

SuDS Flows and Volumes - LLFA Technical Assessment Pro-forma

This form identifies the information required by Oxfordshire County Council LLFA to enable technical assessment of flows and volumes determined as part of drainage / SuDS calculations.

Note : * means delete as appropriate; Numbers in brackets refer to accompanying notes.

SITE DETAILS

- 1.1 Planning application reference 21/02286/F
- 1.2 Site name Land Northwest of Launton Roundabout
Adjoining Skimmingdish Lane
- 1.3 Total application site area (1) 11800m²ha
- 1.4 Is the site located in a CDA or LFRZ Y/N
- 1.5 Is the site located in a SPZ Y/N

VOLUME AND FLOW DESIGN INPUTS

- 2.1 Site area which is positively drained by SuDS (2) 1757m²
- 2.2 Impermeable area drained pre development (3) 0m²
- 2.3 Impermeable area drained post development (3) 1757m²
- 2.4 Additional impermeable area (2.3 minus 2.2) 1757m²
- 2.5 Predevelopment use (4) Greenfield / Brownfield / Mixed*
- 2.6 Method of discharge (5) Infiltration / waterbody / storm sewer/ combined sewer*
- 2.7 Infiltration rate (where applicable) 0.012m/hr
- 2.8 Influencing factors on infiltration
- 2.9 Depth to highest known ground watertable.....mAOD Groundwater not encountered in S1
- 2.10 Coefficient of runoff (Cv) (6) 0.75
- 2.11 Justification for Cv used only areas of impermeable surface used in calcs
- 2.12 FEH rainfall data used (Note that FSR is no longer the preferred rainfall calculation method) Y/N
- 2.13 Will storage be subject to surcharge by elevated water levels in watercourse/ sewer Y/N site in 100 year flood plain
- 2.14 Invert level at outlet (invert level of final flow control) 68.335mAOD
- 2.15 Design level used for surcharge water level at point of discharge (14) 68.050mAOD

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CALCULATION OUTPUTS

Sections 3 and 4 refer to site where storage is provided by attenuation and/or partial infiltration. Where all flows are infiltrated to ground omit Sections 3-5 and complete Section 6.

3.0 Defining rate of runoff from the site

- 3.2 Max. discharge for 1 in 1 year rainfalll/s/ha, 2.4 l/s for the site
- 3.2 Max. discharge for Q_{med} rainfalll/s/ha,l/s for the site
- 3.3 Max. discharge for 1 in 30 year rainfalll/s/ha, 4.5 l/s for the site
- 3.4 Max. discharge for 1 in 100 year rainfalll/s/ha, 5.0 l/s for the site
- 3.5 Max. discharge for 1 in 100 year plus 40% CCl/s/ha, 5.0 l/s for the site

4.0 Attenuation storage to manage peak runoff rates from the site

- 4.1 Storage - 1 in 1 year 15.5 m³ 0.09 m³/m² (of developed impermeable area)
- 4.2 Storage - 1 in 30 year (7) 37.9 m³ 0.022 m³/m²
- 4.3 Storage - 1 in 100 year (8) 51.4 m³ 0.029 m³/m²
- 4.4 Storage - 1 in 100 year plus 40% CC (9) 71.4 m³ 0.044 m³/m²

5.0 Controlling volume of runoff from the site

- 5.1 Pre development runoff volume⁽¹⁾ 128 m³ for the site
- 5.2 Post development runoff volume (unmitigated) (1) 128 m³ for the site
- 5.3 Volume to be controlled/does not leave site (5.2-5.1) 128 m³ for the site
- 5.4 Volume control provided by
- | | |
|---|---------------------|
| Interception losses ⁽¹¹⁾ |m ³ |
| Rain harvesting ⁽¹²⁾ |m ³ |
| Infiltration (even at very low rates) |m ³ |
| Separate area designated as long term storage ⁽¹³⁾ |m ³ |
- 5.5 Total volume control (sum of inputs for 5.4)m³ (15)

6.0 Site storage volumes (full infiltration only)

- 6.1 Storage - 1 in 30 year (7) 37.9 m³ 0.022 m³/m² (of developed impermeable area)
- 6.2 Storage - 1 in 100 year plus CC (9) 71.4 m³ 0.044 m³/m²