site inclusive of urban creep (1.42ha), are compared within **Table 3.3**
- 3.22 As the entirety of the developable area is not proposed to be impermeable, the existing runoff volume has been prorated to account for the remaining permeable areas of the site. Inclusive of urban creep, the remaining permeable areas comprise an area of 0.42ha (1.42ha 1.0ha = 0.42ha). This equates to a runoff volume of 113.0m³ when prorated form the existing volume.

Table 3.3: Runoff Volume Comparison

Existing Volume (m³)	Proposed Volume (m³)	Difference (m³)
383.03	872.30	+488.97

3.23 The 1 in 100-year 6-hour storm runoff volume from the site has been shown to increase as a result of the proposed development. However, as the runoff volume from the development will be discharged at a rate equivalent to the mean annual flow rate (QBAR) rate under greenfield conditions, the volume control criteria will be met.

Long Term Storage

3.24 It is proposed to discharge the runoff from the development at a rate equivalent to the mean annual flow rate (QBAR) rate under greenfield conditions or less. Therefore, provision for long term storage is not required.

Sustainable Drainage Systems

- 3.25 An Illustrative Drainage Strategy is included as **Appendix 7** (ref: BP2-BWB-ZZ-XX-CD-DR-0001), which demonstrates how the above requirements will be incorporated into the masterplan.
- 3.26 The proposed dráinage strategy should be designed with the four pillars of SuDS in mind: amenity, biodiversity, water quality and water quantity. The treatment stages should be determined by the perceived hazard level of the runoff source and in accordance with national guidance.
- 3.27 After considering the site constraints and development aspirations, the necessary surface water storage volume is proposed to be provided within a detention basin.
- 3.28 The basin will provide one level of treatment to surface water prior to discharge. A second level of surface water treatment should be provided to trafficked areas, for example by a forebay within the detention basin, permeable paving or silt traps.



- 3.29 It is proposed for the basin to be deepened and lined to allow a waterbody to permanently establish itself. It is expected that this will enhance the provision of treatment provided by the feature, as well as providing additional biodiversity benefits.
- 3.30 The interception value (the first 5mm of runoff in a rainfall event), should be appropriately treated prior to release into the downstream network to prevent contamination from high pollutant concentrations. This will be achieved in part by soakage from the detention basin.
- 3.31 The detention basin has been designed to have an internal bank slope gradient of 1:3 to allow for maintenance. The detention basin should be appropriately planted to promote further treatment and be unlined, to promote localised infiltration. Planting of vegetation will also serve to improve aesthetic quality, providing amenity benefits.
- 3.32 The basins will also provide biodiversity benefits through the provision of habitat and food sources for wildlife.

Residual Risk and Designing for Exceedance

- 3.33 In line with OCC's local guidance, a minimum 300mm of freeboard has been accounted for in the sizing of the detention basin. The basin has been designed to provide this freeboard during the 1 in 100-year plus 40% climate change event.
- 3.34 It is recommended that the final layout uses the proposed road infrastructure to provide drainage exceedance (overland flood flow) routes through the development and towards Balmoral Avenue to the east, where highway drainage can intercept the excess flow.
- 3.35 In addition to the volume of storage provided within the main attenuation, there will be capacity within upstream pipes and manholes which has not been accounted for at this stage and a further level of redundancy to the network will therefore be provided.



4. MAINTENANCE

- 4.1 The drainage network should be constructed in accordance with the Design and Construction Guidance and proposed for adoption by Thames Water under a Section 104 agreement. Any elements not adopted should be maintained by a management company or homeowners.
- 4.2 Requirements for ongoing maintenance of the drainage network should form part of the Operation and Maintenance manual for the site and should be undertaken by the site management. Any specialist or proprietary products that are specified at detailed design should have a manufacturer specific maintenance regime which should be included within the document.
- 4.3 It is envisaged that the Operation and Maintenance manual will be developed at the detailed design stage, but some examples are included below.
 - i. All drainage features should be located in open areas which are readily accessible.
 - ii. Gullies should be inspected and de-silted at least once a year, where necessary.
 - iii. Pipes, manholes and silt traps should be inspected and de-silted at least once a year, where necessary.
 - iv. If permeable paving is incorporated within the layout, it should be swept a minimum of every six months to maintain flow capacity of the joints between blocks.
 - v. The surface water attenuation areas will be predominantly dry and the base will be seeded with a wildflower grass seed mix that can tolerate wet ground conditions.
 - vi. Regular inspections of the attenuation basin should be undertaken to remove litter/debris, invasive/colonising vegetation and silt build up as necessary. Inlet and outlet structures to be regularly inspected, with remedial work as required to maintain water flows and prevent silt/vegetation build up.
 - vii. Vegetation/grass with the attenuation basin should be maintained appropriately to allow establishment and promote habitat formation, without impeding the operation of the inlet and outlet structure.
 - viii. Flow controls should be inspected every six months, litter/debris and silt build up should be removed as necessary.



5. FOUL WATER DRAINAGE

- 5.1 It is proposed to drain used water from the development separately to surface water. An indicative foul sewer network has been provided within the Illustrative Drainage Strategy (Appendix 7)
- 5.2 Thames Water sewer records indicate the presence of 150mm foul sewers along Balmoral Avenue to the east. In view of the levels between the site and the existing sewerage assets, a gravity connection is proposed to convey flows from the development towards foul sewers located along Balmoral Avenue to the east (nearest manhole ref: 8801).
- 5.3 A Developer's Enquiry has been made to Thames Water; their response is included within **Appendix 3**. They confirm that there is currently only capacity to accept flows from 30 properties, but that further sewer modelling and upgrade works would be required before the remainder of the development could be accepted. Thames Water state that although they would front the cost for these works, the site would need to be accepted for either outline or full planning application before work could commence.
- 5.4 A \$106 (Water Industry Act 1991) application should be made to Thames Water for approval prior to the connection being made.





6. SUMMARY

- 6.1 This statement and supporting appendices demonstrate that the drainage design for the development will comply with the relevant local and national standards, specifically the hierarchy of discharge, runoff rate and volume criterion.
- 6.2 This SDS is intended to support an outline planning application and as such the level of detail included is commensurate and subject to the nature of the proposals.

Table 6.1: Sustainable Drainage Statement Summary

Table 6.1: Sustainable Drainage Statement Summary				
		Existing Site	Proposed Development	
Site Area (ha)		3.15		
Imperm	eable Area (ha)	0	1.0	
Outfall I	ocation	Infiltration/Watercourse	Watercourse	
* =	QBAR	4.8		
Peak Runoff Rate (I/s)	1 in 30-Year	10.8	4.8	
eak Rate	1 in 100-Year	15.2	4.0	
	1 in 100-Year + CC			
Runoff Volume (100yr RP 6 hour Storm)		383.03m³	872.30m ³	
Volume	Contro!		Discharge rate limited to QBAR	
Propose	ed Storage Volume	-	774.2m³	
Interception Volume		-	5m³	
Flow Co	ontrol Type	-	Vortex	
SuDS Fe	atures	-	Detention Basin	
Mainter	nance Responsibility	-	Management Company Sewerage Company	

6.3 It is envisaged that the final drainage strategy will be determined during the detailed design stage, as the development layout is finalised.



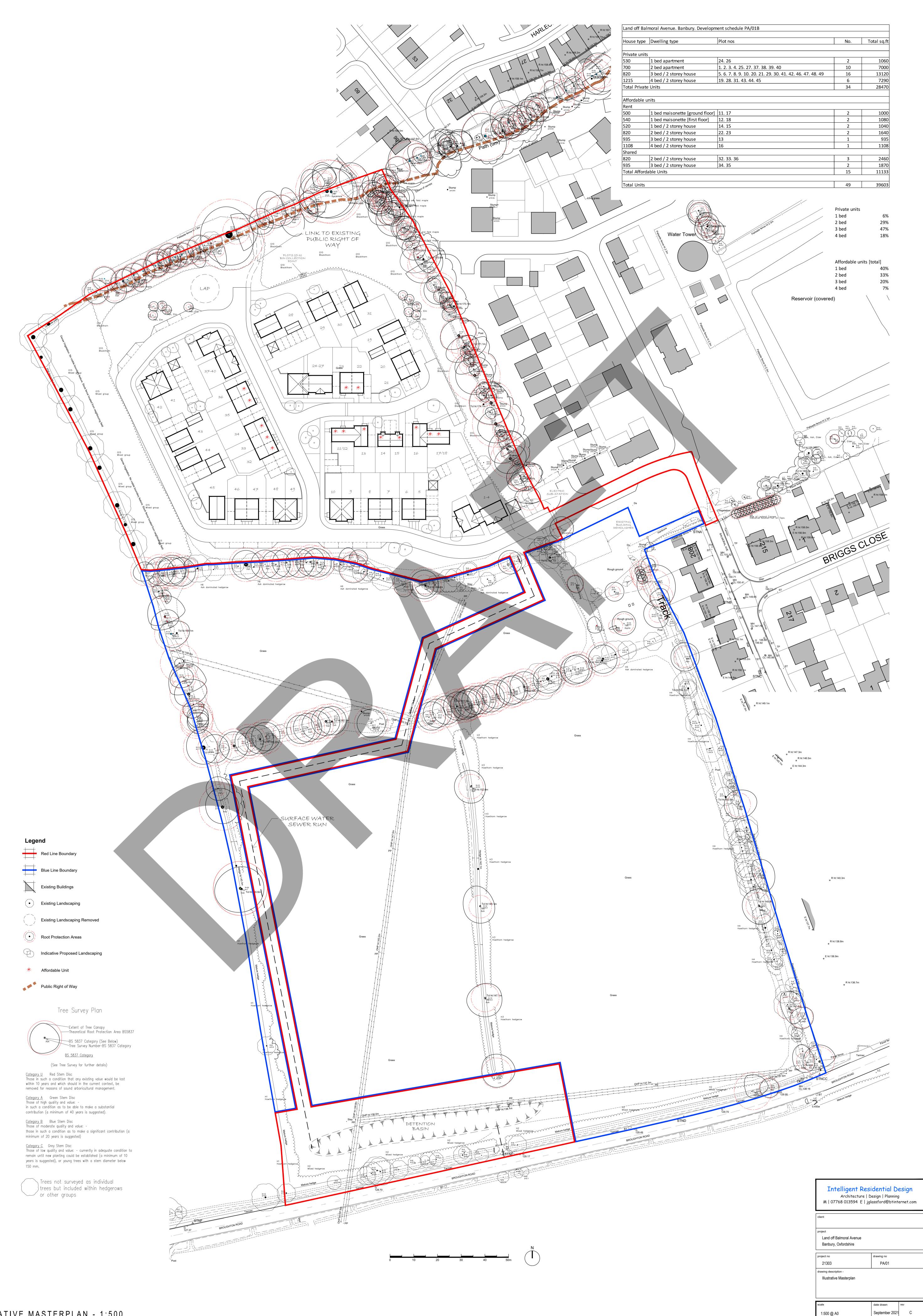






Appendix 1: Indicative **Site Layout** Plan



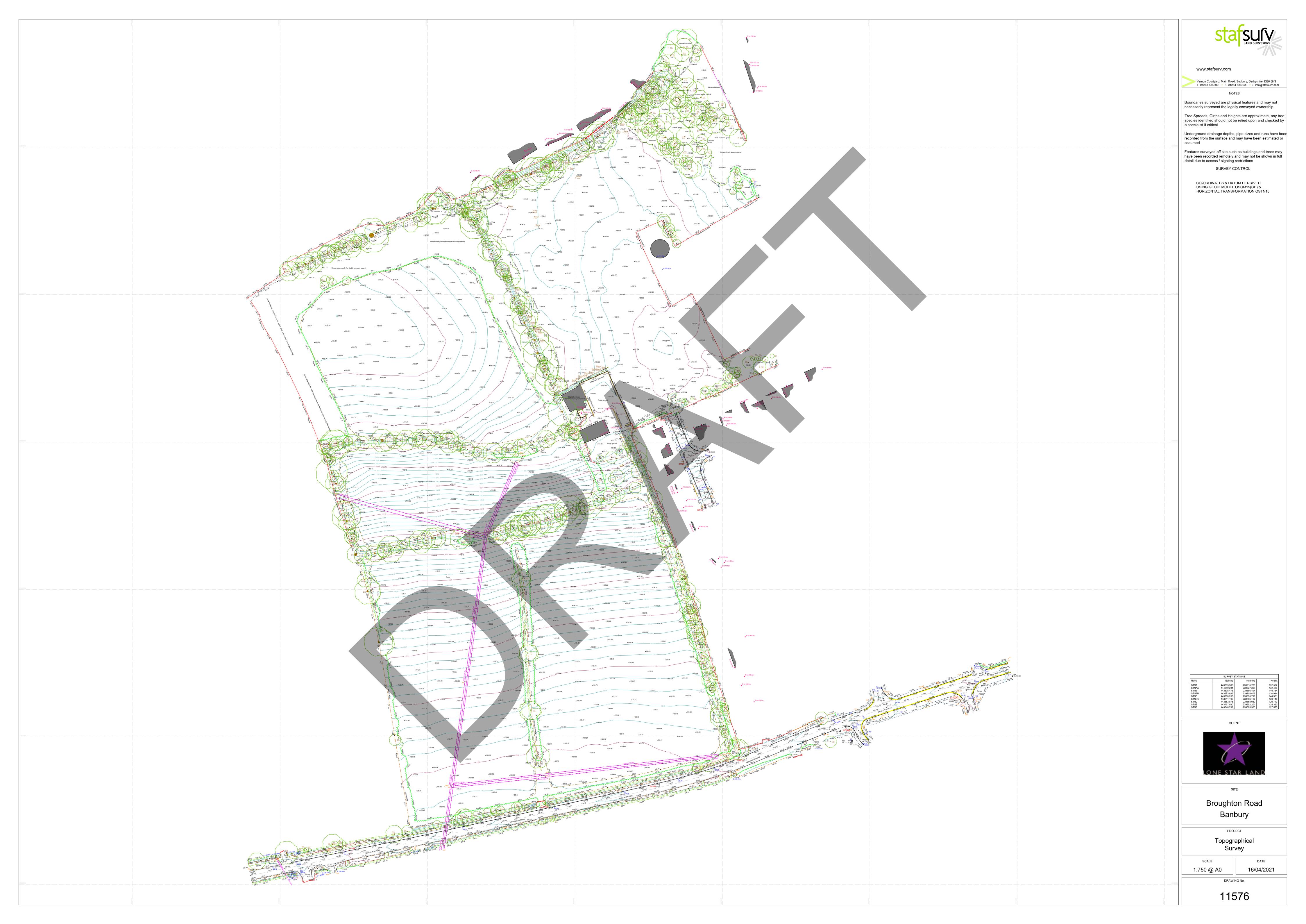






Appendix 2: Topographical Survey









Appendix 3: Thames Water Correspondence



Asset location search



BWB Consulting Limited
5th Floor, Waterfront House Waterfront House

NOTTINGHAM NG2 3DQ

Search address supplied Land North Of Broughton Road

Balmoral Avenue

Banbury OX16 0BG

Your reference BMW3250

Our reference ALS/ALS Standard/2021_4406584

Search date 30 April 2021

Knowledge of features below the surface is essential for every development

The benefits of this knowledge not only include ensuring due diligence and avoiding risk, but also being able to ascertain the feasibility of any development.

Did you know that Thames Water Property Searches can also provide a variety of utility searches including a more comprehensive view of utility providers' assets (across up to 35-45 different providers), as well as more focused searches relating to specific major utility companies such as National Grid (gas and electric).

Contact us to find out more.



Thames Water Utilities Ltd Property Searches, PO Box 3189, Slough SL1 4WW DX 151280 Slough 13



searches@thameswater.co.uk www.thameswater-propertysearches.co.uk





Search address supplied: Land North Of Broughton Road, Balmoral Avenue, Banbury, OX16 0BG

Dear Sir / Madam

An Asset Location Search is recommended when undertaking a site development. It is essential to obtain information on the size and location of clean water and sewerage assets to safeguard against expensive damage and allow cost-effective service design.

The following records were searched in compiling this report: - the map of public sewers & the map of waterworks. Thames Water Utilities Ltd (TWUL) holds all of these.

This searchprovides maps showing the position, size of Thames Water assets close to the proposed development and also manhole cover and invert levels, where available.

Please note that none of the charges made for this report relate to the provision of Ordnance Survey mapping information. The replies contained in this letter are given following inspection of the public service records available to this company. No responsibility can be accepted for any error or omission in the replies.

You should be aware that the information contained on these plans is current only on the day that the plans are issued. The plans should only be used for the duration of the work that is being carried out at the present time. Under no circumstances should this data be copied or transmitted to parties other than those for whom the current work is being carried out.

Thames Water do update these service plans on a regular basis and failure to observe the above conditions could lead to damage arising to new or diverted services at a later date.

Contact Us

If you have any further queries regarding this enquiry please feel free to contact a member of the team on 0800 009 4540, or use the address below:

Thames Water Utilities Ltd Property Searches PO Box 3189 Slough SL1 4WW

Email: searches@thameswater.co.uk

Web: www.thameswater-propertysearches.co.uk



Waste Water Services

Please provide a copy extract from the public sewer map.

The following quartiles have been printed as they fall within Thames' sewerage area:

SP4339NE SP4340SE SP4439NW SP4440SW

Enclosed is a map showing the approximate lines of our sewers. Our plans do not show sewer connections from individual properties or any sewers not owned by Thames Water unless specifically annotated otherwise. Records such as "private" pipework are in some cases available from the Building Control Department of the relevant Local Authority.

Where the Local Authority does not hold such plans it might be advisable to consult the property deeds for the site or contact neighbouring landowners.

This report relates only to sewerage apparatus of Thames Water Utilities Ltd, it does not disclose details of cables and or communications equipment that may be running through or around such apparatus.

The sewer level information contained in this response represents all of the level data available in our existing records. Should you require any further Information, please refer to the relevant section within the 'Further Contacts' page found later in this document.

For your guidance:

- The Company is not generally responsible for rivers, watercourses, ponds, culverts or highway drains. If any of these are shown on the copy extract they are shown for information only.
- Any private sewers or lateral drains which are indicated on the extract of the public sewer map as being subject to an agreement under Section 104 of the Water Industry Act 1991 are not an 'as constructed' record. It is recommended these details be checked with the developer.

Clean Water Services

Please provide a copy extract from the public water main map.

The following quartiles have been printed as they fall within Thames' water area:



SP4339NE SP4340SE SP4439NW SP4440SW

Enclosed is a map showing the approximate positions of our water mains and associated apparatus. Please note that records are not kept of the positions of individual domestic supplies.

For your information, there will be a pressure of at least 10m head at the outside stop valve. If you would like to know the static pressure, please contact our Customer Centre on 0800 316 9800. The Customer Centre can also arrange for a full flow and pressure test to be carried out for a fee.

For your guidance:

- Assets other than vested water mains may be shown on the plan, for information only.
- If an extract of the public water main record is enclosed, this will show known public
 water mains in the vicinity of the property. It should be possible to estimate the
 likely length and route of any private water supply pipe connecting the property to
 the public water network.

Payment for this Search

A charge will be added to your suppliers account.



Further contacts:

Waste Water queries

Should you require verification of the invert levels of public sewers, by site measurement, you will need to approach the relevant Thames Water Area Network Office for permission to lift the appropriate covers. This permission will usually involve you completing a TWOSA form. For further information please contact our Customer Centre on Tel: 0845 920 0800. Alternatively, a survey can be arranged, for a fee, through our Customer Centre on the above number.

If you have any questions regarding sewer connections, budget estimates, diversions, building over issues or any other questions regarding operational issues please direct them to our service desk. Which can be contacted by writing to:

Developer Services (Waste Water)
Thames Water
Clearwater Court
Vastern Road
Reading
RG1 8DB

Tel: 0800 009 3921

Email: developer.services@thameswater.co.uk

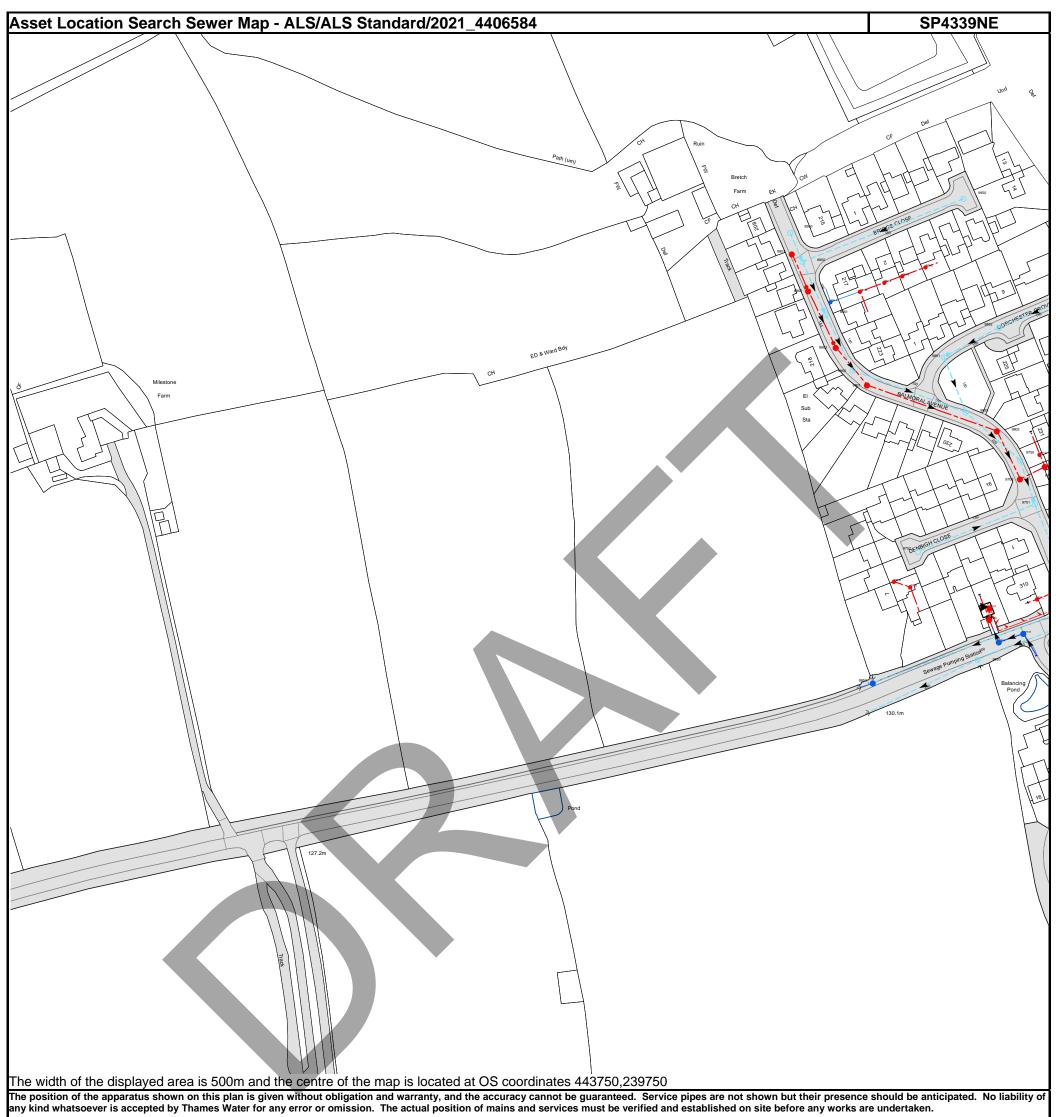
Clean Water queries

Should you require any advice concerning clean water operational issues or clean water connections, please contact:

Developer Services (Clean Water)
Thames Water
Clearwater Court
Vastern Road
Reading
RG1 8DB

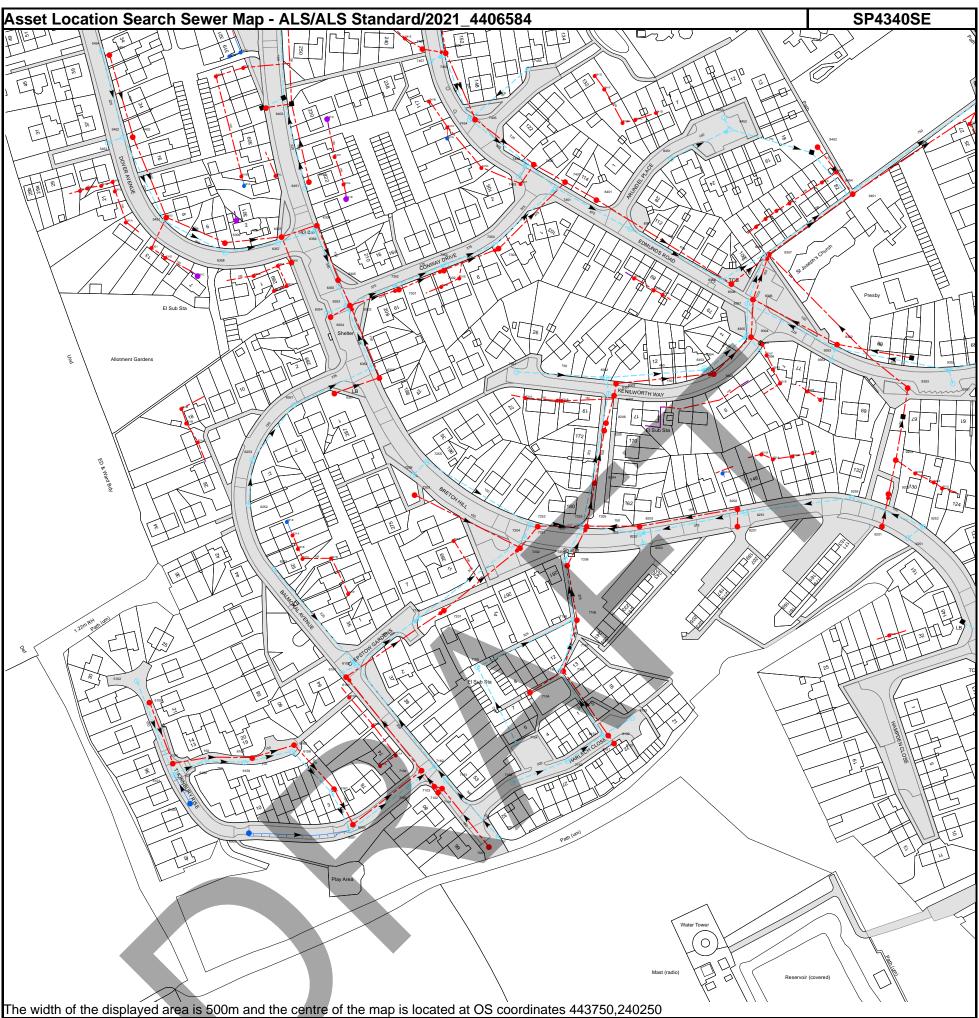
Tel: 0800 009 3921

Email: developer.services@thameswater.co.uk



Manhole Reference	Manhole Cover Level	Manhole Invert Level
8950	151.53	149.6
8801	150.41	148.56
8850	149.62	146.79
8802	147.72	145.16
8851	145.85	143.3
881A	n/a	n/a
8803	143.5	140.68
9850	141.9	139.57
981D	n/a	n/a
9801	140.05	137.4
981A	n/a	n/a
981B	n/a	n/a
981C	n/a	n/a
9851	140.68	138.25
9950	150.32	148.72
9853	137.31	135.24
9802	136.2	134.96
9852	142.69	141.06
971A	n/a	n/a
9652	130.41	129.76
971D	n/a	n/a
971E	n/a	n/a
9752	132.58	131.58
9650	130.6	129.97
971G	131.18	128.94
971F	131.3	128.9
9711	n/a	n/a
9750	135.08	133.26
9703	134.76	134
971H	n/a	n/a
9751	133.92	130.76
971C	n/a	n/a
9702	134.67	133.81
971B	n/a	n/a

The position of the apparatus shown on this plan is given without obligation and warranty, and the accuracy cannot be guaranteed. Service pipes are not shown but their presence should be anticipated. No liability of any kind whatsoever is accepted by Thames Water for any error or omission. The actual position of mains and services must be verified and established on site before any works are undertaken.



The width of the displayed area is 500m and the centre of the map is located at OS coordinates 443750,240250

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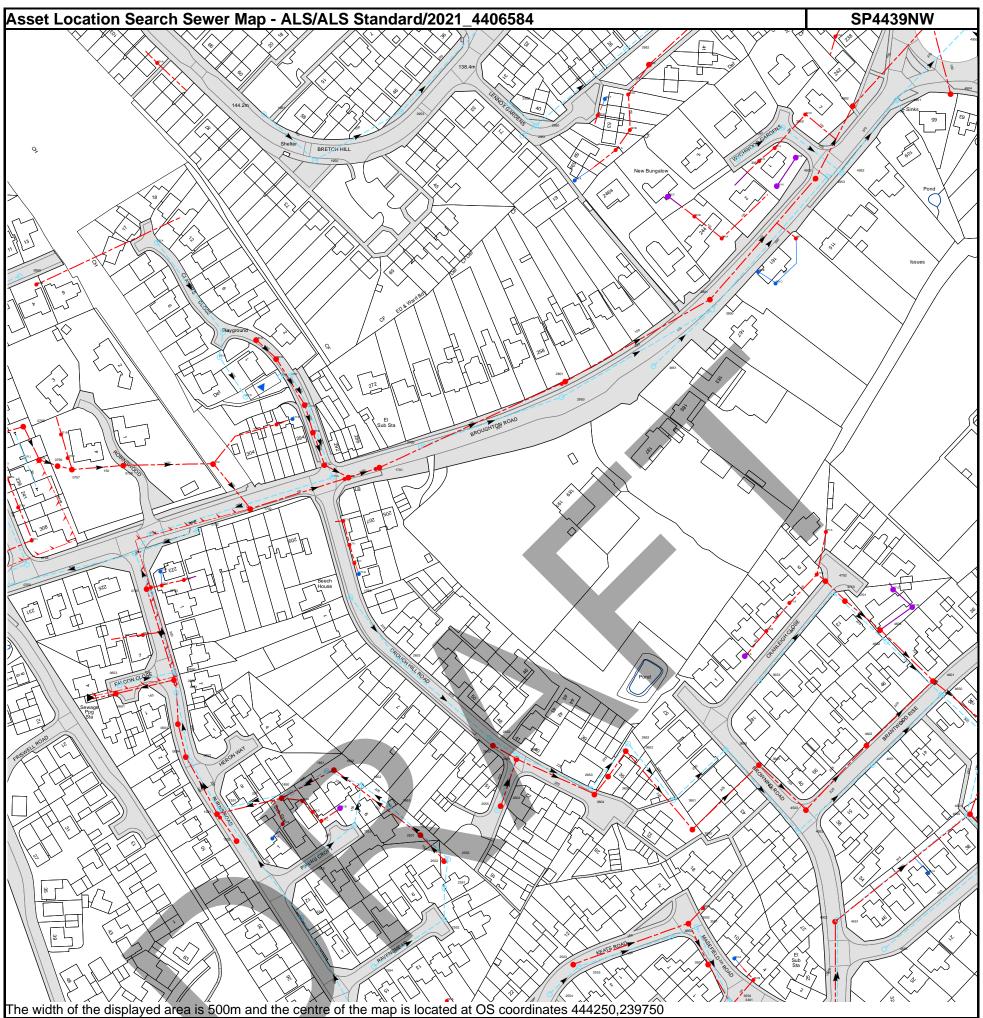
Based on the Ordnance Survey Map with the Sanction of the controller of H.M. Stationery Office, License no. 100019345 Crown Copyright Reserved.

Manhala Deference	Manhala Cayar Layal	Manhala Invest Laval
Manhole Reference 841F	Manhole Cover Level	Manhole Invert Level
841F 841G	n/a n/a	n/a n/a
7455	134.52	132.66
741B	n/a	n/a
7457 7456	136.25 136.19	134.16 131.25
7406	136.07	131.04
7407	136.31	134.15
741E 641P	n/a n/a	n/a
8358	n/a 130.09	n/a 126.95
8307	130.03	127.44
7303	137.07	133.9
7354 8361	137.15 132.88	133.97 129.97
831A	n/a	n/a
841A	n/a	n/a
841B 941A	n/a n/a	n/a n/a
8401	133.82	130.12
7451	134.71	130.3
741F 7401	n/a 134.43	n/a 130.29
7453	135.58	133.57
741D	n/a	n/a
7402	135.28	130.64
7452 8451	135.46 132.23	130.63 130.74
741A	n/a	n/a
8452	131.33	128.85
741C 841E	n/a n/a	n/a n/a
841C	n/a n/a	n/a n/a
7405	136.14	130.92
7454	136.22	131.05
8453 841D	131.47 n/a	129 n/a
9402	130.37	129.14
931A	n/a	n/a
941C 9354	n/a 132.59	n/a 130.15
9353	132.46	129.74
941D	n/a	n/a
9302	132.23	129.63
9255 9401	n/a 129	n/a 125.77
9494	n/a	n/a
9201	142.87	141.09
9202 911A	142.77 n/a	138.84 n/a
9203	140.18	137.06
9251	143.56	140.97
9303 921D	n/a n/a	n/a n/a
9252	143.3	140.45
921E	n/a	n/a
921C 9352	n/a 134.23	n/a 130.8
9352	134.23 134.13	131.07
941E	n/a	n/a
8252 8253	139.1 139.47	135.99 136.42
8253 831C	139.47 n/a	130.42 n/a
831D	n/a	n/a
8353	132.86	130.92
8354 8303	132.97 132.83	130.63 130.17
8359	131.1	128.41
821C	n/a	n/a
8308 8201	131.01 141.55	127.85 138.51
8202	141.46	138.12
8251	141.52	138.58
8357 8355	130.9 131.71	128.85 129.2
8304	131.61	128.71
8306	130.81	128.43
8356 821B	131.84 n/a	129.64 n/a
8305	n/a 131.75	n/a 129.43
831F	n/a	n/a
821A	n/a	n/a
831G 831E	n/a n/a	n/a n/a
921B	n/a	n/a
931B	n/a	n/a
921A 7352	n/a 141.87	n/a 138.83
7352 7256	143.32	141.05
7203	143.74	141.59

Manhole Reference 7255	Manhole Cover Level	Manhole Invert Level
7233 731D	n/a	n/a
7302	139.77	135.92
7353 721A	139.44 n/a	136.27 n/a
731E	n/a	n/a
7351	138.79	136.47
7202 7251	139.73 139.63	137.4 137.68
731A	n/a	n/a
7252	139.35	137.13
7204 731B	139.13 n/a	136.84 n/a
7253	138.56	134.89
7205 8204	138.82 137.16	134.75 134.26
8205	136.03	133.63
8206	135.97	133.52
8352 8351	135.05 135.31	132.29 133.35
8301	135.17	133.19
8302 831B	134.78 n/a	131.88 n/a
8203	139.02	135.39
5102	151.57	150.22
5105 5104	151.6 152.56	149.6 149.43
5104	152.85	149.43
5103	n/a	n/a
6160 6159	152.49 151.47	149 148.84
6002	n/a	n/a
6163	151.35	149.15
6251 6158	148.54 150.3	146.14 148.66
6162	n/a	n/a
621B 621C	n/a n/a	n/a n/a
621D	n/a	n/a
6157	151.693	148.343
6161 6151	151.3 146.91	148.81 144.2
6101	146.93	143.84
621F	n/a	n/a
611C 6001	n/a 150.87	n/a 148.32
6003	n/a	n/a
6152 611B	146.54 n/a	143.91 n/a
611A	n/a	n/a
711A	n/a	n/a
7163 7159	n/a 143.64	n/a 139.95
7164	n/a	n/a
7103	150.29 n/a	148.44 n/a
7162 7102	150.51	148.82
7101	150.36	148.7
7201 7151	142.87 150.29	140.38 147.02
7155	144.11	141.78
7051	152.9	149.44
7152 7154	150.54 148.93	149.14 145.83
7104	144.76	143.22
7158 7105	144.45 144.64	142.34 141.88
7254	139.37	137.67
7206	139.36	137.99
7153 7157	149.7 143.39	148.21 141.41
7156	141.73	140.34
7106 8152	141.93 149.04	140.44 146.44
8102	149.07	146.47
8151	150.3	149.03
8101 8153	150.31 148.76	148.79 146.85
7001	152.9	149.7
621A	n/a	n/a
621E 6252	n/a 149.22	n/a 147.69
6253	148.58	146.8
521A 521B	n/a n/a	n/a n/a
531B	n/a	n/a
6351	146.62	144.4
6301 6302	145.36 144.08	143.22 142.23
6352	144.31	141.8
6304	144.01 143.08	140.46
6354 6353	143.98 143.8	140.74 140.24

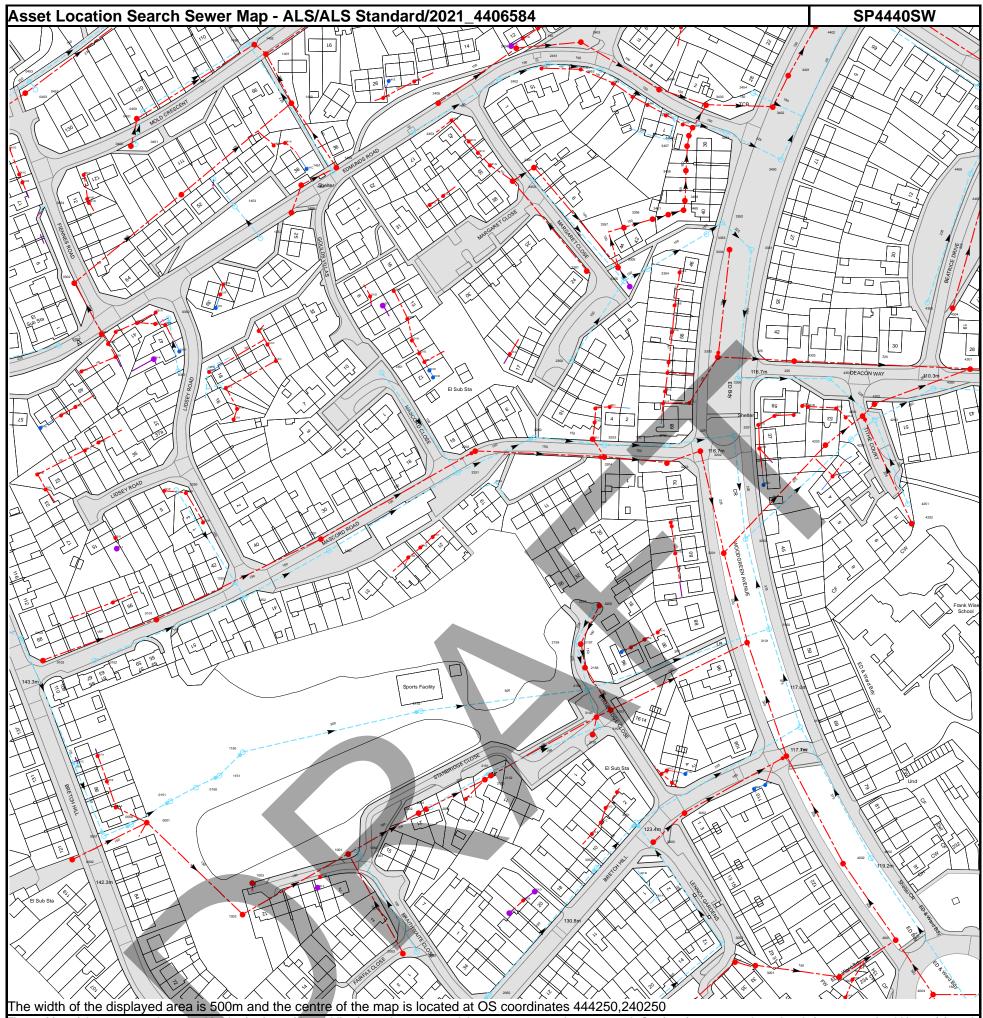
Manhole Reference	Manhole Cover Level	Manhole Invert Level
6303	143.87	140.1
631C	n/a	n/a
7301	141.99	138.18
6355	143.63	140.57
631D	n/a	n/a
6305	143.77	140.79
531C	n/a	n/a
631A	n/a	n/a
631B	n/a	n/a
6308	144.19	142.68
531D	n/a	n/a
6358	145.06	141.18
5301	146.09	142.69
6309	145	142.04
6357	143.75	141.12
6307	143.66	141.62
6356	143.036	140.956
6306	142.91	141.34
531A	n/a	n/a
5451	145.88	141.41
641C	n/a	n/a
5403	145.69	142.49
641E	n/a	n/a
541C	n/a	n/a
541D	n/a	n/a
541E	n/a	n/a
6451	142.36	140.12
541A	n/a	n/a
641A	n/a	n/a
641G	n/a	n/a
6401	142.03	139.81
541B	n/a	n/a
641B	n/a	n/a
641L	n/a	n/a
641F	n/a	n/a
5453	145.87	142.01
5402	145.52	143.06
5452	145.88	141.95
641D	n/a 141.23	n/a 138.52
6403	144.95	
5454 5401	144.94	143.25 143.75
641H	n/a	n/a
641N	n/a	n/a
641O	n/a	n/a
6411	n/a	n/a
V-11	TIVA	11/Q
The position of the enparatus chown on this plan	is given without obligation and warranty, and the acc	l

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Manhole Reference	Manhole Cover Level	Manhole Invert Level
451A	n/a	n/a
4550	131.59	127.57
4501 4650	131.52 130.09	127.38 128.13
4601	130.04	127.57
4652	n/a	n/a
4603 471D	129.96 n/a	127.88 n/a
471C	n/a	n/a
3952	130.93	n/a
3953 491A	n/a n/a	n/a n/a
4904	120.84	119.23
4950 3851	121.7 129.14	118.62 128.47
3850	127.52	125.72
3801	127.18	125.08
381C 381B	n/a n/a	n/a n/a
381A	n/a	n/a
481A	n/a	n/a
391E 391D	n/a n/a	n/a n/a
391H	n/a	n/a
291A	n/a 122.66	n/a
4903 4953	122.66 122.39	120.88 120.71
4952	122.4	120.47
3911	n/a	n/a
3951 491B	121.7 n/a	120.99 n/a
391A	n/a	n/a
391J	n/a	n/a
391B 3950	n/a 121.76	n/a 120.79
391G	n/a	n/a
491C	n/a	n/a
4902 4951	121.19 121.13	118.68 120.18
391F	n/a	n/a
391C	n/a	n/a n/a
171E 181F	n/a n/a	n/a n/a
181I	n/a	n/a
2850 181C	131.71 n/a	130.15 n/a
2801	131.54	128.67
181D	n/a	n/a
181E 181H	n/a n/a	n/a n/a
181G	n/a	n/a
181A	n/a	n/a
1950 2951	143.65 135.8	141.81 n/a
2950	135.26	133.06
1951	144.28	142.91
2953 2952	141,23 136.92	139.45 135.35
0850	143.93	142.5
081B	n/a	n/a
081A 091A	n/a n/a	n/a n/a
181B	n/a	n/a
071G 0750	n/a 131.25	n/a 130.31
071H	n/a	n/a
0705	135.1	133.64
071J 0754	n/a 131.47	n/a 130.37
071I	n/a	n/a
0702	131.52	130.58
0756 071B	n/a n/a	n/a n/a
071C	n/a	n/a
0757 0759	n/a	n/a
0758 0751	n/a 132.41	n/a 130.56
061A	n/a	n/a
0701 071A	132.6 n/a	129.25 n/a
0753	n/a 132.6	130.6
071F	n/a	n/a
071E 0650	n/a 133.03	n/a 130.71
	100.00	
		128.87
0602 071D	133.12 n/a	128.87 n/a
0602 071D 0752	133.12 n/a 133.1	n/a 132.28
0602 071D	133.12 n/a	n/a

Manhole Reference	Manhole Cover Level	Manhole Invert Level
171B	n/a	n/a
171C	n/a	n/a
171A	n/a	n/a
171J	n/a	n/a
171D	135.7	131.22
171 1750	n/a 136.64	n/a 135.02
171H	n/a	n/a
171G	n/a	n/a
1701	135.8	130.96
2653	136.39	134.84
2750 2650	135.47 132.79	133.93 130.85
3604	132.65	130.71
3603	131.99	130.4
3602	131.73	130.14
3652	131.82	130.33
3651 3502	131.27 131.96	129.7 130.76
3503	131.05	129.54
3551	131.97	130.21
351C	n/a	n/a
3650	130.55	129.05
361A 3601	n/a 130.35	n/a 128.85
3653	130.35	128.85
361B	n/a	n/a
371A	n/a	n/a
4552	130.66	128.8
4503	130.45	128.62
471B 4702	n/a 129.36	n/a 128.17
4702 471A	n/a	n/a
4750	129.4	128.46
4502	131.57	128.05
4551	131.53	128.26
4701 4602	129.67 130.32	128.09 128.1
4651	130.32	128.55
0601	132.48	128.51
0653	132.53	130.9
0651	133.35	132.11
0603	134	131.59
0604 0652	134.88 135.03	131.95 132.47
1501	136.86	132.1
1551	136.84	132.81
1502	138.03	136.07
151A	n/a	n/a
1601	138.32 n/a	132.33 n/a
151E 1650	138.31	132.92
1550	139.91	137.05
161A	n/a	n/a
1553	138.61	136.85
151D	n/a	n/a
151C 1651	n/a 137,75	n/a 133
1602	137.69	132.43
151B	n/a	n/a
1552	137.54	135.56
1652	n/a 138.61	n/a 136.36
1554 1653	138.61 137	136.36
251A	n/a	n/a
2554	134.42	132.96
2552	135.63	134
2551	135.33	n/a
2502 2550	n/a 135.71	n/a 133.53
2501	136.17	132.93
2555	134.54	133.09
2602	134.51	133.09
2601	134.16	132.46
2651 2603	134.28 134.71	132.61 133.12
2652	134.84	133.5
3550	132.54	129.97
351B	n/a	n/a
2553	133.48	132
3501 2503	132.17 133.59	130.41 132.26
2503 351A	133.59 n/a	132.26 n/a
4553	132.18	129.98
3552	132.25	130.36
The position of the apparatus share as this	s given without obligation and warranty, and the acc	Durany cannot be guaranteed. Coming allows and
	s given without obligation and warranty, and the acc liability of any kind whatsoever is accepted by Thames	



Manhole Reference Manhole Cover Level Manhole Invert Level	
A401 108.41 106.32 n/a n/a n/a 124.5 126.27 124.5 126.03 123.11 124.5 126.03 123.11 127.07 1403 129.43 127.51 141D n/a n/a n/a n/a 141E n/a n/a n/a 141E n/a n/a n/a 125.28 126.97 125.28 126.97 125.28 126.97 125.28 1241L n/a n/a n/a 127.62 n/a 124.1 127.62 n/a 126.43 124.8 124.1 127.59 126.15 126.35 n/a	
0452	
0453 126.03 123.11 127.07 1403 129.43 127.51 1410 1410 1416	
2405	
141D	
241A	
141E n/a n/a 2452 126.97 125.28 241L n/a 125.11 123.81 1401 127.62 n/a 2404 126.43 124.8 241K n/a 127.59 126.15 1450 127.59 126.35 126.35 241D n/a n/a n/a 241E n/a n/a n/a 241F n/a n/a n/a 3450 115.11 112.44 117.15 3457 118.91 117.15 117.15 341G n/a n/a n/a 3456 118.77 117.01 114.73 3451 116.28 117.01 114.73 3451 116.28 117.01 114.73 341D n/a n/a n/a 341D n/a n/a n/a 341F n/a n/a n/a 341F n/a n/a n/a 3402 114.77 113.12 n/a 341E n/a n/a n/a 3403 117.61 n/a n/a 341B n/a n/a n/a	
2452 241L	
125.11	
1401 127.62 n/a 2404 126.43 124.8 1450 127.59 126.15 1402 127.51 126.35 1402 127.51 126.35 1402 127.51 126.35 1402 112.41 1/a 241E n/a n/a 241F n/a n/a 3450 115.11 112.44 3457 118.91 117.15 341G n/a n/a 3456 118.77 117.01 3451 116.28 114.73 341H n/a n/a 3452 118.82 n/a 341D n/a n/a 3402 114.77 n/a 341E n/a n/a 3403 117.61 n/a 341B n/a 116.27 341B n/a 114.39 341B n/a 112.03 3453 122.99 121.6 241M n/a 124.1 n/a 2403 124.1 n/a 341A n/a 124.1 n/a	
2404 126.43 124.8 n/a 1450 1450 127.59 126.15 1402 127.51 126.35 n/a n	
1450 127.59 126.15 1402 127.51 126.35 241D n/a n/a 241E n/a n/a 241F n/a n/a 3450 115.11 112.44 3457 118.91 117.15 341G n/a n/a 3456 118.77 117.01 3451 116.28 117.01 3452 118.82 n/a 341D n/a n/a 3452 119.49 117.32 341F n/a n/a 3402 114.77 n/a 341E n/a n/a 3403 117.61 n/a 341C n/a 115.3 341C n/a 115.3 341B n/a 117.55 341B n/a 114.39 3453 122.99 121.6 241M n/a 124.1 2403 124.1 121.77 341A n/a 116.a	
1402 127.51 126.35 241D n/a n/a 241E n/a n/a 241F n/a n/a 3450 115.11 112.44 3457 118.91 117.15 341G n/a n/a 3456 118.77 117.01 3451 116.28 114.73 341H n/a n/a 3455 118.82 n/a 341D n/a n/a 3452 119.49 117.32 341F n/a n/a 3402 114.77 113.12 341E n/a n/a 3403 117.61 116.27 341C n/a 115.3 341C n/a 115.3 341B n/a 113.4 3401 114.39 117.55 341B n/a 114.39 3453 122.99 121.6 241M n/a 124.1 121.77 341A n/a 124.1 121.77 341A n/a 124.1 121.77	
241E n/a	
241F n/a n/a 3450 115.11 112.44 3457 118.91 117.15 341G n/a n/a 3456 118.77 117.01 3451 116.28 114.73 341H n/a n/a 3455 118.82 n/a 341D n/a n/a 3452 119.49 117.32 341F n/a n/a 3402 114.77 113.12 341E n/a 117.61 3403 117.61 116.27 341C n/a 115.3 341C n/a 113.4 3454 115.3 113.4 3404 119.96 117.55 341B n/a 114.39 112.03 3453 122.99 121.6 241M n/a 124.1 n/a 341A n/a 124.1 n/a	
3450 115.11 112.44 3457 118.91 117.15 341G n/a n/a 3456 118.77 117.01 3451 116.28 114.73 341H n/a n/a 3455 118.82 n/a 341D n/a n/a 3452 119.49 117.32 341F n/a n/a 3402 114.77 113.12 341E n/a 115.3 341C n/a 116.27 341C n/a 115.3 341A 119.96 117.55 341B n/a 114.39 3453 122.99 121.6 241M n/a 124.1 341A n/a 124.1	
3457 118.91 117.15 3416 n/a 118.77 3451 116.28 114.73 341H n/a n/a 3455 118.82 n/a 341D n/a n/a 3452 119.49 117.32 341F n/a n/a 3402 114.77 113.12 341E n/a 117.61 3403 117.61 116.27 341C n/a 115.3 341C n/a 115.3 341B n/a 117.55 341B n/a 114.39 3453 122.99 121.6 241M n/a 124.1 n/a 2403 124.1 n/a 121.77 341A n/a 124.1 n/a	
341G n/a n/a 3456 118.77 117.01 3451 116.28 114.73 341H n/a n/a 3455 118.82 n/a 341D n/a n/a 3452 119.49 117.32 341F n/a n/a 3402 114.77 113.12 341E n/a 116.27 341C n/a 116.27 341C n/a 115.3 341C n/a 113.4 3454 115.3 113.4 3404 119.96 117.55 341B n/a n/a 3401 114.39 112.03 3453 122.99 121.6 241M n/a n/a 2403 124.1 n/a 341A n/a 124.1 n/a	
3451 116.28 114.73 341H n/a n/a 3455 118.82 n/a 341D n/a n/a 3452 119.49 117.32 341F n/a n/a 3402 114.77 113.12 341E n/a n/a 3403 117.61 116.27 341C n/a 115.3 116.27 3454 115.3 113.4 3404 119.96 117.55 341B n/a 114.39 112.03 3453 122.99 121.6 241M n/a 124.1 n/a 2403 124.1 n/a 121.77 341A n/a n/a 121.77	
341H n/a n/a 3455 118.82 n/a 341D n/a n/a 3452 119.49 117.32 341F n/a n/a 3402 114.77 113.12 341E n/a 117.61 3403 117.61 116.27 341C n/a 115.3 3454 115.3 113.4 3404 119.96 117.55 341B n/a 114.39 3453 122.99 121.6 241M n/a 124.1 10 121.77 n/a 341A n/a 121.77	
3455 118.82 n/a 341D n/a n/a 3452 119.49 117.32 341F n/a n/a 3402 114.77 113.12 341E n/a 117.61 3403 117.61 116.27 341C n/a 115.3 341C 115.3 113.4 3454 119.96 117.55 341B n/a 114.39 3401 114.39 112.03 3453 122.99 121.6 241M n/a 124.1 341A n/a 124.1 n/a 121.77 n/a 121.77 n/a 124.1 n/a 121.77 n/a 121.77 n/a 121.77	
341D n/a n/a 117.32 341F n/a 117.32 n/a 3402 114.77 113.12 n/a 341E n/a 117.61 n/a 116.27 341C n/a 115.3 113.4 3454 115.3 113.4 117.55 341B n/a 114.39 112.03 3453 122.99 121.6 241M n/a 124.1 121.77 341A n/a 124.1 121.77 n/a 124.1 n/a 121.77	
341F n/a n/a 3402 114.77 113.12 341E n/a 117.61 3403 117.61 116.27 341C n/a 115.3 3454 115.3 113.4 3404 119.96 117.55 341B n/a 114.39 3401 114.39 112.03 3453 122.99 121.6 241M n/a 124.1 2403 124.1 121.77 341A n/a 124.1 n/a 121.77 n/a 121.77 n/a 124.1 121.77 n/a 124.1 121.77 n/a 124.1 121.77 124.1 124.1 124.1 125.2 125.2 125.2 126.2 126.2 127.2 127.7 127.2 127.2 128.2 128.2 128.2 129.2 129.2 129.2 120.2 129.2 129.2 120.2 129.2<	
3402 114.77 113.12 341E n/a 117.61 3403 117.61 116.27 341C n/a 115.3 113.4 3454 119.96 117.55 341B n/a 114.39 112.03 3453 122.99 121.6 241M n/a 124.1 121.77 341A n/a 124.1 121.77 n/a 124.1 n/a 121.77 n/a 124.1 n/a 121.77 n/a 124.1 n/a 121.77 n/a 124.1 121.77 n/a	
341E n/a 117.61 116.27 341C n/a 115.3 113.4 3454 115.3 113.4 117.55 341B n/a 114.39 112.03 3401 114.39 112.03 121.6 3453 122.99 121.6 n/a 241M n/a 124.1 121.77 341A n/a 124.1 n/a	
341C n/a n/a 3454 115.3 113.4 3404 119.96 117.55 341B n/a n/a 3401 114.39 112.03 3453 122.99 121.6 241M n/a 124.1 2403 124.1 121.77 341A n/a n/a	
3454 115.3 113.4 3404 119.96 117.55 341B n/a 114.39 3401 114.39 112.03 3453 122.99 121.6 241M n/a 124.1 2403 124.1 121.77 341A n/a n/a	
3404 119.96 341B n/a 3401 114.39 3453 122.99 241M n/a 2403 124.1 341A n/a	
3401 114.39 3453 122.99 241M n/a 2403 124.1 341A n/a 112.03 121.6 n/a 121.77 n/a 121.77 n/a	
3453 241M 2403 341A 122.99 n/a 124.1 n/a 121.77 n/a	
241M 2403 341A n/a 121.77 n/a	
2403 341A 121.77 n/a n/a	
1 4402	
131C n/a n/a	
131D 1/a n/a n/a	
131B n/a	
241I n/a n/a n/a	
241J 2402 n/a 127.78 125.74	
2450 127.37 125.12	
1451 129.69	
141C n/a 1404 131.55 129.51	
2401 127.29 125.41	
241G n/a	
241C	
2453 129.79 178.11	
241B n/a n/a	
031K 031C n/a n/a n/a	
031C	
031E n/a n/a	
031F n/a n/a	
031B n/a n/a 130.55	
031A n/a n/a	
131E n/a n/a	
131F	
0302 130.01 178 178 178 178 178 178 178 178 178 17	
1352 131.88 129.52	
1406 131.55 130.37	
0454 127.45 126.27 1453 130.33 128.95	
041D n/a n/a	
041E n/a n/a	
041A n/a n/a 1405 131.62 130.19	
1405 131.62 130.19 1452 n/a n/a	
041F n/a n/a	
0402 127.92 127.03	
141B n/a 0451 128.06	
141A n/a n/a	
0401 128.06 126.87	
0450 127.92 126.62 3202 116.92 114.94	
321D 116.92 114.94 1/a 1/a	

Mania da Batanana	Maril ala Ossar I sant	Mantala Incomitant
Manhole Reference 4203	Manhole Cover Level	Manhole Invert Level
4252	111.42	108.27
3251 321E	116.46 n/a	113.46 n/a
4250	110.92	107.84
421A 4201	n/a 111.17	n/a 107.86
321F	n/a	n/a
421B	n/a	n/a
431C 431B	n/a n/a	n/a n/a
431A	n/a	n/a
4302 4350	111.1 109.9	107.46 106.92
3350	117.09	114.32
4351 4303	113.48 115.17	110.81 111.69
3303	118.74	115.23
4304	109.28	107.66
4352 3351	109.94 117.14	107.78 115.43
3304	118.03	116.31
3353 3352	118.6 117.58	117.29 115.99
4450	108.63	106.54
3203 3204	119.47 122.78	117.61 120.7
3252	119.31	117.49
3253	122.96	120.85
2201 221B	123.6 n/a	121.46 n/a
331H	n/a	n/a
331I 331F	n/a n/a	n/a n/a
331J	n/a	n/a
331E 331G	n/a n/a	n/a n/a
331D	n/a	n/a
331C	n/a	n/a
331B 3358	n/a n/a	n/a n/a
3355	124.23	122.09
331A 2301	n/a 125.49	n/a 123.64
3354	122.51	120.74
3305 3357	n/a 122.81	n/a 121.08
3356	121.48	118.78
3461 3460	120.4 119.07	118.12 117.7
3459	119.07	117.7
3458	118.88	117.29
221E 1201	n/a 135.53	n/a 134.11
221F	n/a	n/a
221A 2251	n/a 131.03	n/a 129.18
2202	130.13	127.87
2250 231D	127.19 n/a	124.18 n/a
1351	133.83	131.65
231E 231F	n/a n/a	n/a n/a
2350	126.73	124.48
231C	n/a	n/a
231G 231H	n/a n/a	n/a n/a
231B	n/a	n/a
041G 041C	n/a n/a	n/a n/a
0153	143.24	141.68
041B 021J	n/a n/a	n/a n/a
0211	n/a	n/a
021H 021G	n/a n/a	n/a n/a
021F	n/a	n/a
0250	138.59	136.82
021L 021K	n/a n/a	n/a n/a
021C	n/a	n/a
021D 021A	n/a n/a	n/a n/a
121A	n/a	n/a
021B 131K	n/a n/a	n/a n/a
031G	n/a	n/a
131J	n/a	n/a
131N 031J	n/a n/a	n/a n/a
0351	135.69	n/a
131M 131I	n/a n/a	n/a n/a
	1114	1174

Manhala Deference	Manhala Cayer Laval	Manhala Invest Lavel
Manhole Reference	Manhole Cover Level	Manhole Invert Level
031D	n/a	n/a
0352	133.14	131.58
131H 031H	n/a n/a	n/a n/a
0301	132.5	131.31
311C 3102	n/a 118.52	n/a 115.58
2101	120.18	117.64
4150	117.62	115.42
3151 3103	119.91 119.74	117.67
3153	119.74	117.45 116.97
2150	119.43	117.3
2158 311I	119.41 n/a	117.95 n/a
311F	n/a	n/a
2157	119.87	118.36
3101 2159	116.34 119.98	114.03 118.32
311G	n/a	n/a
311H	n/a	n/a
3150 2254	116.27 120.03	114.41 118.53
3255	119.95	118.71
321G	n/a	n/a
3201 3250	116.17 115.44	113.66 113.22
4202	112.59	110.52
321H	n/a	n/a
4251 321C	112.29 n/a	109.55 n/a
421C	n/a	n/a
0050	141.14	136.76
0001	140.29	136.53
2002 2001	129.79 129.01	127.86 n/a
001A	n/a	n/a
2051 0151	128.92 138.65	125.48 133.86
211A	n/a	n/a
0150	137.76	n/a
011B 2103	n/a 124.95	n/a 123.51
2102	124.54	122.05
1151	136.77	129.2
2151 011D	124.19 n/a	121.27 n/a
011C	n/a	n/a
011A	n/a	n/a
1150 2152	135.75 127.63	127.21 121.13
0102	142.97	140.34
0152	141.05	138.5
0101 021E	138.95 n/a	136.94 n/a
1252	136.66	134.95
1253	136.69	135.22
221D 1251	n/a 134.6	n/a 132.92
201A	n/a	n/a
3052 301C	127.88 n/a	125.69
301C 301D	n/a n/a	n/a n/a
3051	124.29	121.24
301B 3003	n/a 125.3	n/a 123.77
301A	n/a	n/a
311D	n/a	n/a
311E 3002	n/a 122.02	n/a 120.69
311A	n/a	n/a
3053	125.82	124.02
3057 311J	n/a n/a	n/a n/a
3001	124.55	122.76
311K	n/a	n/a
4003 4002	n/a 119.77	n/a 116.71
401A	n/a	n/a
401B	n/a 110.74	n/a 117.16
4050 4001	119.74 120.18	117.16 117.39
4004	121.44	118.05
2050	136.57	133.98
2003 2052	139.16 137.06	137.11 135.5
1002	137.46	134.17
201C 201B	n/a n/a	n/a
201B 201D	n/a n/a	n/a n/a
101A	n/a	n/a
1003	n/a	n/a

Manhole Reference	Manhole Cover Level	Manhole Invert Level
101B	n/a	n/a
1051	133.9	131.02
1001	134.28	131.64
1050	133.86	130.78
0002	142.87	141.39
0051	142.32	139.08
441A	n/a	n/a





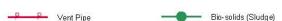
Public Sewer Types (Operated & Maintained by Thames Water)

Foul: A sewer designed to convey waste water from domestic and industrial sources to a treatment works.

-- O-- Surface Water: A sewer designed to convey surface water (e.g. rain water from roofs, yards and car parks) to rivers or watercourses.

Combined: A sewer designed to convey both waste water and surface water from domestic and industrial sources to a treatment works.







Foul Rising Main

Proposed Thames Water





Sewer Fittings

A feature in a sewer that does not affect the flow in the pipe. Example: a vent is a fitting as the function of a vent is to release excess gas.

Air Valve Dam Chase Fitting

Meter

O Vent Column

Operational Controls

A feature in a sewer that changes or diverts the flow in the sewer. Example, A hydrobrake limits the flow passing downstream.

Control Valve

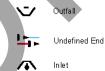
Drop Pipe

Ancillar

Weir

End Items

End symbols appear at the start or end of a sewer pipe. Examples: an Undefined End at the start of a sewer indicates that Thames Water has no knowledge of the position of the sewer upstream of that symbol, Outfall on a surface water sewer indicates that the pipe discharges into a stream or river.



Other Symbols

Symbols used on maps which do not fall under other general categories

Public/Private Pumping Station

Change of characteristic indicator (C.O.C.I.)

Invert Level

Summit

Areas

Lines denoting areas of underground surveys, etc.

Agreement

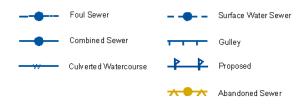
Operational Site

Chamber

Tunnel

Conduit Bridge

Other Sewer Types (Not Operated or Maintained by Thames Water)

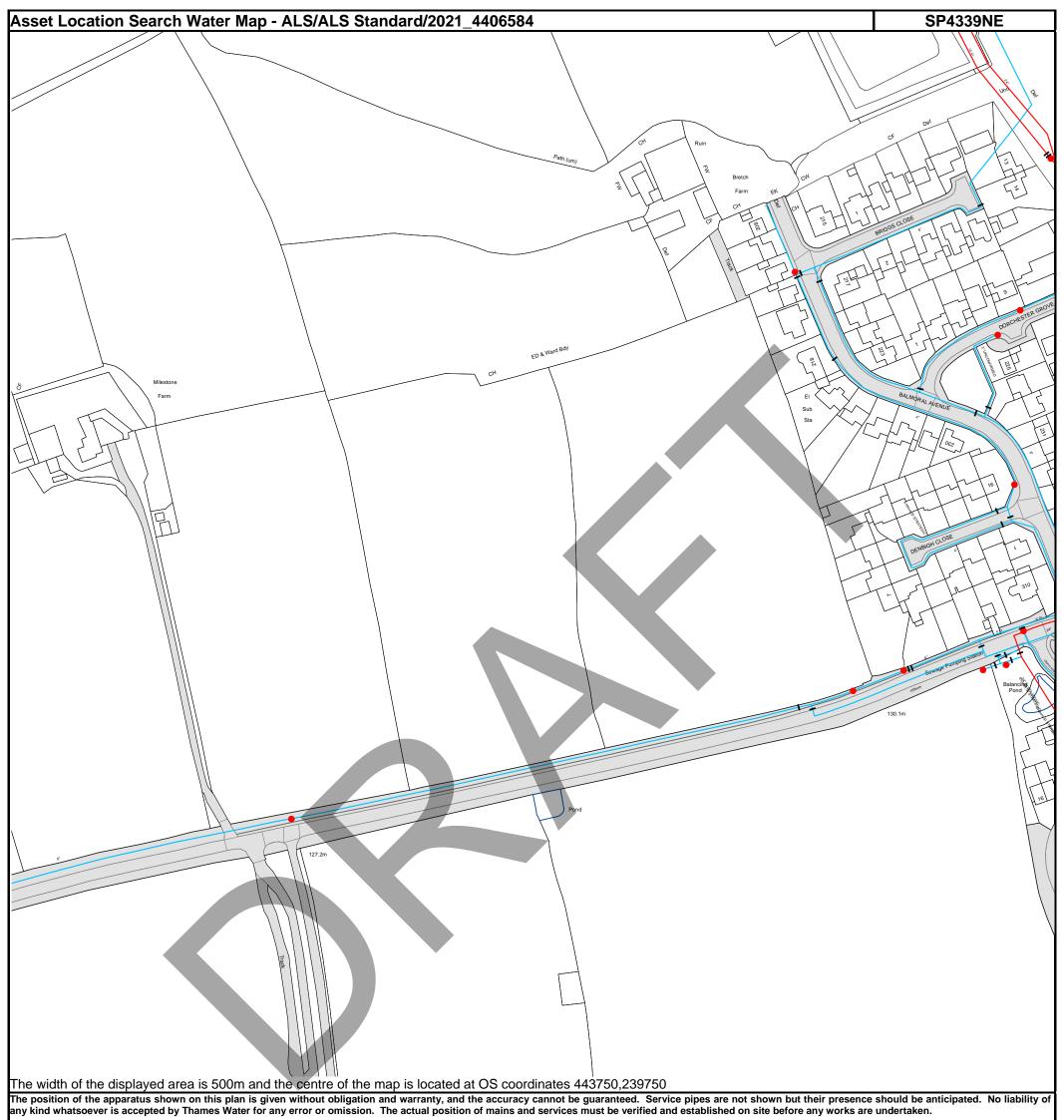


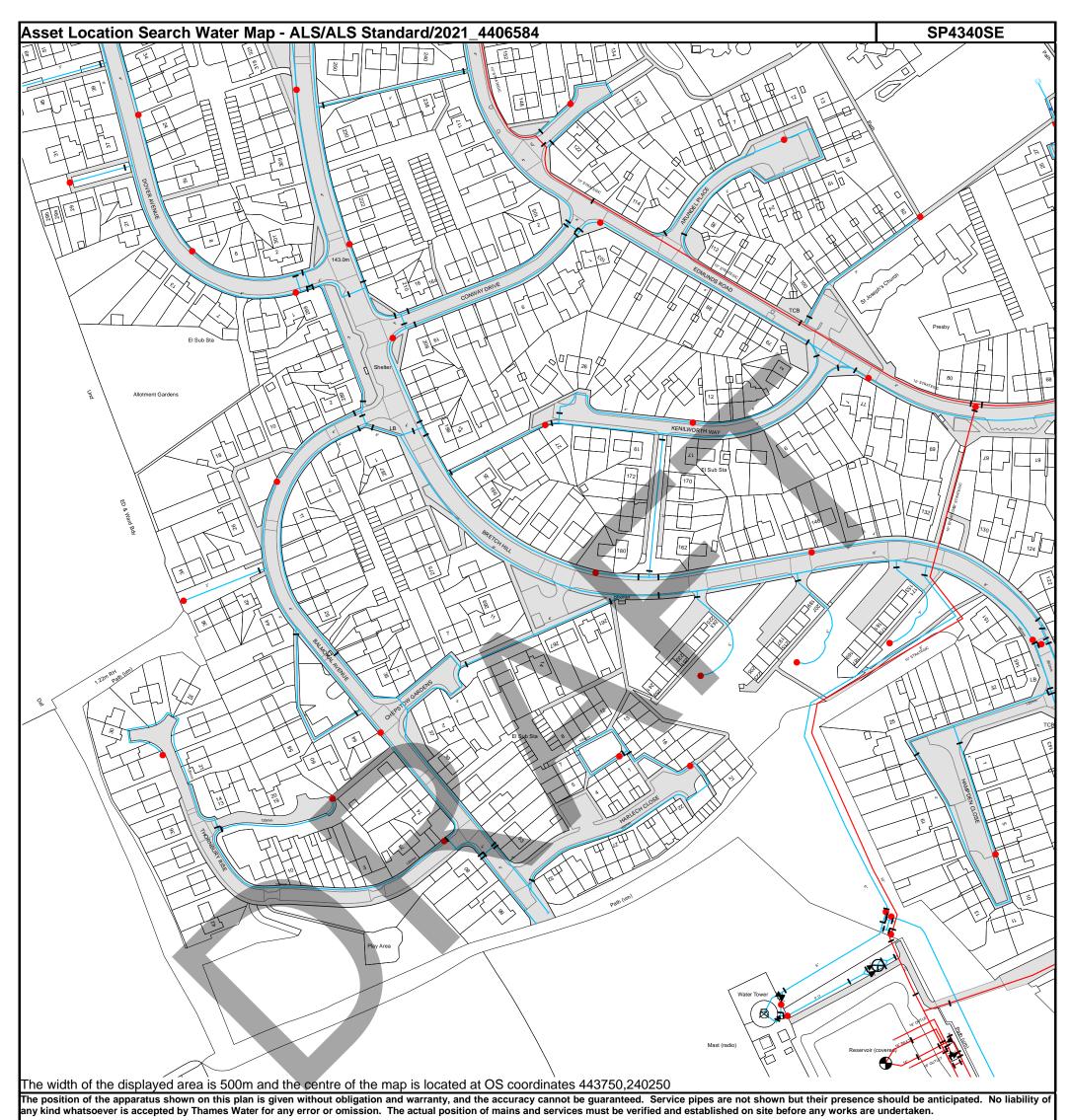
Notes:

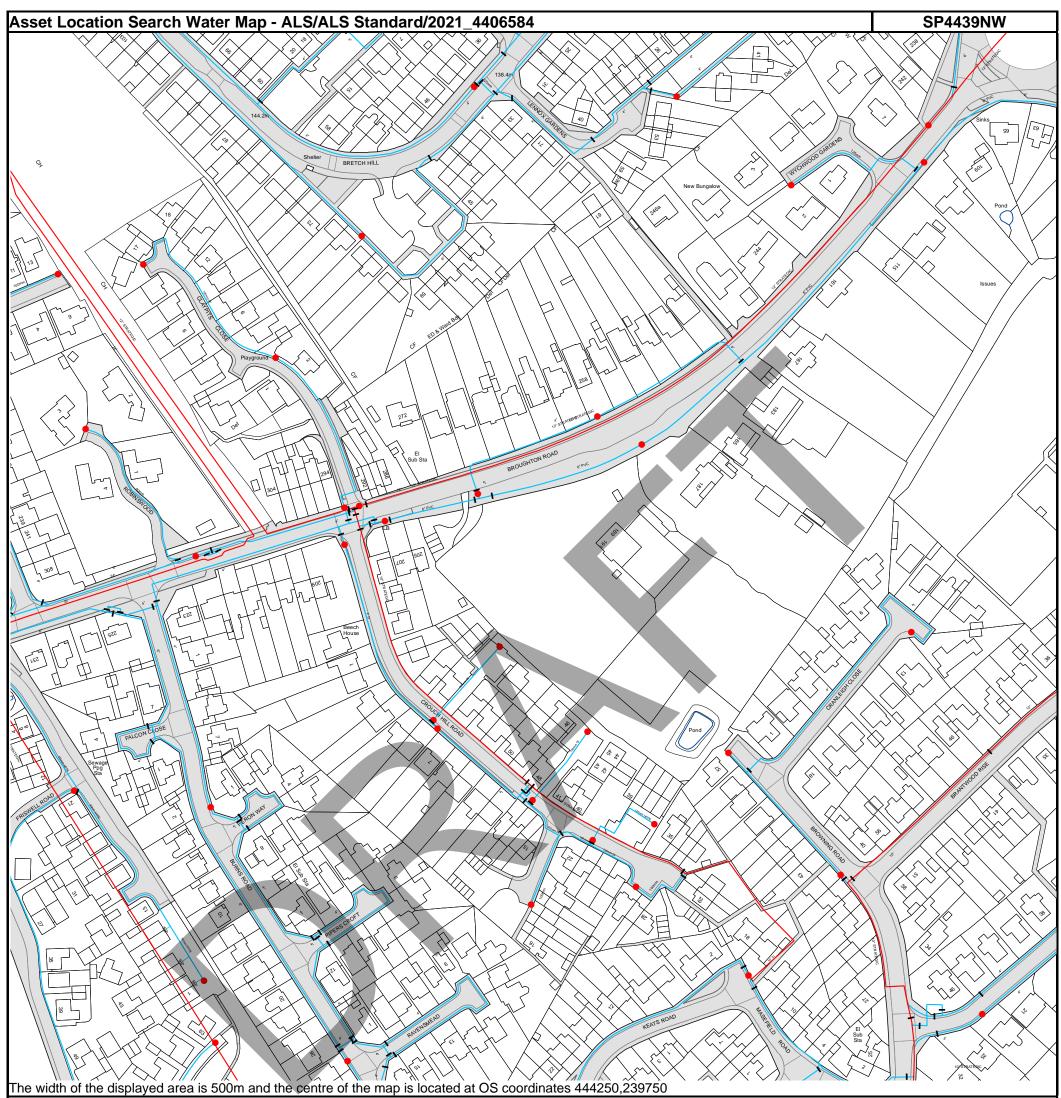
Gallery Gallery

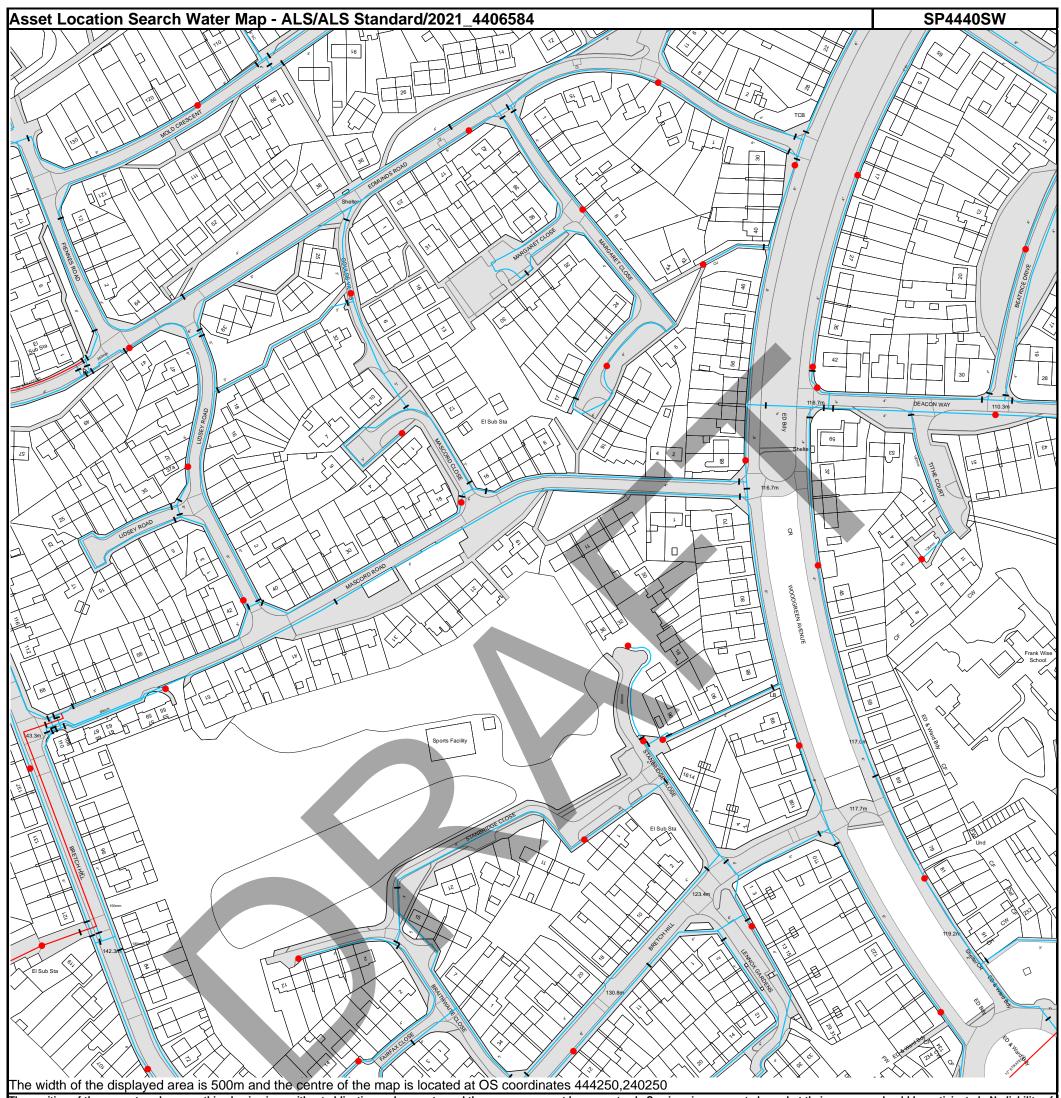
- 1) All levels associated with the plans are to Ordnance Datum Newlyn.
- 2) All measurements on the plans are metric.
- 3) Arrows (on gravity fed sewers) or flecks (on rising mains) indicate direction of
- 4) Most private pipes are not shown on our plans, as in the past, this information has not been recorded.
- 5) 'na' or '0' on a manhole level indicates that data is unavailable.

6) The text appearing alongside a sewer line indicates the internal diameter of the pipe in milimetres. Text next to a manhole indicates the manhole reference number and should not be taken as a measurement. If you are unsure about any text or symbology present on the plan, please contact a member of Property Searches on 0800 009 4540.











3" FIRE

3" METERED

Water Pipes (Operated & Maintained by Thames Water)

Distribution Main: The most common pipe shown on water maps. With few exceptions, domestic connections are only made to distribution mains.

Trunk Main: A main carrying water from a source of supply to a treatment plant or reservoir, or from one treatment plant or reservoir to another. Also a main transferring water in bulk to smaller water mains used for supplying individual customers.

Supply Main: A supply main indicates that the water main is used as a supply for a single property or group of properties.

Fire Main: Where a pipe is used as a fire supply, the word FIRE will be displayed along the pipe.

Metered Pipe: A metered main indicates that the pipe in question supplies water for a single property or group of properties and that quantity of water passing through the pipe is metered even though there may be no meter symbol shown.

Transmission Tunnel: A very large diameter water pipe. Most tunnels are buried very deep underground. These pipes are not expected to affect the structural integrity of buildings shown on the map provided.

Proposed Main: A main that is still in the planning stages or in the process of being laid. More details of the proposed main and its reference number are generally included near the main.

Valves

General PurposeValve

→ Air Valve

➤ Pressure ControlValve

Customer Valve

Hydrants

Single Hydrant

Meters

Meter

End Items

Symbol indicating what happens at the end of ^L a water main.

Blank Flange

Capped End

Emptying Pit

© Undefined End

Manifold

Customer Supply

Fire Supply

Operational Sites

Booster Station

Other

Other (Proposed)

■ Pumping Station

Service Reservoir

Shaft Inspection

Treatment Works

Unknown

Other Symbols

_____ Data Logger

Other Water Pipes (Not Operated or Maintained by Thames Water)

 Other Water Company Main: Occasionally other water company water pipes may overlap the border of our clean water coverage area. These mains are denoted in purple and in most cases have the owner of the pipe displayed along them.

Private Main: Indiates that the water main in question is not owned by Thames Water. These mains normally have text associated with them indicating the diameter and owner of the pipe.

PIPE DIAMETER DEPTH BELOW GROUND

Up to 300mm (12")	900mm (3')	
300mm - 600mm (12" - 24")	1100mm (3' 8")	
600mm and bigger (24" plus)	1200mm (4')	

Terms and Conditions

All sales are made in accordance with Thames Water Utilities Limited (TWUL) standard terms and conditions unless previously agreed in writing.

- 1. All goods remain in the property of Thames Water Utilities Ltd until full payment is received.
- 2. Provision of service will be in accordance with all legal requirements and published TWUL policies.
- 3. All invoices are strictly due for payment 14 days from due date of the invoice. Any other terms must be accepted/agreed in writing prior to provision of goods or service, or will be held to be invalid.
- 4. Thames Water does not accept post-dated cheques-any cheques received will be processed for payment on date of receipt.
- 5. In case of dispute TWUL's terms and conditions shall apply.
- 6. Penalty interest may be invoked by TWUL in the event of unjustifiable payment delay. Interest charges will be in line with UK Statute Law 'The Late Payment of Commercial Debts (Interest) Act 1998'.
- 7. Interest will be charged in line with current Court Interest Charges, if legal action is taken.
- 8. A charge may be made at the discretion of the company for increased administration costs.

A copy of Thames Water's standard terms and conditions are available from the Commercial Billing Team (cashoperations@thameswater.co.uk).

We publish several Codes of Practice including a guaranteed standards scheme. You can obtain copies of these leaflets by calling us on 0800 316 9800

If you are unhappy with our service you can speak to your original goods or customer service provider. If you are not satisfied with the response, your complaint will be reviewed by the Customer Services Director. You can write to her at: Thames Water Utilities Ltd. PO Box 492, Swindon, SN38 8TU.

If the Goods or Services covered by this invoice falls under the regulation of the 1991 Water Industry Act, and you remain dissatisfied you can refer your complaint to Consumer Council for Water on 0121 345 1000 or write to them at Consumer Council for Water, 1st Floor, Victoria Square House, Victoria Square, Birmingham, B2 4AJ.

Ways to pay your bill

- III - I	T 7 4 2 2 7	I = 4 ·	
Credit Card	BACS Payment	Telephone Banking	Cheque
			-
Call 0800 009 4540	Account number	By calling your bank and	Made payable to 'Thames
quoting your invoice	90478703	quoting:	Water Utilities Ltd'
number starting CBA or	Sort code 60-00-01	Account number	Write your Thames Water
ADS / OSS	A remittance advice must	90478703	account number on the
	be sent to:	Sort code 60-00-01	back.
	Thames Water Utilities	and your invoice number	Send to:
	Ltd., PO Box 3189,		Thames Water Utilities
	Slough SL1 4WW.		Ltd., PO Box 3189,
	or email		Slough SL1 4WW
	ps.billing@thameswater.		or by DX to 151280
	<u>co.uk</u>		Slough 13

Thames Water Utilities Ltd Registered in England & Wales No. 2366661 Registered Office Clearwater Court, Vastern Rd, Reading, Berks, RG1 8DB.



Mr Ryan Davies
BWB Consulting
Ryan.Davies@bwbconsulting.com



09 September 2021

Pre-planning enquiry: Capacity concerns

Site address: Land north of Balmoral Avenue, Phase 2, Banbury, OX16 0BG

Dear Mr Davies,

Thank you for providing information on your development for the proposed 48no. residential units on previously Greenfield land. We have based our assessment on the information you provided to us and have copied below for clarity:-

Proposed foul water flows to discharge via gravity into foul water manhole ref. 8801.

We've assessed your **foul water** proposals and concluded from our initial review, that our sewerage network does have sufficient capacity for **30no. residential units**, however, we're unable to meet the needs of the remaining units at this time.

As you've not supplied us with details of your **surface water** proposals, we've assumed you **don't** intend to discharge your surface water to the public sewerage system.

In order to ensure we make the appropriate upgrades – or 'off-site reinforcement' – to serve the remainder of your development, we'll need to carry out modelling work, design a solution and build the necessary improvements. Typical timescales for a development of your size are:

Modelling: 8 months
Design: 6 months
Construction: 6 months

Total: 20 months

If the time you're likely to take from planning and construction through to first occupancy is longer than this, we'll be able to carry out the necessary upgrades in time for your development. If it's shorter, please contact me on the number below to discuss the timing of our activities.

What do you need to tell us before we start modelling?

We're responsible for funding any modelling and reinforcement work. We need, though, to spend our customers' money wisely, so we'll only carry out modelling once we're confident that your development will proceed.

In order to have this confidence, we'll need to know that you **own the land and have either outline or full planning permission**. Please email this information to us as soon as you have it.

If the modelling shows we need to carry out reinforcement work, then before we start construction, we'll need you to supply us with notification that you've confirmed your 'nominated competent person' (NCP) submission to the Health and Safety Executive.

Please note that you must keep us informed of any changes to your design – for example, an increase in the number or density of homes. Such changes could mean there is no longer sufficient supply capacity.

If you have any further questions, please do not hesitate to contact me.

Yours sincerely

Rahim Khan
Thames Water – Adoptions Engineer
rahim.khan@thameswater.co.uk





Appendix 4: Greenfield Runoff Rate Calculations



BWB Consulting Ltd		Page 1
5th Floor, Waterfront House		
35 Station Street		
Nottingham, NG2 3DQ		Micro
Date 06/10/2021 21:40	Designed by Ryan.Davies	Drainage
File FEH Storage.SRCX	Checked by	Dialilade
Innovyze	Source Control 2020.1	

ICP SUDS Mean Annual Flood

Input

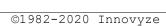
Return Period (years) 100 Soil 0.400
Area (ha) 1.400 Urban 0.000
SAAR (mm) 700 Region Number Region 6

Results 1/s

QBAR Rural 4.8 QBAR Urban 4.8

Q100 years 15.2

Q1 year 4.0 Q30 years 10.8 Q100 years 15.2









BWB Consulting Ltd		Page 1
5th Floor, Waterfront House		
35 Station Street		
Nottingham, NG2 3DQ		Mirro
Date 06/10/2021 21:53	Designed by Ryan.Davies	Drainage
File	Checked by	Dialilade
Innovyze	Source Control 2020.1	

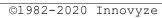
Greenfield Runoff Volume

FEH Data

Return Period (years)				100
Storm Duration (mins)				360
FEH Rainfall Version				1999
Site Location	GB 44335	0 239550	SP 43350	39550
C (1km)				-0.024
D1 (1km)				0.315
D2 (1km)			^	0.328
D3 (1km)				0.236
E (1km)				0.300
F (1km)				2.484
Areal Reduction Factor	4			1.00
Area (ha)				1.420
SAAR (mm)				646
CWI				95.280
SPR Host				40.720
URBEXT (1990)				0.0617

Results

Percentage Runoff (%) 39.23 Greenfield Runoff Volume (m³) 383.027







Appendix 6: Microdrainage Storage Calculations



BWB Consulting Ltd		Page 1
5th Floor, Waterfront House	BMW3250	
35 Station Street	Bretch Hill, Banbury Phase 2	
Nottingham, NG2 3DQ	FEH Storage	Micro
Date 06/10/2021 21:42	Designation of last Designation	Drainage
File FEH Storage.SRCX	Checked by	Dialilade
Innovyze	Source Control 2020.1	

Summary of Results for 100 year Return Period (+40%)

Storm Event		Max Level (m)	Max Depth (m)	Max Control (1/s)	Max Volume (m³)	Status	
15	min	Summer	99.352	0.352	4.8	370.8	ОК
30	min	Summer	99.400	0.400	4.8	424.9	O K
60	min	Summer	99.452	0.452	4.8	484.1	O K
120	min	Summer	99.506	0.506	4.8	546.3	0 K
180	min	Summer	99.536	0.536	4.8	582.0	O K
240	min	Summer	99.556	0.556	4.8	606.1	O K
360	min	Summer	99.581	0.581	4.8	636.0	O K
480	min	Summer	99.595	0.595	4.8	652.7	O K
600	min	Summer	99.603	0.603	4.8	661.5	O K
720	min	Summer	99.606	0.606	4.8	665.2	O K
960	min	Summer	99.606	0.606	4.8	665.9	O K
1440	min	Summer	99.588	0.588	4.8	643.8	O K
2160	min	Summer	99.557	0.557	4.8	607.1	O K
2880	min	Summer	99.528	0.528	4.8	573.1	O K
4320	min	Summer	99.447	0.447	4.8	478.2	O K
5760	min	Summer	99.378	0.378	4.8	400.3	ОК
7200	min	Summer	99.319	0.319	4.8	334.3	O K
8640	min	Summer	99.268	0.268	4.8	278.1	O K
10080	min	Summer	99.224	0.224	4.8	231.4	O K
15	min	Winter	99.392	0.392	4.8	415.9	O K
30	min	Winter	99.446	0.446	4.8	477.0	O K

Storm		Rain	Flooded	Discharge	Time-Peak	
Event		(mm/hr)	Volume	Volume	(mins)	
				(m³)	(m³)	
15	min	Summer	200.975	0.0	365.2	27
30	min	Summer	115.798	0.0	393.1	41
60	min	Summer	66.721	0.0	499.9	70
120	min	Summer	38.443	0.0	576.2	130
180	min	Summer	27.846	0.0	626.0	190
240	min	Summer	22.150	0.0	664.0	248
360	min	Summer	16.044	0.0	721.1	368
480	min	Summer	12.763	0.0	746.3	486
600	min	Summer	10.687	0.0	757.0	606
720	min	Summer	9.244	0.0	757.3	724
960	min	Summer	7.381	0.0	746.9	962
1440	min	Summer	5.375	0.0	721.2	1316
2160	min	Summer	3.914	0.0	1056.1	1632
2880	min	Summer	3.125	0.0	1124.4	2016
4320	min	Summer	2.192	0.0	1183.0	2776
5760	min	Summer	1.705	0.0	1226.6	3576
7200	min	Summer	1.402	0.0	1261.7	4328
8640	min	Summer	1.196	0.0	1290.9	5024
10080	min	Summer	1.045	0.0	1316.1	5752
15	min	Winter	200.975	0.0	389.1	26
30	min	Winter	115.798	0.0	403.1	41

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BWB Consulting Ltd		Page 2
5th Floor, Waterfront House	BMW3250	
35 Station Street	Bretch Hill, Banbury Phase 2	
Nottingham, NG2 3DQ	FEH Storage	Micro
Date 06/10/2021 21:42	Designed by Ryan.Davies	Drainage
File FEH Storage.SRCX	Checked by	Dialilade
Innovyze	Source Control 2020.1	

Summary of Results for 100 year Return Period (+40%)

	Stor Even		Max Level (m)	Max Depth (m)	Max Control (1/s)	Max Volume (m³)	Status
60	min	Winter	99.504	0.504	4.8	544.3	ОК
120	min	Winter	99.565	0.565	4.8	616.0	O K
180	min	Winter	99.600	0.600	4.8	658.4	ОК
240	min	Winter	99.624	0.624	4.8	687.7	O K
360	min	Winter	99.655	0.655	4.8	725.2	ОК
480	min	Winter	99.673	0.673	4.8	747.1	O K
600	min	Winter	99.683	0.683	4.8	760.1	O K
720	min	Winter	99.689	0.689	4.8	767.3	O K
960	min	Winter	99.695	0.695	4.8	774.2	ОК
1440	min	Winter	99.685	0.685	4.8	762.5	O K
2160	min	Winter	99.649	0.649	4.8	717.9	O K
2880	min	Winter	99.611	0.611	4.8	671.4	O K
4320	min	Winter	99.492	0.492	4.8	530.9	O K
5760	min	Winter	99.391	0.391	4.8	415.0	O K
7200	min	Winter	99.305	0.305	4.8	318.5	ОК
8640	min	Winter	99.233	0.233	4.8	240.6	ОК
10080	min	Winter	99.176	0.176	4.7	180.4	O K

	Stor	m 📄	Rain	Flooded	Discharge	Time-Peak
4	Even	t	(mm/hr)	Volume	Volume	(mins)
_				(m³)	(m³)	
60	min	Winter	66.721	0.0	559.9	70
120	min	Winter	38.443	0.0	645.3	128
180	min	Winter	27.846	0.0	701.3	186
240	min	Winter	22.150	0.0	735.3	244
360	min	Winter	16.044	0.0	759.0	362
480	min	Winter	12.763	0.0	757.2	478
600	min	Winter	10.687	0.0	750.9	594
720	min	Winter	9.244	0.0	743.7	710
960	min	Winter	7.381	0.0	728.8	938
1440	min	Winter	5.375	0.0	700.9	1380
2160	min	Winter	3.914	0.0	1182.9	1968
2880	min	Winter	3.125	0.0	1259.5	2228
4320	min	Winter	2.192	0.0	1320.9	3036
5760	min	Winter	1.705	0.0	1374.1	3864
7200	min	Winter	1.402	0.0	1413.1	4608
8640	min	Winter	1.196	0.0	1445.8	5280
10080	min	Winter	1.045	0.0	1474.1	5952

BWB Consulting Ltd		Page 3
5th Floor, Waterfront House	BMW3250	
35 Station Street	Bretch Hill, Banbury Phase 2	
Nottingham, NG2 3DQ	FEH Storage	Micro
Date 06/10/2021 21:42	Designed by Ryan.Davies	Drainage
File FEH Storage.SRCX	Checked by	pran laye
Innovyze	Source Control 2020.1	•

Rainfall Details

Rainfall Model		FEH
Return Period (years)		100
FEH Rainfall Version		1999
Site Location	GB 443350 239550 SP 43350	39550
C (1km)	_	-0.024
D1 (1km)		0.315
D2 (1km)		0.328
D3 (1km)		0.236
E (1km)		0.300
F (1km)		2.484
Summer Storms		Yes
Winter Storms		Yes
Cv (Summer)		0.750
Cv (Winter)		0.840
Shortest Storm (mins)		15
Longest Storm (mins)		10080
Climate Change %		+40

Time Area Diagram

Total Area (ha) 1.000

Time	(mins)		II.			1	(mins)	
From:	To:	(ha)	From:	To:	(ha)	From:	To:	(ha)
0	4	1 0.333	4	\8	0.333	8	12	0.333

BWB Consulting Ltd		Page 4
5th Floor, Waterfront House	BMW3250	
35 Station Street	Bretch Hill, Banbury Phase 2	
Nottingham, NG2 3DQ	FEH Storage	Micro
Date 06/10/2021 21:42	Designed by Ryan.Davies	Drainage
File FEH Storage.SRCX	Checked by	Dialilade
Innovyze	Source Control 2020.1	1

Model Details

Storage is Online Cover Level (m) 100.000

Tank or Pond Structure

Invert Level (m) 99.000

Depth (m) Area (m^2) Depth (m) Area (m^2)

0.000 993.3 1.000 1356.7

Hydro-Brake® Optimum Outflow Control

Unit Reference MD-SHE-0103-4800-1050-4800 Design Head (m) 1.050 Design Flow (1/s) 4.8 Flush-Flo™ Calculated Objective Minimise upstream storage Application Surface Sump Available Diameter (mm) 103 Invert Level (m) 98.950 Minimum Outlet Pipe Diameter (mm) 150 Suggested Manhole Diameter (mm) 1200

Control Points Head (m) Flow (1/s)

Design Point (Calcula	ted) 1.050 4.8	
Flush-	Florm 0.311 4.8	
Kick-	Flo® 0.666 3.9	,
Mean Flow over Head R	ange - 4.2	

The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated

Depth (m) Fl	ow (1/s)	epth (m) Flo	w (1/s)	Depth (m) Flow	7 (1/s)	Depth (m)	Flow $(1/s)$
0.100	3.4	1.200	5.1	3.000	7.8	7.000	11.7
0.200	4.6	1.400	5.5	3.500	8.4	7.500	12.1
0.300	4.8	1.600	5.8	4.000	9.0	8.000	12.5
0.400	4.7	1.800	6.2	4.500	9.5	8.500	12.9
0.500	4.6	2.000	6.5	5.000	10.0	9.000	13.2
0.600	4.3	2.200	6.8	5.500	10.4	9.500	13.6
0.800	4.2	2.400	7.1	6.000	10.9		
1.000	4.7	2.600	7.3	6.500	11.3		

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BWB Consulting Ltd				
5th Floor, Waterfront House	BMW3250			
35 Station Street	Bretch Hill, Banbury Phase 2			
Nottingham, NG2 3DQ	FSR Storage	Micro		
Date 06/10/2021 21:43	Designed by Ryan.Davies	Drainage		
File FSR Storage.SRCX	Checked by	Dialilade		
Innovyze	Source Control 2020.1			

Summary of Results for 100 year Return Period (+40%)

Storm Event		Max Level (m)	Max Depth (m)	Max Control (1/s)	Max Volume (m³)	Status	
15	min	Summer	99.242	0.242	4.8	250.5	ОК
30	min	Summer	99.312	0.312	4.8	326.6	O K
60	min	Summer	99.380	0.380	4.8	402.7	O K
120	min	Summer	99.444	0.444	4.8	474.6	O K
180	min	Summer	99.476	0.476	4.8	511.5	ОК
240	min	Summer	99.494	0.494	4.8	533.2	O K
360	min	Summer	99.515	0.515	4.8	556.9	O K
480	min	Summer	99.525	0.525	4.8	568.7	O K
600	min	Summer	99.528	0.528	4.8	573.0	ОК
720	min	Summer	99.528	0.528	4.8	572.3	O K
960	min	Summer	99.519	0.519	4.8	561.7	O K
1440	min	Summer	99.494	0.494	4.8	533.1	O K
2160	min	Summer	99.460	0.460	4.8	492.9	O K
2880	min	Summer	99.427	0.427	4.8	455.4	O K
4320	min	Summer	99.367	0.367	4.8	387.2	O K
5760	min	Summer	99.312	0.312	4.8	327.0	ОК
7200	min	Summer	99.265	0.265	4.8	274.7	O K
8640	min	Summer	99.223	0.223	4.8	230.0	O K
10080	min	Summer	99.188	0.188	4.7	192.9	O K
15	min	Winter	99.271	0.271	4.8	281.3	O K
30	min	Winter	99.348	0.348	4.8	366.8	O K

	Storm		Rain	Flooded	Discharge	Time-Peak
	Even	t	(mm/hr)	Volume	Volume	(mins)
				(m³)	(m³)	
15	min	Summer	136.710	0.0	255.9	26
30	min	Summer	89.520	0.0	335.2	41
60	min	Summer	55.837	0.0	418.2	70
120	min	Summer	33.658	0.0	504.5	130
180	min	Summer	24.706	0.0	555.4	188
240	min	Summer	19.726	0.0	591.2	248
360	min	Summer	14.299	0.0	643.0	366
480	min	Summer	11.384	0.0	682.6	486
600	min	Summer	9.532	0.0	714.4	604
720	min	Summer	8.241	0.0	731.2	722
960	min	Summer	6.546	0.0	747.7	956
1440	min	Summer	4.725	0.0	736.5	1154
2160	min	Summer	3.404	0.0	918.5	1520
2880	min	Summer	2.696	0.0	970.0	1932
4320	min	Summer	1.937	0.0	1045.5	2728
5760	min	Summer	1.531	0.0	1102.0	3512
7200	min	Summer	1.275	0.0	1146.8	4256
8640	min	Summer	1.097	0.0	1184.6	4944
10080	min	Summer	0.966	0.0	1217.3	5656
15	min	Winter	136.710	0.0	286.6	26
30	min	Winter	89.520	0.0	364.3	41

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5th Floor, Waterfront House	BMW3250			
35 Station Street	Bretch Hill, Banbury Phase 2			
Nottingham, NG2 3DQ	FSR Storage	Micro		
Date 06/10/2021 21:43	Designation of last Designation	Drainage		
File FSR Storage.SRCX	Checked by	Dialilade		
Innovyze	Source Control 2020.1			

Summary of Results for 100 year Return Period (+40%)

Storm Event		Max Level (m)	Max Depth (m)	Max Control (1/s)	Max Volume (m³)	Status	
60	min	Winter	99.425	0.425	4.8	453.0	ОК
120	min	Winter	99.496	0.496	4.8	535.5	O K
180	min	Winter	99.533	0.533	4.8	578.8	ОК
240	min	Winter	99.556	0.556	4.8	605.3	O K
360	min	Winter	99.581	0.581	4.8	636.0	ОК
480	min	Winter	99.596	0.596	4.8	653.5	O K
600	min	Winter	99.604	0.604	4.8	662.6	O K
720	min	Winter	99.606	0.606	4.8	666.1	ОК
960	min	Winter	99.603	0.603	4.8	662.4	ОК
1440	min	Winter	99.577	0.577	4.8	631.0	O K
2160	min	Winter	99.529	0.529	4.8	573.5	O K
2880	min	Winter	99.483	0.483	4.8	519.8	O K
4320	min	Winter	99.393	0.393	4.8	416.8	O K
5760	min	Winter	99.311	0.311	4.8	325.9	O K
7200	min	Winter	99.242	0.242	4.8	249.9	ОК
8640	min	Winter	99.185	0.185	4.7	189.3	ОК
10080	min	Winter	99.140	0.140	4.6	142.9	O K

Storm		Rain	Flooded	Discharge	Time-Peak	
	Event		(mm/hr) Volume		Volume	(mins)
_				(m³)	(m³)	
60	min	Winter	55.837	0.0	468.6	70
120	min	Winter	33.658	0.0	565.0	128
180	min	Winter	24.706	0.0	622.1	186
240	min	Winter	19.726	0.0	662.3	244
360	min	Winter	14.299	0.0	720.2	360
480	min	Winter	11.384	0.0	745.9	478
600	min	Winter	9.532	0.0	757.0	594
720	min	Winter	8.241	0.0	757.8	708
960	min	Winter	6.546	0.0	749.5	934
1440	min	Winter	4.725	0.0	729.0	1360
2160	min	Winter	3.404	0.0	1028.8	1668
2880	min	Winter	2.696	0.0	1086.5	2108
4320	min	Winter	1.937	0.0	1171.1	2948
5760	min	Winter	1.531	0.0	1234.1	3752
7200	min	Winter	1.275	0.0	1284.4	4472
8640	min	Winter	1.097	0.0	1326.8	5184
10080	min	Winter	0.966	0.0	1363.3	5848

BWB Consulting Ltd				
5th Floor, Waterfront House	BMW3250			
35 Station Street	Bretch Hill, Banbury Phase 2			
Nottingham, NG2 3DQ	FSR Storage	Micro		
Date 06/10/2021 21:43	Designed by Ryan.Davies	Drainage		
File FSR Storage.SRCX	Checked by	Dialilade		
Innovyze	Source Control 2020.1	•		

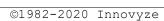
Rainfall Details

Return Period (years) 100 Cv (Summer) 0.750
Region England and Wales Cv (Winter) 0.840
M5-60 (mm) 19.700 Shortest Storm (mins) 15
Ratio R 0.407 Longest Storm (mins) 10080
Summer Storms Yes Climate Change % +40

Time Area Diagram

Total Area (ha) 1.000

	(mins)							
From:	To:	(ha)	From:	To:	(ha)	From:	To:	(ha)
0	4	0.333	4	8	0.333	8	12	0.333



BWB Consulting Ltd					
5th Floor, Waterfront House	BMW3250				
35 Station Street	Bretch Hill, Banbury Phase 2				
Nottingham, NG2 3DQ	FSR Storage	Micro			
Date 06/10/2021 21:43	Designed by Ryan.Davies	Drainage			
File FSR Storage.SRCX	Checked by	Dialilade			
Innovyze	Source Control 2020.1	1			

Model Details

Storage is Online Cover Level (m) 100.000

Tank or Pond Structure

Invert Level (m) 99.000

Depth (m) Area (m²) Depth (m) Area (m²)

0.000 993.3 1.000 1356.7

Hydro-Brake® Optimum Outflow Control

Unit Reference MD-SHE-0103-4800-1050-4800 Design Head (m) 1.050 Design Flow (1/s) 4.8 Flush-Flo™ Calculated Objective Minimise upstream storage Application Surface Sump Available Diameter (mm) 103 Invert Level (m) 98.950 Minimum Outlet Pipe Diameter (mm) 150 Suggested Manhole Diameter (mm) 1200

Control Points Head (m) Flow (1/s)

Design Point	(Calculated)	1.050	4.8
	Flush-Flo™	0.311	4.8
	Kick-Flo®	0.666	3.9
Mean Flow over	Head Range	_	4.2

The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated

Depth (m) Flo	ow (1/s) De	pth (m) Flo	w (1/s)	Depth (m) Flow	(1/s)	Depth (m)	Flow (1/s)
0.100	3.4	1.200	5.1	3.000	7.8	7.000	11.7
0.200	4.6	1.400	5.5	3.500	8.4	7.500	12.1
0.300	4.8	1.600	5.8	4.000	9.0	8.000	12.5
0.400	4.7	1.800	6.2	4.500	9.5	8.500	12.9
0.500	4.6	2.000	6.5	5.000	10.0	9.000	13.2
0.600	4.3	2.200	6.8	5.500	10.4	9.500	13.6
0.800	4.2	2.400	7.1	6.000	10.9		
1.000	4.7	2.600	7.3	6.500	11.3		

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