

FLOOD MODELLING REPORT

Firethorn Developments Limited

Land at North West Bicester

October 2021

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1 Introduction

Authorisation

1.1 This flood modelling study has been undertaken by Vectos on behalf of Firethorn Developments Limited. This has been undertaken to support the outline planning application (21/01630/OUT) at North West Bicester.

Background

1.2 Town Brook, an ordinary watercourse, flows alongside the east boundary of the site. A tributary of Town Brook, another ordinary watercourse, flows alongside the south boundary of the site (these are further described in Section 3).

1.3 The Environment Agency Flood Map for Planning (available from <https://flood-map-for-planning.service.gov.uk/>) identifies the fluvial flood extents across the site associated with Town Brook. These are presented in Figure 1, which shows that Town Brook results in some relatively limited flooding alongside the east site boundary.



Figure 1: Flood Zone 3 and Flood Zone 2

1.4 Typically, the Flood Map for Planning excludes watercourses with catchment areas less than 3 km². This includes the tributary on the south site boundary. However, flooding associated with this tributary is presented on the Risk of Flooding from Surface Water map. These surface water flood

zones are presented in Figure 2. Similar to Town Brook, some relatively limited flooding is identified alongside the site boundary.

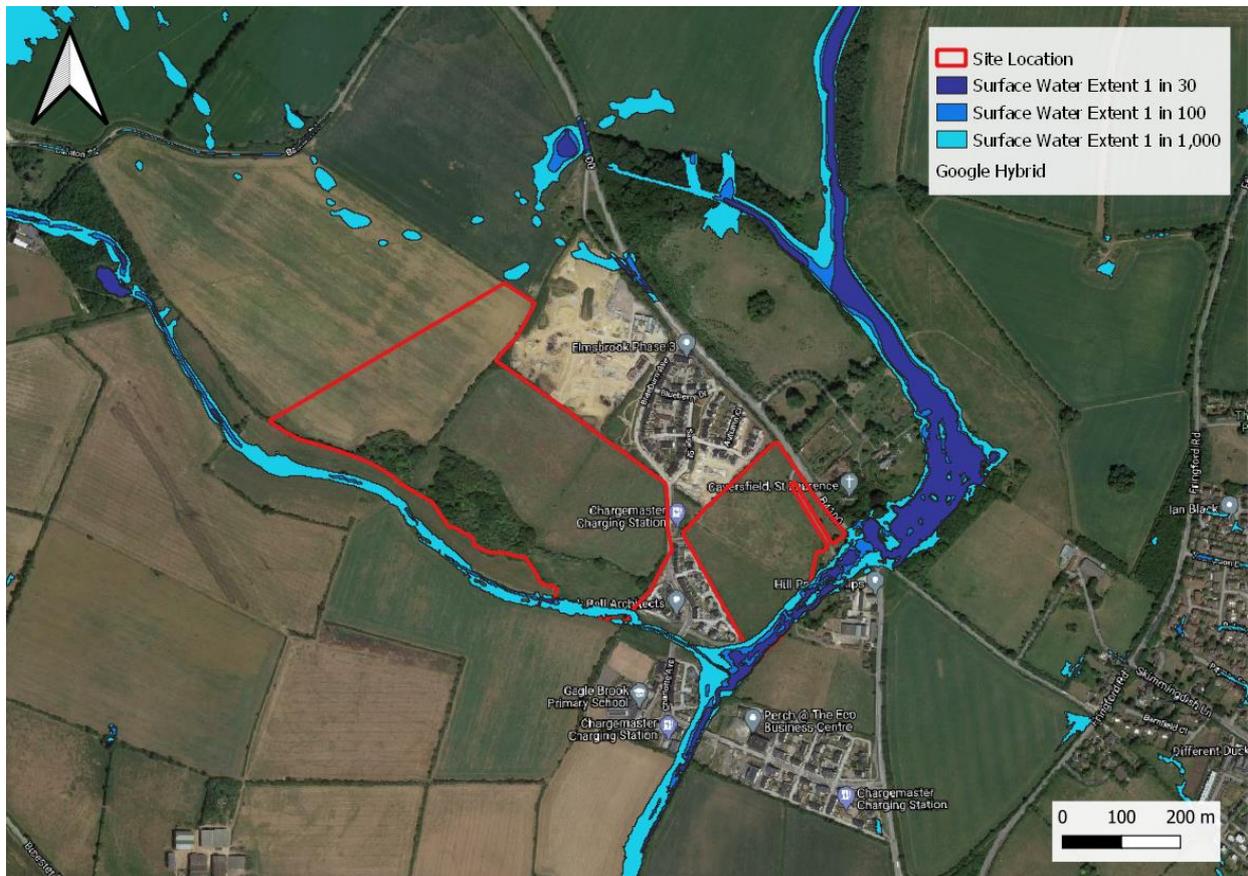


Figure 2: Surface Water Flood Zones

- 1.5 In the proximity of the site, the Flood Map for Planning has been derived using JFLOW. This is a broad-brush flood modelling technique, which was developed as part of the national flood mapping programme. The methodology takes little or no account of watercourse capacity, nor does it allow for hydraulic structures, and is often found to result in an overestimation of flooding. A similar approach was adopted for the Risk of Flooding from Surface Water map and the accuracy of the associated flood mapping can be poor.
- 1.6 The Flood Risk Assessment (FRA) prepared to support the planning application at the site considered these sources of flood risk and made allowances for climate change. However, following their review of the FRA, the Environment Agency (EA) stipulated the need for hydraulic modelling to obtain a more reliable estimation of flood risk, which should be used for comparison to development proposals.

Purpose of this Report

- 1.7 The purpose of this report is to summarise details of the flood modelling undertaken. The flood modelling objectives were defined at the outset of this study, these were:
 - i) Derive hydrological boundaries (i.e. design inflows) for the study area;

- ii) Develop a baseline 1D-2D hydraulic model of the site, extending both up and downstream;
 - iii) Simulate the hydraulic model for a series of design inflow events;
 - iv) Derive baseline flood maps for the site for comparison to the existing flood zones, Land Use Parameters Plans, Framework Plan and Illustrative Masterplan.
- 1.8 The work has been undertaken in accordance with the guidelines set out in the National Planning Policy Framework (NPPF).

2 Consultation

- 2.1 A modelling methodology was prepared and issued to the EA for comment. The EA response was used to inform the preparation of the flood model. Details are enclosed in Appendix A.

3 Flood Model Development

3.1 The flood model has been developed to improve the understanding of fluvial flood risk at the site. The flood model comprises of two parts: the hydrological model and the hydraulic model. Both of which are discussed in this section.

Hydrological Modelling

3.2 The hydrological assessment allows for the calculation of watercourse flows, or design hydrographs, which are subsequently used as inflow boundary conditions in the hydraulic model.

3.3 The hydrographs were estimated using the latest methodologies described in the Flood Estimation Handbook (FEH). Ultimately, the statistical method has been selected to derive design hydrographs at the site due to the permeable nature of the catchment. This method was found to result in slightly lower peak flows than the ReFH (2.2) method. Full details of the hydrological analysis are outlined in Appendix B.

3.4 Design hydrographs have been derived to represent three separate runoff contributions. These relate to two upstream catchment areas and from within the model reach itself (i.e. applied as lateral inflows). Figure 3 shows the extents of the contributing areas for which design hydrographs have been derived.

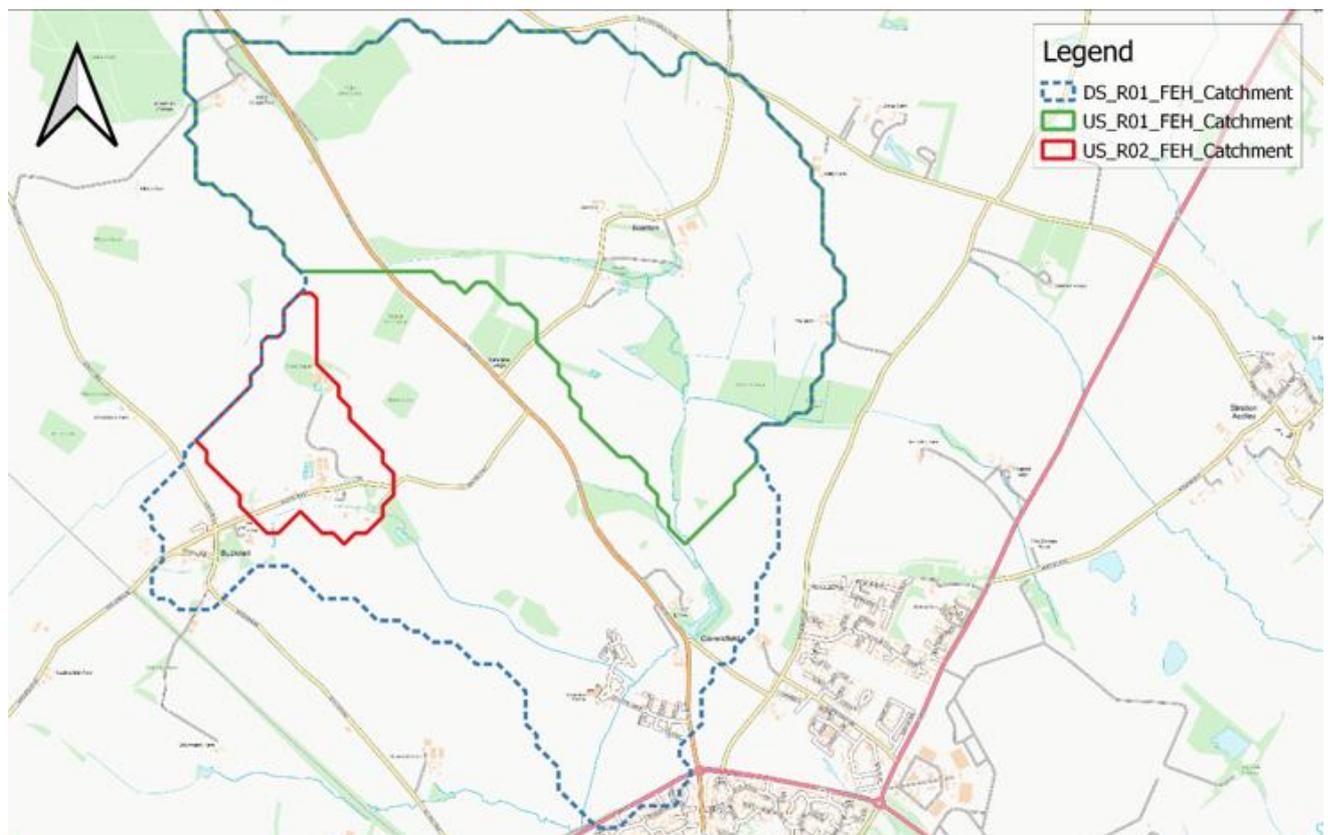


Figure 3: Hydrological Areas

3.5 Table 1 presents a summary of the peak flows associated with each design event, for the total catchment area. It also includes the size of each catchment, which has been used to scale the peak flows. These scaled peak flows are also identified.

Table 1: Estimated Peak Flows (m³/s)

Design Event	Catchment			
	Total	Reach 1	Reach 2	Lateral
	7.60 km ²	3.88 km ²	0.53 km ²	3.19 km ²
1 in 20	0.64	0.33	0.04	0.27
1 in 100	0.98	0.50	0.41	0.07
1 in 100 + 15%CC	1.13	0.58	0.47	0.08
1 in 100 + 25%CC	1.23	0.63	0.51	0.09
1 in 1000	1.74	0.89	0.73	0.12

Baseline Hydraulic Modelling

3.6 The baseline hydraulic model has been developed using TUFLOW software. TUFLOW is an industry standard computational engine that provides one-dimensional (1D) and two-dimensional (2D) solutions of the free-surface flow equations to simulate hydrodynamic behaviour in river, floodplains and urban drainage environments. The 1D component uses a tool built into TUFLOW, called ESTRY. The TUFLOW software version used was 2020-10-AA-iDP-w64.

3.7 General details concerning the hydraulic model development, parameters and assumptions are outlined in the following sections.

Hydraulic Model Approach and Extent

3.8 A linked 1D-2D model has been developed including the entire extent of the existing flood zones on site. It has also extended 600 m upstream and 800 m downstream of the site. A 1D model of the stream network and hydraulic structures has been incorporated to accurately represent in channel hydraulics, with this linked to a 2D floodplain domain.

3.9 The 1D model extends across the full length of the 2D model. The downstream extent was chosen because it was located sufficiently far from the site in order to minimise boundary effects.

3.10 Figure 4 illustrates the extent of the 1D and 2D model.

Model Grid Size

3.11 The model was constructed with a 2 m grid cell size. This was chosen as it represented a good balance between the degree of precision (i.e. ability to model overland flow paths along roads or through ditches) and model run (“simulation”) times.

Topographic Data

- 3.12 1 m resolution LiDAR (Light Detection And Ranging) data was downloaded to provide a Digital Terrain Map (DTM) of the study area including the site and surrounding catchment. This covered the entire 2D model extent and was used to represent floodplain ground levels.



Figure 4: 1D and 2D Model Extent

- 3.13 The site topographic survey was used to validate the LiDAR data. The two data sets were found to compare well across the site. Consequently, the LiDAR was considered to be acceptable for use as part of the hydraulic model.
- 3.14 A cross section topographic survey of the watercourse and hydraulic structures was captured across the full length of the 1D model. Details are enclosed in Appendix C.

Upstream Boundary Conditions

- 3.15 The hydrographs derived as part of the hydrological modelling were incorporated into the hydraulic model as upstream boundary conditions (i.e. inflows). The two upstream catchments were incorporated into the most upstream 1D cross section / node, for Reach 1 and Reach 2. The lateral inflows were proportioned across the model reach (see Figure 5).

Downstream Boundaries

- 3.16 A stage-discharge boundary was added to allow water to exit the 2D model at the downstream extent. The stage-discharge relationship is calculated automatically by TUFLOW based on gradients determined from LiDAR data.
- 3.17 A fixed water level of 78.14 m AOD was applied as a downstream boundary for the 1D model. This was the surveyed water level and is thought to represent a typical likely dry weather flow condition. Alternative, more conservative, methods for a downstream boundary condition were tested but introduced instability into the model during model testing.



Figure 5: Inflow Locations

- 3.18 The results in the proximity of the site were not considered to be sensitive to this boundary condition given the distance and level differences. However, the sensitivity of the model to this downstream boundary conditions was tested and is discussed in Section 5 of this report.

Structures

- 3.19 The watercourse survey captured a total of 21 structures, which are summarised in Table 2. All surveyed structures were included in the hydraulic model. Figure 6 presents the location of these various structures.
- 3.20 On Reach 1 a lake is present upstream of the B4100 (see Figure 7). Water flows both in and out of the lake at the same location (at node 01450). However, an overflow is present at the downstream

extent of the lake, which flows in a culvert beneath the B4100. The water level in the lake was set to be full (i.e. 85.04 m AOD, which is equivalent to the invert level of the culvert that provides flow into it).

- 3.21 The only surveyed feature that was ignored is a small pond located at the upstream extent of Reach 2. The modelled watercourse supplies flow to this pond, but its inclusion was thought to offer no benefit to the accuracy of the model results. In theory it could offer some storage potential, so its exclusion was conservative.



Figure 6: Structure Locations

Floodplain Topography

- 3.22 The only change to the floodplain topography was the inclusion of a stone wall that surrounds the lake. A gap in this wall was included, at the location of a small gate.

Hydraulic Roughness

- 3.23 Hydraulic roughness is the measure of the amount of frictional resistance water experiences when passing over land and channel features. Manning’s ‘n’ coefficient values provide a measure of these roughness values.
- 3.24 Roughness values are variable and include a default value of 0.06 for arable land and 0.02 for roads, for example. Limited variation in Manning’s values was noted across the small rural model extent.

3.25 Roughness land uses were defined using aerial photography, which was considered adequate given the rural floodplain and small variation in land use.



Figure 7: Lake and Wall Locations



Figure 8: Wall Surrounding the Lake

Linking

- 3.26 The 2D model was dynamically linked to the 1D model using a 'HX' boundary, which transfers flows based on a water level between the two models.

Model Run Parameters

- 3.27 The default TUFLOW Classic model run parameters were used and adopted for all simulations.

Design Runs

- 3.28 The baseline and proposed hydraulic models were simulated for the following design events:
- i) 1 in 20 year event.
 - ii) 1 in 100 year event.
 - iii) 1 in 100 year event plus a 15% allowance for climate change event (central estimate).
 - iv) 1 in 100 year event plus a 25% allowance for climate change event (higher central estimate).
 - v) 1 in 1000 year event.
- 3.29 These climate change allowances are in accordance with latest Planning Practice Guidance, which indicates that the central estimate (i.e. 15%) is applicable for the site. It is this 1 in 100 year event plus a 15% allowance for climate change event that represents the key design event which development must be protected from.
- 3.30 Based on the recommended ranges for the cell size, a 0.5 second timestep was adopted for the 2D TUFLOW model. A 0.25 second timestep was adopted for the 1D component.

4 Model Results

Model Stability

- 4.1 The stability of a model is critical to the understanding of the robustness and its ability to simulate a flood event accurately.
- 4.2 Stability in the model is assessed by examining the cumulative error (or mass balance) of the model, as well as the warnings output by the model during the simulation.
- 4.3 The 1 in 100 year plus 15% climate change event was run using TUFLOW Classic. Iterations of the model were undertaken to improve stability etc, where necessary. The cumulative error of the model is within the range of $\pm 1.3\%$ throughout the simulation. No negative depths were encountered. Various warnings and checks occurred prior to the simulation, which were found to be of little consequence and were therefore considered acceptable.
- 4.4 Similar observations were made for the wider design simulations.

Baseline Model Results

- 4.5 The results have been summarised through the presentation of a flood map showing the various design events. This is shown in Figure 9. Further flood mapping is enclosed in Appendix D.
- 4.6 It can be seen that in a 1 in 20 year event, flooding is limited to the confluence of the two streams. However, in a 1 in 100 year event, flooding on the east site boundary occurs. This is a result of the capacity restriction associated with a series of hydraulic structures downstream of the B4100. Under the model extreme events, the flood extents become broader but limited other areas experience flooding. It is noted that floodwater is contained within the banks of Reach 2. The downstream reach of the model first floods in a 1 in 1000 year event.
- 4.7 The 1 in 1000 year flood event generates the most extensive floodplain out of the various design events investigated. Figure 10 compares this with the flood extent on the Flood Map for Planning (i.e. Flood Zone 2). It shows that the flood extents derived from this study are smaller than that presented on the Flood Map for Planning. They are also smaller than the flood extents presented on the Risk of Flooding from Surface Water Map (Figure 12).
- 4.8 As part of the FRA prepared to support the planning application, a climate change flood extent was derived based on the extrapolation of JFLOW flood levels. The resultant flood extent is shown in Figure 13. This extrapolated flood extent, along with the flood extents presented on the Flood Map for Planning and Risk of Flooding from Surface Water Map, were all originally used to inform the Land Use Parameters Plans, Framework Plan and Illustrative Masterplan associated with the planning submission. This ensured that the proposed development (including SuDS) was entirely steered out of the floodplain.
- 4.9 The flood extents prepared as part of this flood modelling study are no greater than those extents previously available and it can be concluded therefore, that the development remains safe from flooding without impacting on third parties. This is therefore in accordance with the requirements of planning policy.



Figure 9: All Flood Extents



Figure 10: Comparison of Modelled 1 in 1000 Year Event and Flood Zone 2



Figure 11: Comparison of Modelled 1 in 100 Year Event and Flood Zone 3



Figure 12: Comparison of Modelled 1 in 1000 Year Event and Surface Water Flood Extent

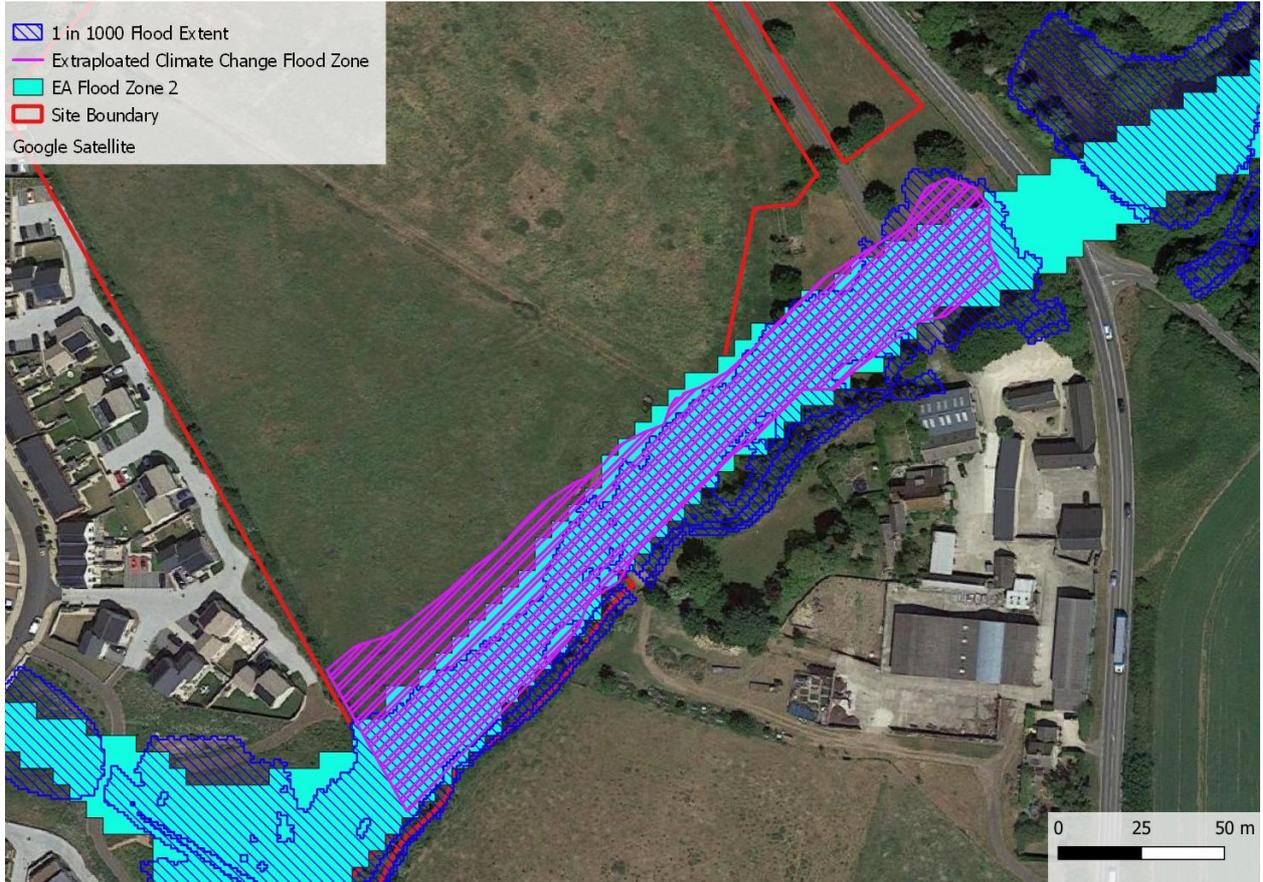


Figure 13: Comparison of Modelled 1 in 1000 Year Event and FRA Extrapolated Flood Zone

5 Sensitivity Testing

5.1 Sensitivity testing is the process of adjusting key parameters within the hydraulic model to examine the impact. This allows an understanding of how any uncertainty associated with those parameters may affect the outcome of the model. Sensitivity testing is often undertaken in the absence of data used for the calibration purposes. Sensitivity testing has been undertaken on the following parameters:

- i) Manning’s ‘n’ roughness;
- ii) Flow;
- iii) Downstream boundary condition; and
- iv) Blockages.

5.2 The sensitivity testing has been undertaken on the basis of the 1 in 100 year event.

Manning’s ‘n’ Roughness

5.3 Manning’s ‘n’ roughness was adjusted by $\pm 20\%$ in the channel and floodplain. The results of this sensitivity test are identified in Figure 14 and 15, which are difference maps compared to the baseline model.

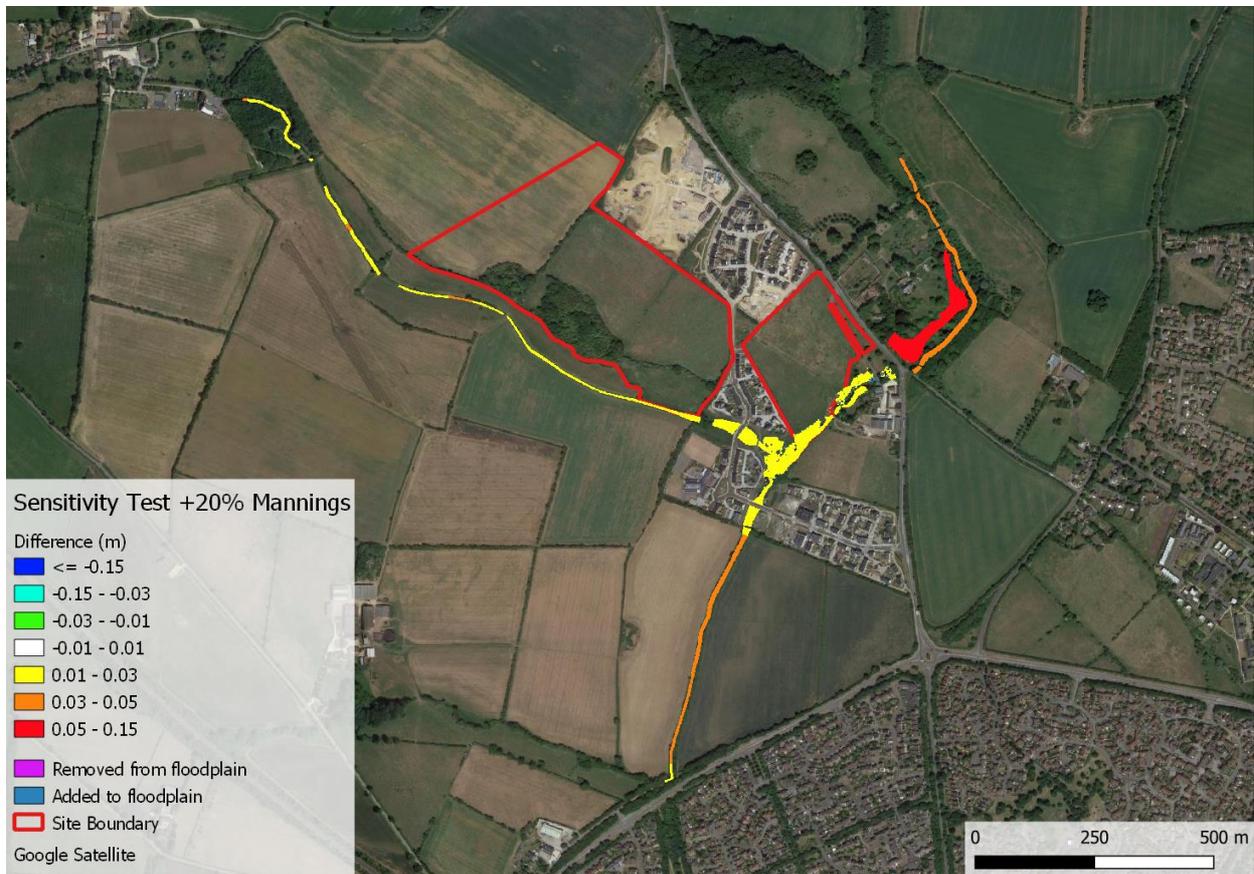


Figure 14: Manning’s Plus 20% Difference Map

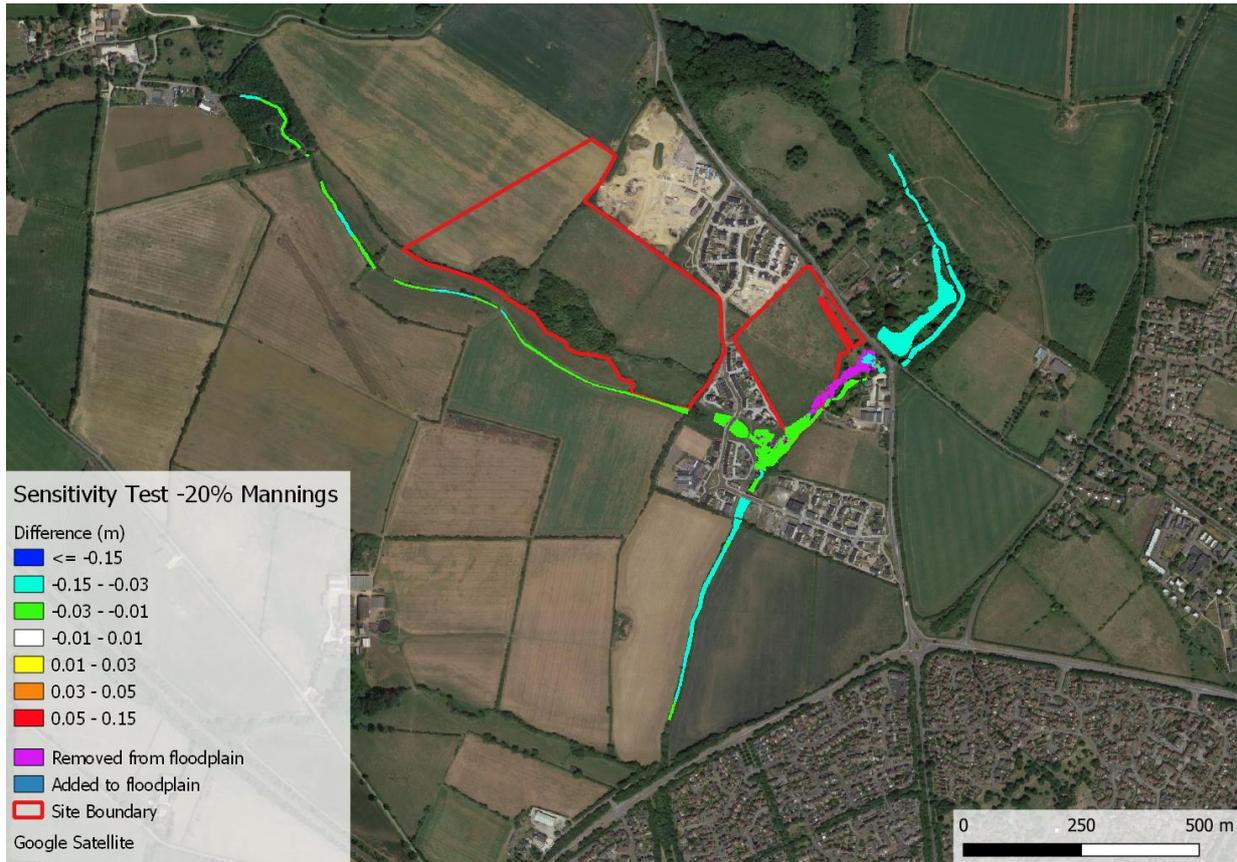


Figure 15: Manning's Minus 20% Difference Map

- 5.4 The results of the sensitivity testing show a similar impact in both the decreased and increased scenarios. The variation in peak water level is minor for what is a relatively significant adjustment to roughness values. The model is therefore not considered to be particularly sensitive to Manning's roughness.

Flow

- 5.5 The upstream boundary condition (i.e. inflow) was adjusted by $\pm 20\%$. The results of this sensitivity test are identified in Figure 16 and 17, which are difference maps compared to the baseline model.
- 5.6 The results of the sensitivity testing show a similar impact in both the decreased and increased scenarios. The variation in peak water level is minor for what is a relatively significant adjustment to inflow values. The model is therefore not considered to be particularly sensitive to inflows.

Downstream Boundary Conditions

- 5.7 The downstream boundary condition was modified by applying an elevated water level throughout the simulation in both 1D and 2D. This was increased from 78.14 m AOD to 79.5 m AOD. A level of 79.5 m AOD was determined based on the estimated flood level for the 1 in 1000 year event from the Flood Map for Planning. It meant that the culvert on the downstream boundary was submerged throughout the simulation in the sensitivity test. The results of this sensitivity test are identified in Figure 18, which is a difference map compared to the baseline model.

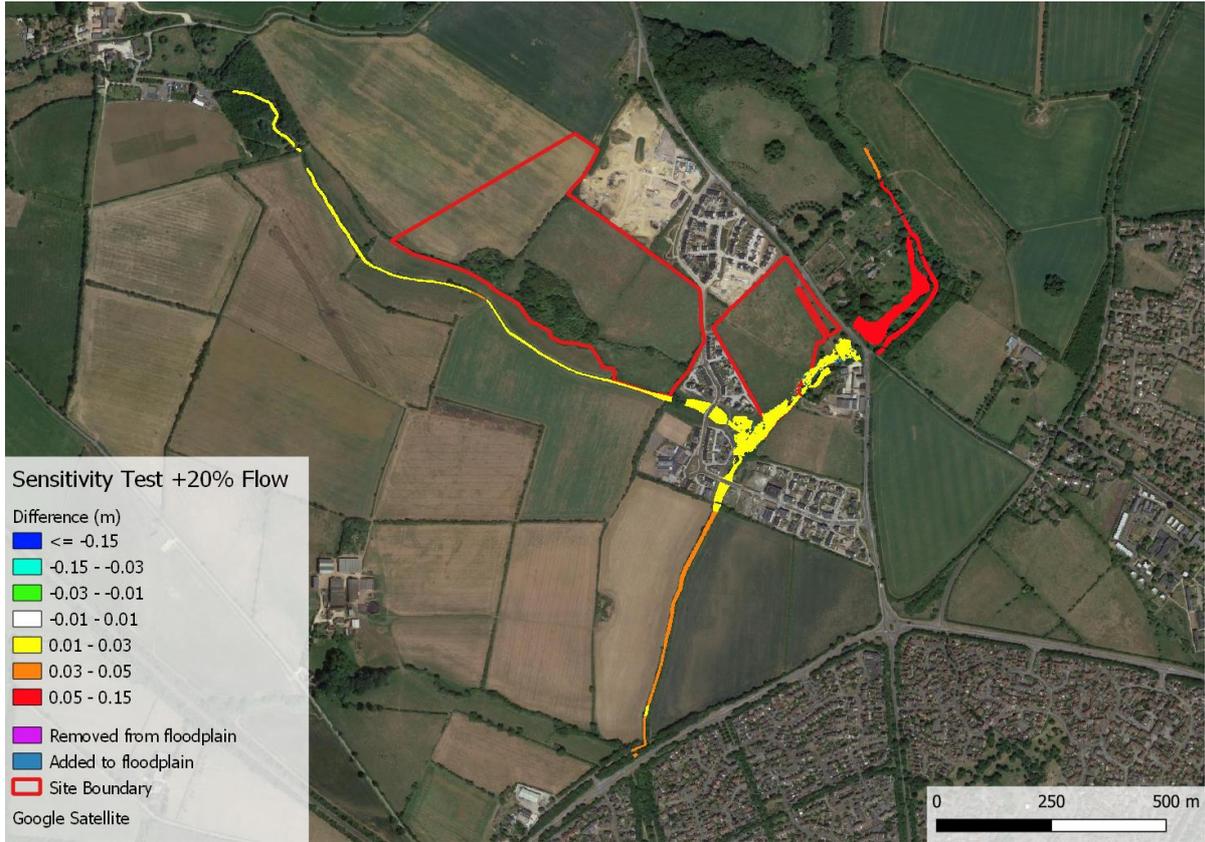


Figure 16: Flow Plus 20% Difference Map

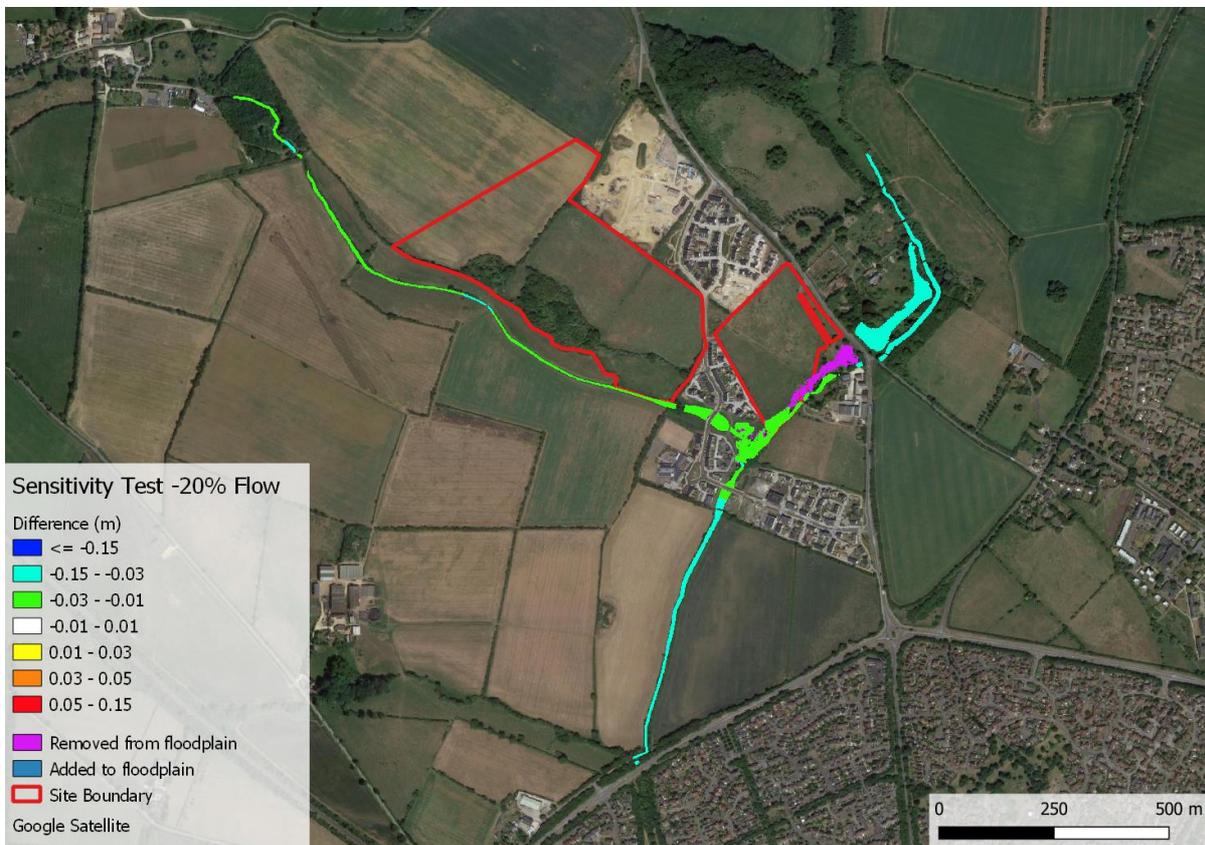


Figure 17: Flow Minus 20% Difference Map



Figure 18: Increased Downstream Boundary Difference Map

5.8 The impacts are significant at the downstream end of the hydraulic model, but with distance upstream from the downstream boundary, the effects become less significant. The effects of the downstream boundary were not felt at the site. This is important because it confirms that any uncertainty associated with the downstream boundary is likely to have no effect upon peak water levels at the site.

Summary

5.9 Generally, the watercourse was found to be relatively insensitive to adjustments of key parameters within the hydraulic model. Consequently, the model is considered to offer a robust representation of the watercourse for this assessment and peak water levels/flood extents are considered reliable.

Blockages

5.10 Whilst not required by the EA, as part of their model methodology review, the impact of blockages was also assessed. Following a site visit it was considered that the hydraulic structures most likely to increase flooding on site were beneath the B4100.

5.11 If these were to block, water would accumulate upstream in the proximity of the lake and spill towards the site once flood levels get high enough to flow through the gap/gate in the wall. As part of this blockage assessment, the lake was set to be full (i.e. above culvert inlet / outlet level and up to bank top). This conservative approach was not possible under a non-blocked scenario, because

water drains out of the lake into the stream at the start of the simulation. To achieve this, the culvert inlet / outlet was modelled as 100% blocked.

5.12 As series of very conservative blockage scenarios have been assessed, which are as described below:

1. Main culvert beneath B4100 100% blocked, lake full – see Figure 19
2. All culverts beneath B4100 100% blocked, lake full – see Figure 20
3. All culverts on site 100% blocked, lake not full – see Figure 21



Figure 19: Blockage Scenario 1

5.13 The flood depth results are presented in Figure 22 to 24. Ultimately, whilst different flood mechanisms are identified as a result of the three blockage scenarios the flood extents on site are no greater than that identified in the 1 in 1000 year event. Therefore, the conclusions drawn in Section 4 are still valid.



Figure 20: Blockage Scenario 2



Figure 21: Blockage Scenario 3



Figure 22: Flood Depth Map – Blockage Scenario 1

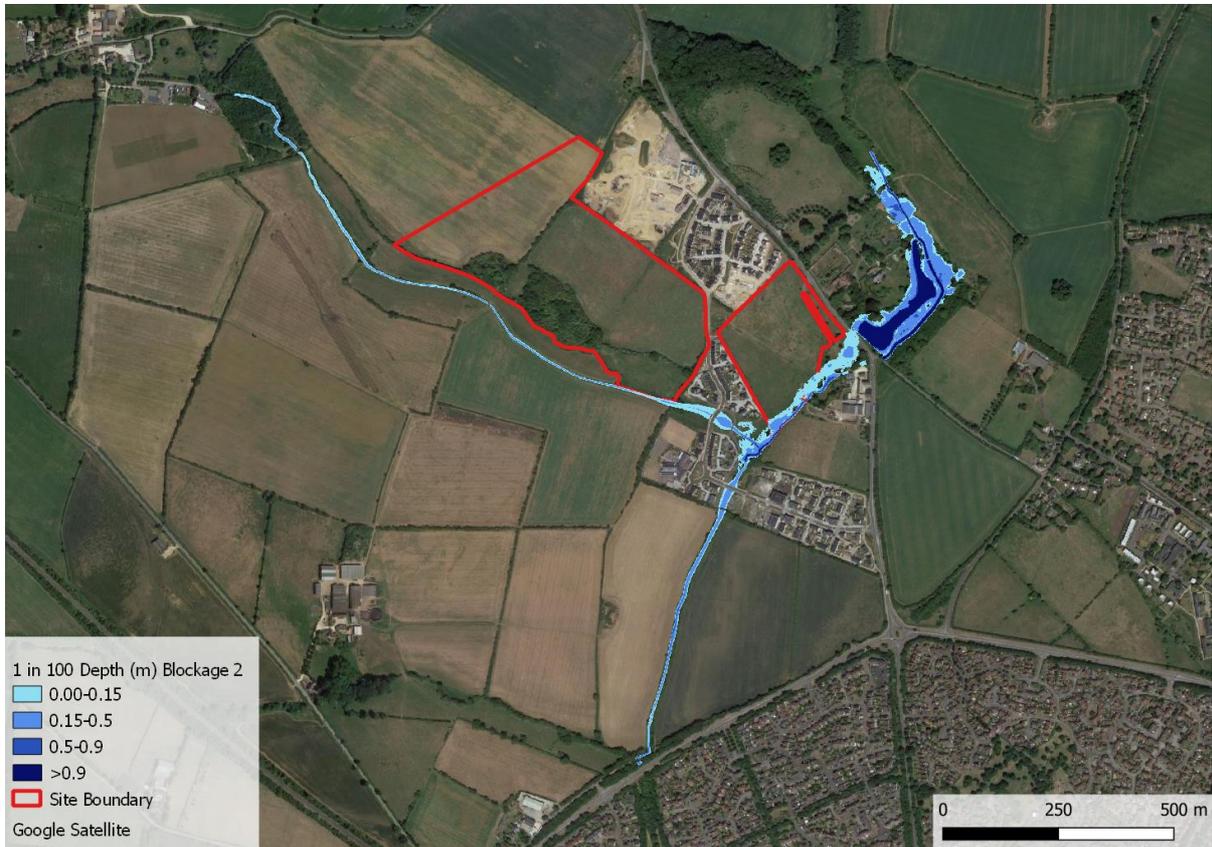


Figure 23: Flood Depth Map – Blockage Scenario 2



Figure 24: Flood Depth Map – Blockage Scenario 3

6 Conclusion

- 6.1 This flood modelling study has been undertaken by Vectos on behalf of Firethorn Developments Limited. This has been undertaken to support the outline planning application (21/01630/OUT) at North West Bicester.
- 6.2 Following the EA review of the planning submission, they stipulated the need for hydraulic modelling to obtain a reliable estimation of flood risk, which should be used for comparison to development proposals.
- 6.3 The flood modelling study has been completed using 1D-2D ESTRY-TUFLOW software to investigate the fluvial flood risk associated with two ordinary watercourses on the site boundary. This has shown that the existing Flood Map for Planning and Risk of Flooding from Surface Water map overestimate the floodplain in the proximity of the site.
- 6.4 Given that the Land Use Parameters Plans, Framework Plan and Illustrative Masterplan for the proposed development were all based on these desktop sources of flood risk, it can be concluded that the development remains safe from flooding without impacting on third parties.

Appendix 1

Environment Agency Correspondence

Mr Nick Bosanko
Vectos
4 Colston Avenue
Bristol
Avon
BS1 4ST

Our ref: WA/2021/129288/01-L01
Agreement No: ENVPAC/1/THM/00590
Your ref: 21/01630/OUT
Date: 04 October 2021

Dear Mr Bosanko

Planning advice for North West Bicester

Thank you for accepting our offer to provide detailed planning advice. The advice detailed below is intended to inform your development proposal. It is not our statutory response to a planning application consultation.

We are providing this advice under Agreement No. ENVPAC/1/THM/00590 and Firethorn Developments will be invoiced accordingly.

We have reviewed the proposed modelling methodology you submitted by email dated 22/07/2021.

Please note that we are only providing you with our advice on the matters as outlined in our offer as requested.

Hydrology

The proposed hydrological analysis will be performed using ReFH2 and statistical techniques for all key design storms. This is an appropriate approach. The return periods of 5%, 1%, 1% CC and 0.1% AEP are appropriate, although further return periods such as 10%, 2% and 0.5%AEP should be considered. The climate change uplift is noted as 35% for the Thames higher central estimate. A further climate change simulation with an upper end estimate is suggested. Any climate change uplifts should follow appropriate climate change guidance.

A sensitivity test should be completed with flow + /- 20% for the 1% AEP return period.

The location for the hydrological analysis is appropriate.

Hydraulics

Did you know the Environment Agency has a **Planning Advice Service**? We can help you with all your planning questions, including overcoming our objections. If you would like our help please email us at planning_THM@environment-agency.gov.uk

A 1D-2D model is appropriate for this study, with the 1D-2D links using HX boundaries. The 2D model extent matches the proposed 1D so the whole model will be 1D in-channel and 2D floodplain. A 2m cell size using 1m LiDAR is appropriate and should give good resolution of the model without heavily compromising run times.

The survey coverage is appropriate and appears to contain all future intended structures across the river channels. Any further alterations to the site plans around the channel should be incorporated into the model.

The 2D Manning's roughness is suggested as 0.05 for agricultural fields and aerial photography will be used to define other roughness values. Whilst this is appropriate it is suggested that mastermap data is used to define the roughness values on the floodplain, with updates to the roughness used for the design/with scheme simulation.

The downstream boundary location appears to be appropriate. Vectos describes the initial backwater calculation showing an appropriate distance downstream of the site. Results at this location should be investigated following to ensure that there is no flow or stage increase in the design/with scheme simulations. Any increase will require further investigation and possibly the requirement to map the knock-on effects downstream of the proposed model.

Sensitivity tests have not been included within the methodology proposal. These should be undertaken with + /-20% sensitivities to the inflows, manning's n and downstream boundary.

The modelling will be required to follow Environment Agency guidance which you can access through the following link:

<https://www.gov.uk/government/publications/river-modelling-technical-standards-and-assessment>

Conclusions

The modelling methodology is reasonable for the study. As mentioned above further considerations should be taken for inflows and sensitivity tests and particular attention should be given to the downstream boundary, although the methodology supplied suggests initial tests suggest it is appropriate.

Next steps

Any modelling compiled in support of the development at North West Bicester will need to undergo a technical review by the Environment Agency. Please note that a review can take anywhere between 3 – 8 weeks depending on the nature of the model and programme of reviews already underway.

Final comments

Once again, thank you for contacting us with your enquiry. Our comments are based on our available records and the information as submitted to us.

I hope the above advice is helpful. If there is any further work you anticipate needing our detailed advice on in relation to this project, please let me know so it can be incorporated into this charging agreement.

Disclaimer

Please note that the views expressed in this report by the Environment Agency, is a response to a pre-application enquiry only and **does not represent our final view in**

relation to any future statutory consultations made in relation to this site. We reserve the right to change our position in relation to any such application.

We have only provided advice in relation to the environmental constraints as outlined in our offer as requested. However, we will comment on all environmental constraints within our remit in our statutory response.

Please quote our reference number in any future correspondence. If you have any queries please feel free to contact me.

Yours sincerely

Miss Sarah Green
Sustainable Places - Planning Advisor

Direct dial 0208 474 9253

Direct e-mail planning_THM@environment-agency.gov.uk

Appendix 2

Hydrological Analysis

Flood estimation report: Land at Northwest Bicester

Introduction

This report template is a supporting document to the Environment Agency's Flood Estimation Guidelines. It provides a record of the hydrological context, the method statement, the calculations and decisions made during flood estimation and the results.

Contents

1	SUMMARY OF ASSESSMENT	1
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Approvals

Revision stage	Analyst / Reviewer name & qualifications	Amendments	Date
Method statement preparation	GEORGE FRISBY	N/A	02/11/2021
Initial calculations preparation	GEORGE FRISBY	N/A	02/11/2021
Calculations - Revision 1 preparation	GEORGE FRISBY	N/A	02/11/2021

Review by Matthew Scott

Abbreviations

AEP	annual exceedance probability
AM	Annual Maximum
AREA	Catchment area (km ²)
BFI	Base Flow Index
BFIHOST	Base Flow Index derived using the HOST soil classification
CPRE	Council for the Protection of Rural England
FARL	FEH index of flood attenuation due to reservoirs and lakes
FEH	Flood Estimation Handbook
FSR	Flood Studies Report
HOST	Hydrology of Soil Types
NRFA	National River Flow Archive
OS	Ordnance Survey
POT	Peaks Over a Threshold
QMED	Median Annual Flood (with return period 2 years)
ReFH	Revitalised Flood Hydrograph method
ReFH2	Revitalised Flood Hydrograph 2 method
SAAR	Standard Average Annual Rainfall (mm)
SPR	Standard percentage runoff
SPRHOST	Standard percentage runoff derived using the HOST soil classification
Tp(0)	Time to peak of the instantaneous unit hydrograph
URBAN	Flood Studies Report index of fractional urban extent
URBEXT1990	FEH index of fractional urban extent
URBEXT2000	Revised index of urban extent, measured differently from URBEXT1990
WINFAP-FEH	Windows Frequency Analysis Package – used for FEH statistical method

1 SUMMARY OF ASSESSMENT

1.1 Summary

This table provides a summary of the key information contained within the detailed assessment in the following sections. The aim of the table is to enable quick and easy identification of the type of assessment undertaken. This should assist in identifying an appropriate reviewer and the ability to compare different studies more easily.

Catchment location	North Bicester - 457650,224300
Purpose of study and scope	Inflows for hydraulic model – A lumped catchment (DS_R01) and two sub catchments (US_R01 and US_R02).
Key catchment features	Permeable Catchment – Rural
Flooding mechanisms	Fluvial and potential Groundwater
Gauged / ungauged	Ungauged
Final choice of method	FEH
Key limitations / uncertainties in results	Interaction with Groundwater during long duration storm events.

1.2 Note on flood frequencies

The frequency of a flood can be quoted in terms of a return period, which is defined as the average time between years with at least one larger flood, or as an annual exceedance probability (AEP), which is the inverse of the return period.

Return periods are output by the Flood Estimation Handbook (FEH) software and can be expressed more succinctly than AEP. However, AEP can be helpful when presenting results to members of the public who may associate the concept of return period with a regular occurrence rather than an average recurrence interval. Results tables in this document contain both return period and AEP titles; both rows can be retained or the relevant row can be retained and the other removed, depending on the requirement of the study.

The table below is provided to enable quick conversion between return periods and annual exceedance probabilities.

Annual exceedance probability (AEP) and related return period reference table

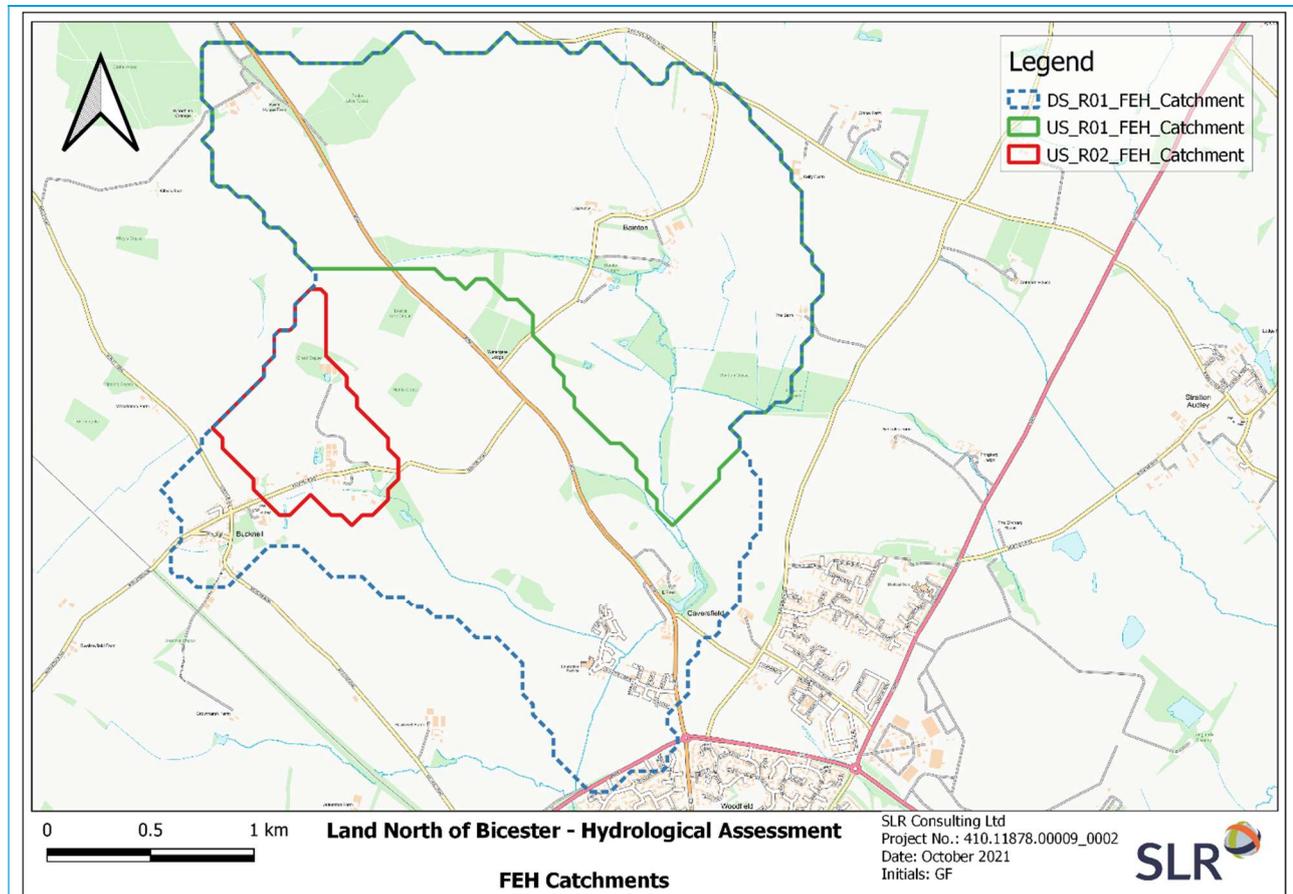
AEP (%)	50	20	10	5	3.33	2	1.33	1	0.5	0.1
AEP	0.5	0.2	0.1	0.05	0.033	0.02	0.0133	0.01	0.005	0.001
Return period (yrs)	2	5	10	20	30	50	75	100	200	1,000

2 METHOD STATEMENT

2.1 Requirements for flood estimates

Overview	<p>Estimate peak flows and generate associated hydrographs for use as upstream boundary conditions/inflows to a hydraulic model covering Bainton, Swallowfield Farm and Caversfield.</p> <p>1 lumped catchment and 2 sub catchments.</p> <p>0.05, 0.01 and 0.001 AEP flood events Allowance for climate change (CC) of 15 and 25% (allowance for relevant EA catchment) applied to the 0.01 AEP hydrographs in the hydraulic model.</p>
Project scope	No existing hydrological studies incorporated into this assessment.

2.2 The catchment



Description	<p>The total area is divided into one lumped catchment and 2 sub catchments as shown in the figure above. The two main catchment watercourses converge at 457882, 224889 before flowing south west – under the A4095 Bicester bypass, which forms the lumped catchment (DS_R01) outlet.</p> <p>The Geology of the catchment is predominantly Cornbrash Formation Limestone, with in channel bedrock of Forest Marble Limestone and Mudstone and superficial deposits of alluvium (clay, silt, sand and gravel). The porous limestone in the catchment leads to high levels of permeability.</p> <p>Rural, predominantly agricultural catchment area with low relief, falling from approx. 120m Above Ordnance Datum in the northern upper end of the catchment, to 80m AOD at the Bicester bypass and catchment outlet.</p>
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2.3 Source of flood peak data

Source	NRFA peak flows dataset, Version 10, released August 2020/21. This contains data up to water year 2019/20.
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2.4 Gauging stations (flow or level)

Water-course	Station name	NRFA number	Distance (km)	BFIHOST	Catchment area (km ²)	QMED observed	Pooling Suitability
Bedford	Thornborough Mill	33005	11.34	0.822	387.67	21.8	Yes
Cherwell	Enslow Mill	39021	17.72	0.48	555.45	19.3	Yes
Tove	Cappenham Bridge	33018	22.39	0.59	132.55	17.04	No
Cherwell	Banbury	39026	24.7	0.368	204.59	16.731	No
Evenlode	Cassington Mill)	39034	24.86	0.41	427.14	20.6	Yes

2.5 Hydrological understanding of catchment

Conceptual model	Main site of interest is at Caversfield House, and therefore flood risk is dominated by flows in US_R01. Therefore, the critical duration for this reach will dictate the durations for the other catchments.
Unusual catchment features	FEH catchment descriptors indicate the catchment is highly permeable – with very high BFIHOST of 0.822 and a low SPRHOST of 15.59%.

2.6 Initial choice of approach

Is FEH appropriate?	Yes
Initial choice of method(s) and reasons How will hydrograph shapes be derived if needed? Will the catchment be split into sub-catchments? If so, how?	FEH statistical method – Including permeable adjustment method as the SPRHOST of the catchment is below the threshold of 20%. Hydrographs will be derived using ReFH2. FEH statistical method will be completed on the total lumped catchment (DS_R01). ReFH2 completed on the lumped catchment and the sub catchments. US_R01 and US_R02 sub catchment hydrographs to be scaled proportionately using the FEH statical method and the relative peak flows derived from the ReFH2 hydrographs.
Software to be used (with version numbers)	FEH Web Service ¹ / WINFAP 42.7282 ² / ReFH spreadsheet / ReFH2.2 / Flood Modeller Pro

¹ CEH 2015. The Flood Estimation Handbook (FEH) Online Service, Centre for Ecology & Hydrology, Wallingford, UK.

² WINFAP 4 © Wallingford HydroSolutions Limited 2016.

3 LOCATIONS WHERE FLOOD ESTIMATES REQUIRED

The table below lists the locations of subject sites. The site codes listed below are used in all subsequent tables to save space.

3.1 Summary of subject sites and key catchment descriptors

Site code	Type of estimate L: lumped catchment S: Sub-catchment	Watercourse (all officially unnamed)	Easting	Northing	AREA on FEH Webservice	BFI HOST	SPR HOST (%)	URBEXT 2000	FARL
DS_R01	L	Reach 1	457650	224300	7.60	0.822	15.59	0.0105	0.965
US_R01	S	Reach 1	458100	225550	3.88	0.788	18.08	0.0071	1
US_R02	S	Reach 2	456700	225700	0.53	0.754	20.58	0.0165	1

Note: Lumped catchments (L) are complete catchments draining to points at which design flows are required.

Sub-catchments (S) are catchments or intervening areas that are being used as inputs to a semi-distributed model of the river system. There is no need to report any design flows for sub-catchments, as they are not relevant: the relevant result is the hydrograph that the sub-catchment is expected to contribute to a design flood event at a point further downstream in the river system. This will be recorded within the hydraulic model output files. However, catchment descriptors and ReFH model parameters should be recorded for sub-catchments so that the results can be reproduced.

The schematic diagram illustrates the distinction between lumped and sub-catchment estimates.

3.2 Checking catchment descriptors

Record how catchment boundary was checked and describe any changes	The FEH webservice catchment delineation was checked against OS mapping contours and contoured 2m LiDAR data and was found to be an appropriate estimate for the catchment.
Record how other catchment descriptors were checked and describe any changes.	Checked against OS mapping and BGS superficial/bedrock geology and hydrogeology maps. High permeability values were validated due to the predominant limestone geology of the area.

4 STATISTICAL METHOD

4.1 Application of Statistical method

What is the purpose of applying this method?	Statistical method applied only for the overall lumped catchment (DS_R01). This is the only catchment deemed appropriate for this type of analysis – with the subcatchments being too small (less than 5km ²)
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4.2 Overview of estimation of QMED at each subject site

Site code	QMED (rural) from CDs (m ³ /s)	Final method	Data transfer					Urban adjustment factor UAF	Final estimate of QMED (m ³ /s)
			NRFA numbers for donor sites used (see 4.3)	Distance between centroids d _{ij} (km)	Moderated QMED adjustment factor, (A/B) ^a	If more than one donor			
						Weight	Weighted ave. adjustment		
DS_01	0.290	0.298	1	24.86		1	-	-	0.298
Are the values of QMED spatially consistent?						-			
Method used for urban adjustment for subject and donor sites						WINFAP v4 ³			
Parameters used for WINFAP v4 urban adjustment if applicable									
Impervious fraction for built-up areas, IF			Percentage runoff for impervious surfaces, PR _{imp}			Method for calculating fractional urban cover, URBAN			
0.3			70%			From updated URBEXT2000			
Notes									
Methods: AM – Annual maxima; POT – Peaks over threshold; DT – Data transfer (with urban adjustment); CD – Catchment descriptors alone (with urban adjustment); BCW – Catchment descriptors and bankfull channel width (add details); LF – Low flow statistics (add details).									
The QMED adjustment factor A/B for each donor site is moderated using the power term, a, which is a function of the distance between the centroids of the subject catchment and the donor catchment. The final estimate of QMED is (A/B) ^a times the initial (rural) estimate from catchment descriptors.									
Important note on urban adjustment									
The method used to adjust QMED for urbanisation published in Kjeldsen (2010) Error! Bookmark not defined. in which PRUAF is calculated from BFIHOST is not correctly applied in WINFAP-FEH v3.0.003. Significant differences occur only on urban catchments that are highly permeable. This is discussed in Wallingford HydroSolutions (2016) ³ .									

4.3 Search for donor sites for QMED (if applicable)

Comment on potential donor sites	The 5 closest donor sites are listed in Section 2.4. Gauging station 39034 (Evenlode @ Cassington Mill) has been selected as this is the only catchment with similar geology and to DS_R01 – with majority limestone and permeable bedrock. This is reflected in the high value of BFI HOST. Therefore the relationship between the observed and Catchment Descriptor values of QMED (ratio of 1.07) is incorporated in to the QMED estimation.
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4.4 Derivation of pooling groups and growth curves

The pooling group has been generated using WINFAP v.4. Station 206006 (Analog @ Recorder) has been removed due to old dataset and 72014 (Conder @ Galgate) has been added to being a suitable small catchment. The pooling group can be found in Appendix 01 along with the details of the application of the permeable adjustment method. Only the GL distribution can be applied to the permeable adjustment method.

³ Wallingford HydroSolutions (2016). WINFAP 4 Urban adjustment procedures.

4.5 Derivation of flood growth curves at subject sites – Perm adjusted GL distribution

Site code	Flood peak (m ³ /s) for the following return periods (in years)									
	2	5	10	20	25	50	100	200	500	1000
	Flood peak (m ³ /s) for the following AEP (%) events									
	50	20	10	5	3.33	2	1.33	1	0.5	0.1
DS_R01	1.000	1.445	1.784	2.161	2.294	2.751	3.285	3.915	4.926	5.854

4.6 Flood estimates from the statistical method

Site code	Flood peak (m ³ /s) for the following return periods (in years)									
	2	5	10	20	25	50	100	200	500	1000
	Flood peak (m ³ /s) for the following AEP (%) events									
	50	20	10	5	3.33	2	1.33	1	0.5	0.1
DS_R01	0.30	0.43	0.53	0.64	0.68	0.82	0.98	1.17	1.47	1.74

6 REVITALISED FLOOD HYDROGRAPH 2 (REFH2) METHOD

6.1 Application of ReFH2 method

The ReFH2 method has been applied at all lumped and sub-catchments. The ReFH2 identified critical storm duration for the US-R01 (key reach for the site) catchment has been applied to all other catchments in the study so that uniform storm durations can be applied throughout the hydraulic model.

The ReFH2 full details and parameterisation can be found in Appendix 02.

7 DISCUSSION AND SUMMARY OF RESULTS

7.1 Comparison of results from different methods

The table below shows the comparison in ReFH2 and statistical peak flows for the lumped downstream catchment.

Site code	DS_R01 Flood peak (m ³ /s) for the following return periods (in years)									
	2	5	10	20	25	50	100	200	500	1000
	DS_R01 Flood peak (m ³ /s) for the following AEP (%) events									
	50	20	10	5	3.33	2	1.33	1	0.5	0.1
Statistical Method	0.30	0.43	0.53	0.64	0.68	0.82	0.98	1.17	1.47	1.74
ReFH2				0.84			1.24			2.19

Due to the catchment being permeable, it is deemed more appropriate to use the peak flows from the statistical method analysis. These include allocation for the permeable adjustment method and adjusted QMED from a suitable donor catchment with similar catchment characteristics and geology. These significant elements of the catchment are not considered fully by the ReFH2 calculations.

ReFH2 hydrographs were scaled based on relative catchment sizes.

Appendix 1

WHS Permeable Adjustment Worksheet



Project / Site Name
PeakFlow Dataset Location C:\Users\gfrisby\AppData\Local\Wallingford_HydroSolutions\WINFAP\data\NRFAPeakFlow_v10\suitable-for-pooling

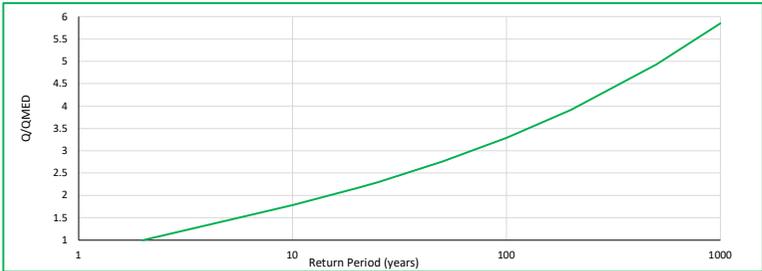
Pooling Group

Station	Distance	Years of data	QMED AM	L-CV	L-SKEW	Station No	L-CVAdj	L-SKEWAdj	k (Shape)	Beta (Scale)	Warnings	
27051 (Crimple @ Burn Bridge)	0.988	48	4.544	0.219	0.146	27051	0.219	0.146	-0.146	0.223		
26016 (Gypsey Race @ Kirby Grindalythe)	1.25	23	0.101	0.312	0.258	26016	0.283	0.313	-0.313	0.279	SPRHOST <=20.	
25019 (Leven @ Easby)	1.313	42	5.384	0.338	0.386	25019	0.338	0.386	-0.386	0.326		
45816 (Haddeo @ Upton)	1.469	27	3.456	0.298	0.417	45816	0.298	0.417	-0.417	0.271		
49005 (Bolingey Stream @ Bolingey Cocks Bridge)	1.557	10	5.972	0.256	0.136	49005	0.256	0.136	-0.136	0.263		
28033 (Dove @ Hollinsclough)	1.653	45	4.15	0.225	0.373	28033	0.225	0.373	-0.373	0.203		
27073 (Brompton Beck @ Snainton Ings)	1.728	40	0.816	0.214	0.02	27073	0.199	0.055	-0.055	0.202		SPRHOST <=20.
27010 (Hodge Beck @ Bransdale Weir)	1.734	41	9.42	0.224	0.293	27010	0.224	0.293	-0.293	0.216		
44008 (South Winterbourne @ Winterbourne Steepleton)	1.8	41	0.448	0.407	0.319	44008	0.399	0.303	-0.303	0.421		SPRHOST <=20. Non-Flood Yrs over 15% of record, consider removing station.
47022 (Tory Brook @ Newnham Park)	1.813	26	5.88	0.257	0.195	47022	0.257	0.195	-0.195	0.262		
36010 (Bumpstead Brook @ Broad Green)	1.88	53	7.5	0.377	0.173	36010	0.377	0.173	-0.173	0.401		
25011 (Langdon Beck @ Langdon)	1.907	34	15.878	0.228	0.316	25011	0.228	0.316	-0.316	0.217	SPRHOST <=20.	
26014 (Water Forlornes @ Driffield)	2.191	22	0.431	0.298	0.12	26014	0.287	0.099	-0.099	0.296		
72014 (Conder @ Galgate)	2.193	52	16.779	0.23	0.149	72014	0.230	0.149	-0.149	0.235		
73015 (Keer @ High Keer Weir)	2.218	29	12.421	0.202	0.238	73015	0.202	0.238	-0.238	0.199		
						-	-	-	-	-		
						-	-	-	-	-		
						-	-	-	-	-		
						-	-	-	-	-		
						-	-	-	-	-		
						-	-	-	-	-		

URBAN	0.02
URBEXT2000	0.0105

SPECIFY TARGET RETURN PERIODS	2	5	10	20	25	50	100	200	500	1000
	1.000	1.445	1.784	2.161	2.294	2.751	3.285	3.915	4.926	5.854

Final Permeable Adjusted Growth Curve



Appendix 2

DS_R01

UK Design Flood Estimation

Generated on 21 September 2021 15:32:41 by gfrisby
Printed from the ReFH2 Flood Modelling software package, version 3.2.7650.24314

Summary of estimate using the Flood Estimation Handbook revitalised flood hydrograph method (ReFH2)

Site details

Checksum: F98A-D453

Site name: FEH_Catchment_Descriptors_457650_224300

Easting: 457650

Northing: 224300

Country: England, Wales or Northern Ireland

Catchment Area (km²): 7.6

Using plot scale calculations: No

Model: 2.3

Site description: None

Model run: 20 year

Summary of results

Rainfall - FEH 2013 model (mm):	50.48	Total runoff (ML):	24.43
Total Rainfall (mm):	33.29	Total flow (ML):	94.83
Peak Rainfall (mm):	5.68	Peak flow (m ³ /s):	0.84

Parameters

Where the user has overridden a system-generated value, this original value is shown in square brackets after the value used.

* Indicates that the user locked the duration/timestep

Rainfall parameters (Rainfall - FEH 2013 model)

Name	Value	User-defined?
Duration (hh:mm:ss)	07:30:00 [09:00:00]	Yes
Timestep (hh:mm:ss)	00:30:00 [01:00:00]	Yes
SCF (Seasonal correction factor)	0.69	No
ARF (Areal reduction factor)	0.96	No
Seasonality	Winter	No

Loss model parameters

Name	Value	User-defined?
Cini (mm)	63.06	No
Cmax (mm)	862.91	No
Use alpha correction factor	No	No
Alpha correction factor	n/a	No

Routing model parameters

Name	Value	User-defined?
Tp (hr)	5.27	No
Up	0.65	No

Uk	0.8	No
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Baseflow model parameters

<u>Name</u>	<u>Value</u>	<u>User-defined?</u>
BF0 (m ³ /s)	0	No
BL (hr)	57.11	No
BR	3.05	No

Urbanisation parameters

<u>Name</u>	<u>Value</u>	<u>User-defined?</u>
Urban area (km ²)	0.13	No
Urbext 2000	0.01	No
Impervious runoff factor	0.7	No
Imperviousness factor	0.4	No
Tp scaling factor	0.75	No
Depression storage depth (mm)	0.5	No
Exporting drained area (km ²)	0.00	Yes
Sewer capacity (m ³ /s)	0.00	Yes

UK Design Flood Estimation

Generated on 21 September 2021 15:32:22 by gfrisby
Printed from the ReFH2 Flood Modelling software package, version 3.2.7650.24314

Summary of estimate using the Flood Estimation Handbook revitalised flood hydrograph method (ReFH2)

Site details

Checksum: F98A-D453

Site name: FEH_Catchment_Descriptors_457650_224300

Easting: 457650

Northing: 224300

Country: England, Wales or Northern Ireland

Catchment Area (km²): 7.6

Using plot scale calculations: No

Model: 2.3

Site description: None

Model run: 100 year

Summary of results

Rainfall - FEH 2013 model (mm):	69.41	Total runoff (ML):	36.09
Total Rainfall (mm):	45.77	Total flow (ML):	140.46
Peak Rainfall (mm):	7.81	Peak flow (m ³ /s):	1.24

Parameters

Where the user has overridden a system-generated value, this original value is shown in square brackets after the value used.

* Indicates that the user locked the duration/timestep

Rainfall parameters (Rainfall - FEH 2013 model)

Name	Value	User-defined?
Duration (hh:mm:ss)	07:30:00 [09:00:00]	Yes
Timestep (hh:mm:ss)	00:30:00 [01:00:00]	Yes
SCF (Seasonal correction factor)	0.69	No
ARF (Areal reduction factor)	0.96	No
Seasonality	Winter	No

Loss model parameters

Name	Value	User-defined?
Cini (mm)	63.06	No
Cmax (mm)	862.91	No
Use alpha correction factor	No	No
Alpha correction factor	n/a	No

Routing model parameters

Name	Value	User-defined?
Tp (hr)	5.27	No
Up	0.65	No

Uk	0.8	No
----	-----	----

Baseflow model parameters

<u>Name</u>	<u>Value</u>	<u>User-defined?</u>
BF0 (m ³ /s)	0	No
BL (hr)	57.11	No
BR	3.05	No

Urbanisation parameters

<u>Name</u>	<u>Value</u>	<u>User-defined?</u>
Urban area (km ²)	0.13	No
Urbext 2000	0.01	No
Impervious runoff factor	0.7	No
Imperviousness factor	0.4	No
Tp scaling factor	0.75	No
Depression storage depth (mm)	0.5	No
Exporting drained area (km ²)	0.00	Yes
Sewer capacity (m ³ /s)	0.00	Yes

UK Design Flood Estimation

Generated on 21 September 2021 15:32:53 by gfrisby
Printed from the ReFH2 Flood Modelling software package, version 3.2.7650.24314

Summary of estimate using the Flood Estimation Handbook revitalised flood hydrograph method (ReFH2)

Site details

Checksum: F98A-D453

Site name: FEH_Catchment_Descriptors_457650_224300

Easting: 457650

Northing: 224300

Country: England, Wales or Northern Ireland

Catchment Area (km²): 7.6

Using plot scale calculations: No

Model: 2.3

Site description: None

Model run: 1000 year

Summary of results

Rainfall - FEH 2013 model (mm):	107.89	Total runoff (ML):	64.01
Total Rainfall (mm):	71.15	Total flow (ML):	250.18
Peak Rainfall (mm):	12.14	Peak flow (m ³ /s):	2.19

Parameters

Where the user has overridden a system-generated value, this original value is shown in square brackets after the value used.

* Indicates that the user locked the duration/timestep

Rainfall parameters (Rainfall - FEH 2013 model)

Name	Value	User-defined?
Duration (hh:mm:ss)	07:30:00 [09:00:00]	Yes
Timestep (hh:mm:ss)	00:30:00 [01:00:00]	Yes
SCF (Seasonal correction factor)	0.69	No
ARF (Areal reduction factor)	0.96	No
Seasonality	Winter	No

Loss model parameters

Name	Value	User-defined?
Cini (mm)	63.06	No
Cmax (mm)	862.91	No
Use alpha correction factor	No	No
Alpha correction factor	n/a	No

Routing model parameters

Name	Value	User-defined?
Tp (hr)	5.27	No
Up	0.65	No

Uk	0.8	No
----	-----	----

Baseflow model parameters

<u>Name</u>	<u>Value</u>	<u>User-defined?</u>
BF0 (m ³ /s)	0	No
BL (hr)	57.11	No
BR	3.05	No

Urbanisation parameters

<u>Name</u>	<u>Value</u>	<u>User-defined?</u>
Urban area (km ²)	0.13	No
Urbext 2000	0.01	No
Impervious runoff factor	0.7	No
Imperviousness factor	0.4	No
Tp scaling factor	0.75	No
Depression storage depth (mm)	0.5	No
Exporting drained area (km ²)	0.00	Yes
Sewer capacity (m ³ /s)	0.00	Yes

US_R01

UK Design Flood Estimation

Generated on 21 September 2021 15:10:56 by gfrisby
Printed from the ReFH2 Flood Modelling software package, version 3.2.7650.24314

Summary of estimate using the Flood Estimation Handbook revitalised flood hydrograph method (ReFH2)

Site details

Checksum: DC9A-D895

Site name: FEH_Catchment_Descriptors_458100_225550

Easting: 458100

Northing: 225550

Country: England, Wales or Northern Ireland

Catchment Area (km²): 3.88

Using plot scale calculations: No

Model: 2.3

Site description: None

Model run: 20 year

Summary of results

Rainfall - FEH 2013 model (mm):	50.51	Total runoff (ML):	13.55
Total Rainfall (mm):	33.66	Total flow (ML):	52.31
Peak Rainfall (mm):	5.74	Peak flow (m ³ /s):	0.53

Parameters

Where the user has overridden a system-generated value, this original value is shown in square brackets after the value used.

* Indicates that the user locked the duration/timestep

Rainfall parameters (Rainfall - FEH 2013 model)

Name	Value	User-defined?
Duration (hh:mm:ss)	07:30:00	No
Timestep (hh:mm:ss)	00:30:00	No
SCF (Seasonal correction factor)	0.69	No
ARF (Areal reduction factor)	0.97	No
Seasonality	Winter	No

Loss model parameters

Name	Value	User-defined?
Cini (mm)	65.35	No
Cmax (mm)	812.87	No
Use alpha correction factor	No	No
Alpha correction factor	n/a	No

Routing model parameters

Name	Value	User-defined?
Tp (hr)	4.38	No
Up	0.65	No

Uk	0.8	No
----	-----	----

Baseflow model parameters

<u>Name</u>	<u>Value</u>	<u>User-defined?</u>
BF0 (m ³ /s)	0	No
BL (hr)	52.44	No
BR	2.95	No

Urbanisation parameters

<u>Name</u>	<u>Value</u>	<u>User-defined?</u>
Urban area (km ²)	0.04	No
Urbext 2000	0.01	No
Impervious runoff factor	0.7	No
Imperviousness factor	0.4	No
Tp scaling factor	0.75	No
Depression storage depth (mm)	0.5	No
Exporting drained area (km ²)	0.00	Yes
Sewer capacity (m ³ /s)	0.00	Yes

UK Design Flood Estimation

Generated on 21 September 2021 15:11:16 by gfrisby
Printed from the ReFH2 Flood Modelling software package, version 3.2.7650.24314

Summary of estimate using the Flood Estimation Handbook revitalised flood hydrograph method (ReFH2)

Site details

Checksum: DC9A-D895

Site name: FEH_Catchment_Descriptors_458100_225550

Easting: 458100

Northing: 225550

Country: England, Wales or Northern Ireland

Catchment Area (km²): 3.88

Using plot scale calculations: No

Model: 2.3

Site description: None

Model run: 100 year

Summary of results

Rainfall - FEH 2013 model (mm):	69.49	Total runoff (ML):	20.04
Total Rainfall (mm):	46.31	Total flow (ML):	77.46
Peak Rainfall (mm):	7.90	Peak flow (m ³ /s):	0.79

Parameters

Where the user has overridden a system-generated value, this original value is shown in square brackets after the value used.

* Indicates that the user locked the duration/timestep

Rainfall parameters (Rainfall - FEH 2013 model)

Name	Value	User-defined?
Duration (hh:mm:ss)	07:30:00	No
Timestep (hh:mm:ss)	00:30:00	No
SCF (Seasonal correction factor)	0.69	No
ARF (Areal reduction factor)	0.97	No
Seasonality	Winter	No

Loss model parameters

Name	Value	User-defined?
Cini (mm)	65.35	No
Cmax (mm)	812.87	No
Use alpha correction factor	No	No
Alpha correction factor	n/a	No

Routing model parameters

Name	Value	User-defined?
Tp (hr)	4.38	No
Up	0.65	No

Uk	0.8	No
----	-----	----

Baseflow model parameters

<u>Name</u>	<u>Value</u>	<u>User-defined?</u>
BF0 (m ³ /s)	0	No
BL (hr)	52.44	No
BR	2.95	No

Urbanisation parameters

<u>Name</u>	<u>Value</u>	<u>User-defined?</u>
Urban area (km ²)	0.04	No
Urbext 2000	0.01	No
Impervious runoff factor	0.7	No
Imperviousness factor	0.4	No
Tp scaling factor	0.75	No
Depression storage depth (mm)	0.5	No
Exporting drained area (km ²)	0.00	Yes
Sewer capacity (m ³ /s)	0.00	Yes

UK Design Flood Estimation

Generated on 21 September 2021 15:11:32 by gfrisby
Printed from the ReFH2 Flood Modelling software package, version 3.2.7650.24314

Summary of estimate using the Flood Estimation Handbook revitalised flood hydrograph method (ReFH2)

Site details

Checksum: DC9A-D895

Site name: FEH_Catchment_Descriptors_458100_225550

Easting: 458100

Northing: 225550

Country: England, Wales or Northern Ireland

Catchment Area (km²): 3.88

Using plot scale calculations: No

Model: 2.3

Site description: None

Model run: 1000 year

Summary of results

Rainfall - FEH 2013 model (mm):	107.99	Total runoff (ML):	35.53
Total Rainfall (mm):	71.97	Total flow (ML):	137.16
Peak Rainfall (mm):	12.28	Peak flow (m ³ /s):	1.40

Parameters

Where the user has overridden a system-generated value, this original value is shown in square brackets after the value used.

* Indicates that the user locked the duration/timestep

Rainfall parameters (Rainfall - FEH 2013 model)

Name	Value	User-defined?
Duration (hh:mm:ss)	07:30:00	No
Timestep (hh:mm:ss)	00:30:00	No
SCF (Seasonal correction factor)	0.69	No
ARF (Areal reduction factor)	0.97	No
Seasonality	Winter	No

Loss model parameters

Name	Value	User-defined?
Cini (mm)	65.35	No
Cmax (mm)	812.87	No
Use alpha correction factor	No	No
Alpha correction factor	n/a	No

Routing model parameters

Name	Value	User-defined?
Tp (hr)	4.38	No
Up	0.65	No

Uk	0.8	No
----	-----	----

Baseflow model parameters

<u>Name</u>	<u>Value</u>	<u>User-defined?</u>
BF0 (m ³ /s)	0	No
BL (hr)	52.44	No
BR	2.95	No

Urbanisation parameters

<u>Name</u>	<u>Value</u>	<u>User-defined?</u>
Urban area (km ²)	0.04	No
Urbext 2000	0.01	No
Impervious runoff factor	0.7	No
Imperviousness factor	0.4	No
Tp scaling factor	0.75	No
Depression storage depth (mm)	0.5	No
Exporting drained area (km ²)	0.00	Yes
Sewer capacity (m ³ /s)	0.00	Yes

US_R02

UK Design Flood Estimation

Generated on 21 September 2021 15:39:44 by gfrisby
Printed from the ReFH2 Flood Modelling software package, version 3.2.7650.24314

Summary of estimate using the Flood Estimation Handbook revitalised flood hydrograph method (ReFH2)

Site details

Checksum: 1AF1-C1B9

Site name: FEH_Catchment_Descriptors_456700_225700

Easting: 456700

Northing: 225700

Country: England, Wales or Northern Ireland

Catchment Area (km²): 0.53

Using plot scale calculations: No

Model: 2.3

Site description: None

Model run: 20 year

Summary of results

Rainfall - FEH 2013 model (mm):	50.03	Total runoff (ML):	2.01
Total Rainfall (mm):	33.78	Total flow (ML):	7.25
Peak Rainfall (mm):	5.76	Peak flow (m ³ /s):	0.12

Parameters

Where the user has overridden a system-generated value, this original value is shown in square brackets after the value used.

* Indicates that the user locked the duration/timestep

Rainfall parameters (Rainfall - FEH 2013 model)

Name	Value	User-defined?
Duration (hh:mm:ss)	07:30:00 [03:30:00]	Yes
Timestep (hh:mm:ss)	00:30:00	No
SCF (Seasonal correction factor)	0.69	No
ARF (Areal reduction factor)	0.98	No
Seasonality	Winter	No

Loss model parameters

Name	Value	User-defined?
Cini (mm)	66.48	No
Cmax (mm)	789.97	No
Use alpha correction factor	No	No
Alpha correction factor	n/a	No

Routing model parameters

Name	Value	User-defined?
Tp (hr)	2.13	No
Up	0.65	No

Uk	0.8	No
----	-----	----

Baseflow model parameters

<u>Name</u>	<u>Value</u>	<u>User-defined?</u>
BF0 (m ³ /s)	0	No
BL (hr)	39.75	No
BR	2.9	No

Urbanisation parameters

<u>Name</u>	<u>Value</u>	<u>User-defined?</u>
Urban area (km ²)	0.01	No
Urbext 2000	0.02	No
Impervious runoff factor	0.7	No
Imperviousness factor	0.4	No
Tp scaling factor	0.75	No
Depression storage depth (mm)	0.5	No
Exporting drained area (km ²)	0.00	Yes
Sewer capacity (m ³ /s)	0.00	Yes

UK Design Flood Estimation

Generated on 21 September 2021 15:39:58 by gfrisby
Printed from the ReFH2 Flood Modelling software package, version 3.2.7650.24314

Summary of estimate using the Flood Estimation Handbook revitalised flood hydrograph method (ReFH2)

Site details

Checksum: 1AF1-C1B9

Site name: FEH_Catchment_Descriptors_456700_225700

Easting: 456700

Northing: 225700

Country: England, Wales or Northern Ireland

Catchment Area (km²): 0.53

Using plot scale calculations: No

Model: 2.3

Site description: None

Model run: 100 year

Summary of results

Rainfall - FEH 2013 model (mm):	68.96	Total runoff (ML):	2.96
Total Rainfall (mm):	46.56	Total flow (ML):	10.69
Peak Rainfall (mm):	7.94	Peak flow (m ³ /s):	0.18

Parameters

Where the user has overridden a system-generated value, this original value is shown in square brackets after the value used.

* Indicates that the user locked the duration/timestep

Rainfall parameters (Rainfall - FEH 2013 model)

Name	Value	User-defined?
Duration (hh:mm:ss)	07:30:00 [03:30:00]	Yes
Timestep (hh:mm:ss)	00:30:00	No
SCF (Seasonal correction factor)	0.69	No
ARF (Areal reduction factor)	0.98	No
Seasonality	Winter	No

Loss model parameters

Name	Value	User-defined?
Cini (mm)	66.48	No
Cmax (mm)	789.97	No
Use alpha correction factor	No	No
Alpha correction factor	n/a	No

Routing model parameters

Name	Value	User-defined?
Tp (hr)	2.13	No
Up	0.65	No

Uk	0.8	No
----	-----	----

Baseflow model parameters

<u>Name</u>	<u>Value</u>	<u>User-defined?</u>
BF0 (m ³ /s)	0	No
BL (hr)	39.75	No
BR	2.9	No

Urbanisation parameters

<u>Name</u>	<u>Value</u>	<u>User-defined?</u>
Urban area (km ²)	0.01	No
Urbext 2000	0.02	No
Impervious runoff factor	0.7	No
Imperviousness factor	0.4	No
Tp scaling factor	0.75	No
Depression storage depth (mm)	0.5	No
Exporting drained area (km ²)	0.00	Yes
Sewer capacity (m ³ /s)	0.00	Yes

UK Design Flood Estimation

Generated on 21 September 2021 15:40:10 by gfrisby
Printed from the ReFH2 Flood Modelling software package, version 3.2.7650.24314

Summary of estimate using the Flood Estimation Handbook revitalised flood hydrograph method (ReFH2)

Site details

Checksum: 1AF1-C1B9

Site name: FEH_Catchment_Descriptors_456700_225700

Easting: 456700

Northing: 225700

Country: England, Wales or Northern Ireland

Catchment Area (km²): 0.53

Using plot scale calculations: No

Model: 2.3

Site description: None

Model run: 1000 year

Summary of results

Rainfall - FEH 2013 model (mm):	107.06	Total runoff (ML):	5.22
Total Rainfall (mm):	72.29	Total flow (ML):	18.86
Peak Rainfall (mm):	12.33	Peak flow (m ³ /s):	0.32

Parameters

Where the user has overridden a system-generated value, this original value is shown in square brackets after the value used.

* Indicates that the user locked the duration/timestep

Rainfall parameters (Rainfall - FEH 2013 model)

Name	Value	User-defined?
Duration (hh:mm:ss)	07:30:00 [03:30:00]	Yes
Timestep (hh:mm:ss)	00:30:00	No
SCF (Seasonal correction factor)	0.69	No
ARF (Areal reduction factor)	0.98	No
Seasonality	Winter	No

Loss model parameters

Name	Value	User-defined?
Cini (mm)	66.48	No
Cmax (mm)	789.97	No
Use alpha correction factor	No	No
Alpha correction factor	n/a	No

Routing model parameters

Name	Value	User-defined?
Tp (hr)	2.13	No
Up	0.65	No

Uk	0.8	No
----	-----	----

Baseflow model parameters

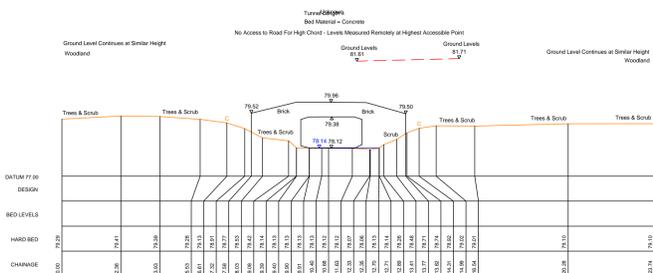
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BF0 (m ³ /s)	0	No
BL (hr)	39.75	No
BR	2.9	No

Urbanisation parameters

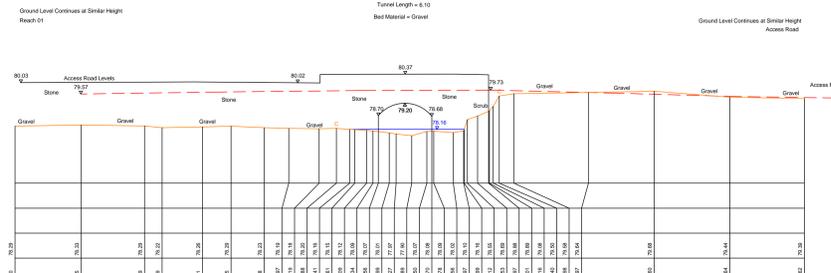
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Urban area (km ²)	0.01	No
Urbext 2000	0.02	No
Impervious runoff factor	0.7	No
Imperviousness factor	0.4	No
Tp scaling factor	0.75	No
Depression storage depth (mm)	0.5	No
Exporting drained area (km ²)	0.00	Yes
Sewer capacity (m ³ /s)	0.00	Yes

Appendix 3

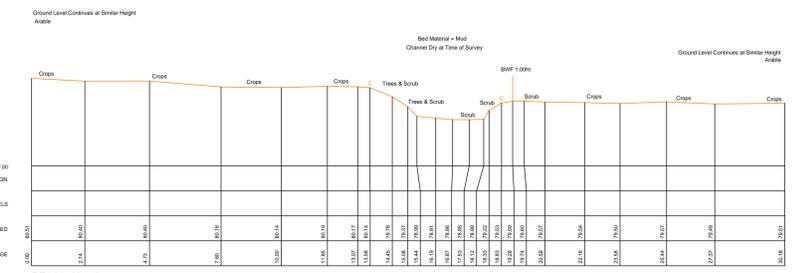
Watercourse Survey



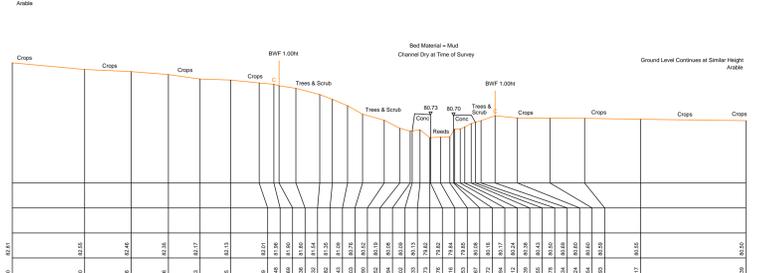
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Culvert Entrance
29/07/2021



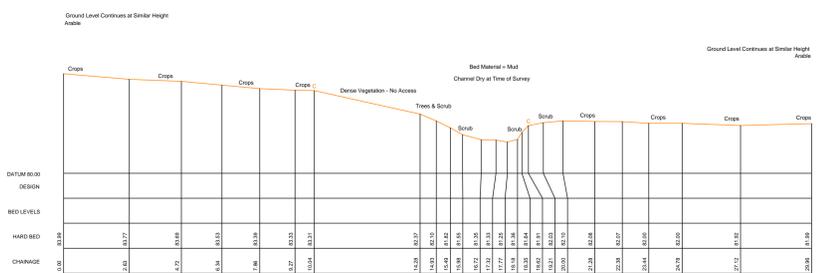
REA01_00015
CHAINAGE 14.661
457617.85E 224221.31N BEARING 244
Access Bridge
29/07/2021



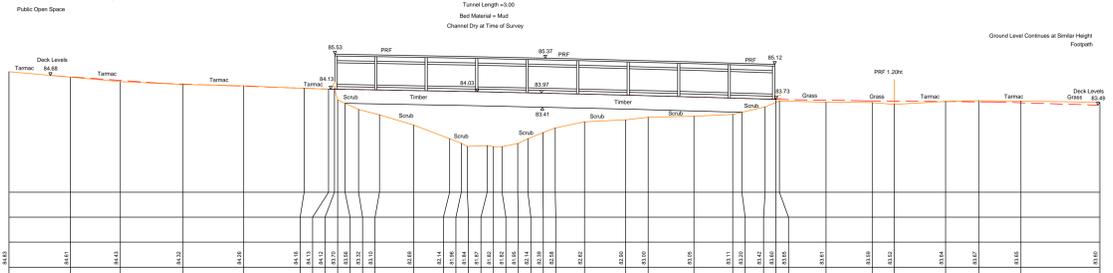
REA01_00115
CHAINAGE 115.179
457648.79E 224262.83N BEARING 286
Open Channel
29/07/2021



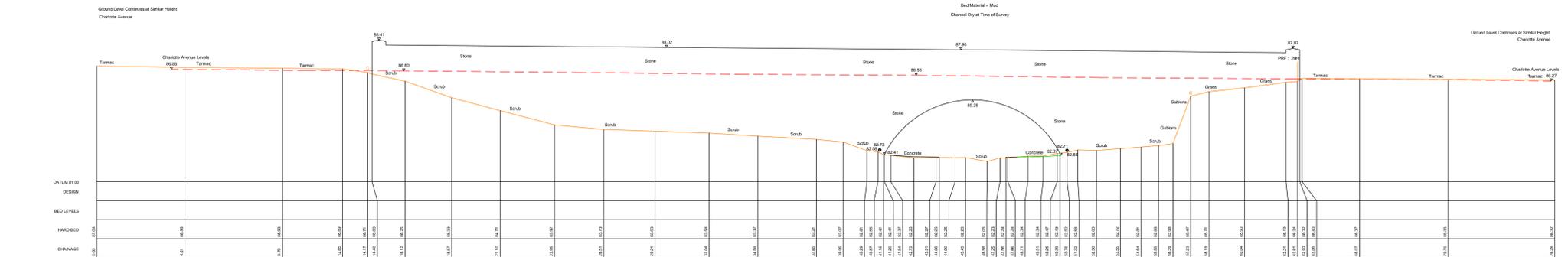
REA01_00318
CHAINAGE 318.268
457639.97E 224491.13N BEARING 276
Other Structure - Dilapidated Footbridge
29/07/2021



REA01_00546
CHAINAGE 546.420
457778.30E 224697.27N BEARING 297
Open Channel
29/07/2021



REA01_00608
CHAINAGE 608.071
457809.61E 224757.14N BEARING 279
Footbridge
29/07/2021



REA01_00660
CHAINAGE 660.291
457851.89E 224791.09N BEARING 292
Charlotte Avenue - Road Bridge
29/07/2021

CROSS SECTION KEY:
WATER LEVEL
BED LEVEL
SILT LEVEL

NOTES:
ALL CROSS SECTIONS SHOWN AS A REPRESENTATIVE SECTION UNLESS OTHERWISE STATED.
ALL CROSS SECTIONS HAVE BEEN CONDUCTED ON A BEST EFFORT BASIS.
ALL DIMENSIONS & CHANGES ARE IN METRES UNLESS OTHERWISE STATED.

SURVEY LEGEND	
AP	AP MARK
B	BENCH MARK
CH	CHANNEL
CL	CULVERT
CP	CROSS POINT
CS	CROSS SECTION
CT	CULVERT TIE
DA	DATA POINT
DB	DATA POINT
DC	DATA POINT
DD	DATA POINT
DE	DATA POINT
DF	DATA POINT
DG	DATA POINT
DH	DATA POINT
DI	DATA POINT
DJ	DATA POINT
DK	DATA POINT
DL	DATA POINT
DM	DATA POINT
DN	DATA POINT
DO	DATA POINT
DP	DATA POINT
DQ	DATA POINT
DR	DATA POINT
DS	DATA POINT
DT	DATA POINT
DU	DATA POINT
DV	DATA POINT
DW	DATA POINT
DX	DATA POINT
DY	DATA POINT
DZ	DATA POINT
EA	EXISTING
EB	EXISTING
EC	EXISTING
ED	EXISTING
EE	EXISTING
EF	EXISTING
EG	EXISTING
EH	EXISTING
EI	EXISTING
EJ	EXISTING
EK	EXISTING
EL	EXISTING
EM	EXISTING
EN	EXISTING
EO	EXISTING
EP	EXISTING
EQ	EXISTING
ER	EXISTING
ES	EXISTING
ET	EXISTING
EU	EXISTING
EV	EXISTING
EW	EXISTING
EX	EXISTING
EY	EXISTING
EZ	EXISTING
FA	PROPOSED
FB	PROPOSED
FC	PROPOSED
FD	PROPOSED
FE	PROPOSED
FF	PROPOSED
FG	PROPOSED
FH	PROPOSED
FI	PROPOSED
FJ	PROPOSED
FK	PROPOSED
FL	PROPOSED
FM	PROPOSED
FN	PROPOSED
FO	PROPOSED
FP	PROPOSED
FQ	PROPOSED
FR	PROPOSED
FS	PROPOSED
FT	PROPOSED
FU	PROPOSED
FV	PROPOSED
FW	PROPOSED
FX	PROPOSED
FY	PROPOSED
FZ	PROPOSED

REV.	ADDENDUM	CHKD.	DATE

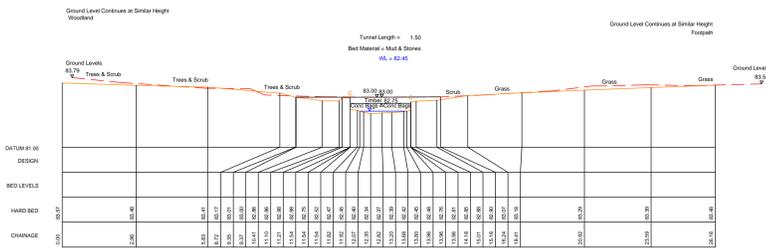
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11: 100% CONTROL
12: 100% CONTROL
13: 100% CONTROL
14: 100% CONTROL
15: 100% CONTROL
16: 100% CONTROL
17: 100% CONTROL
18: 100% CONTROL
19: 100% CONTROL
20: 100% CONTROL

STORM GEOMATICS
Tel: 01608 664910
mail@storm-geomatics.com
www.storm-geomatics.com

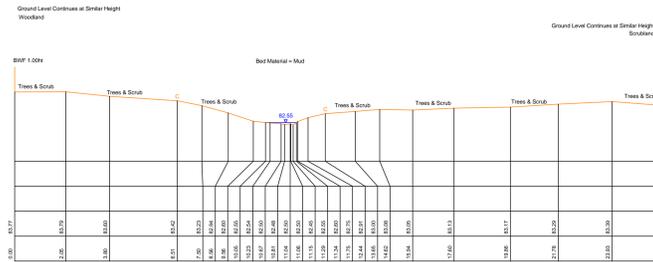
RICS
ICES
Construction

CLIENT: Vectos
PROJECT: NW Bicester Survey
SITE: Reach 01
Charlotte Ave, Bicester OX27 8BL
REA01_00000 to REA01_00660
Sheet 1 of 3

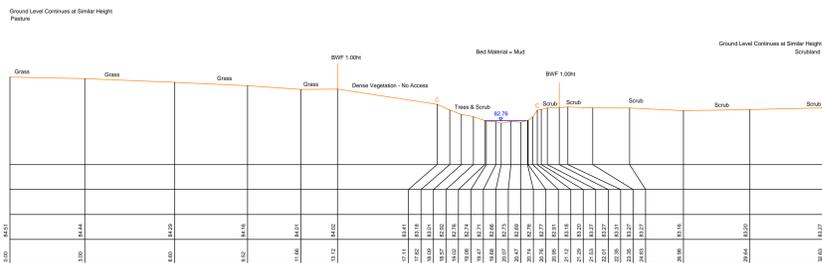
SURVEYED BY: Storm Geomatics Ltd		Project Ref: 21FD1391	
SURVEY DATE: July 2021			
SCALE: 1:100 (A0 Sheet)	DRN: MBM	CHKD: SRD	
DATUM: OS Newlyn (15)	DATE: 06/08/2021	DATE: 12/08/2021	
GRID: OS Grid (15)	DRAWING NO: 21FD1391/01	REV:	



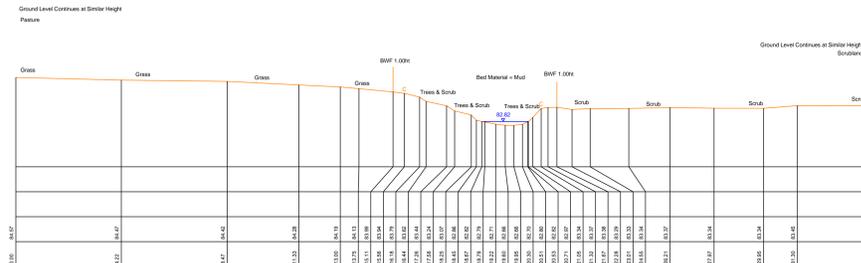
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Footbridge
29/07/2021



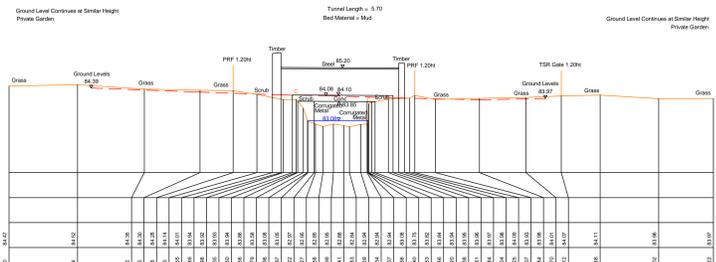
REA01_00777
CHAINAGE_777.027
457882.59E 224886.64N BEARING 306
Open Channel
29/07/2021



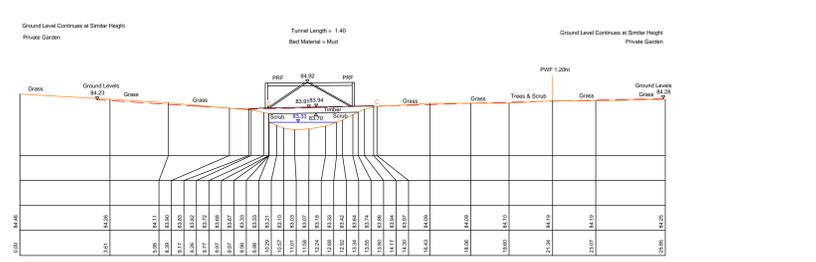
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CHAINAGE_844.252
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Open Channel
29/07/2021



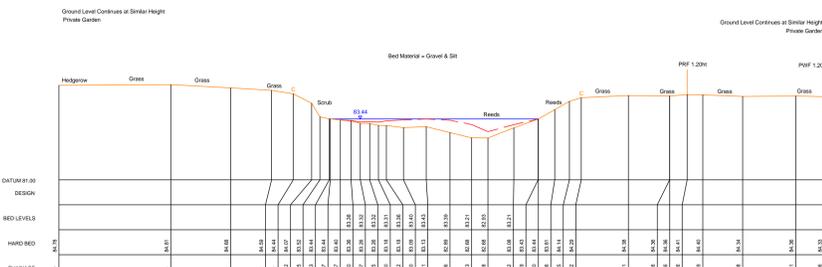
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CHAINAGE_862.305
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Open Channel
29/07/2021



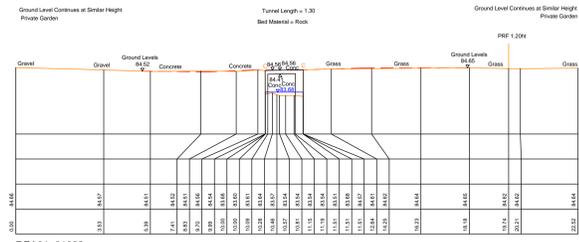
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CHAINAGE_925.003
457981.13E 224988.69N BEARING 316
Access Bridge
29/07/2021



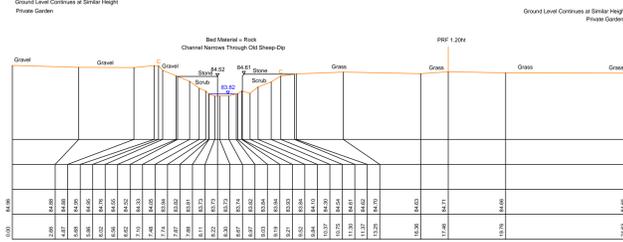
REA01_00946
CHAINAGE_946.246
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Footbridge
29/07/2021



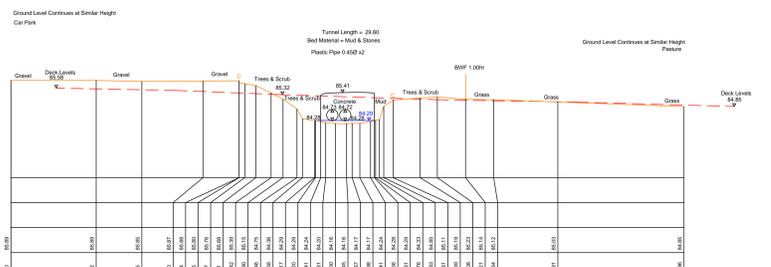
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Open Channel
29/07/2021



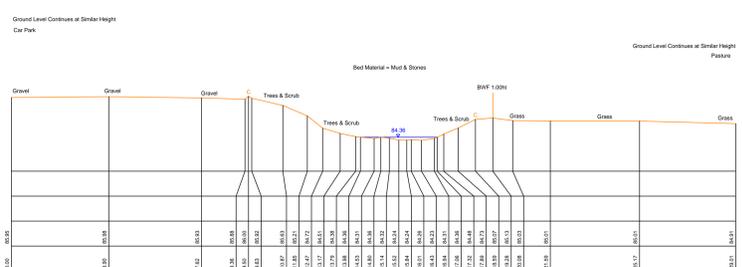
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CHAINAGE_1023.172
458048.29E 225043.24N BEARING 350
Footbridge
29/07/2021



REA01_01028
CHAINAGE_1028.168
458053.48E 225046.05N BEARING 346
Other Structure - Sheep-Dip
29/07/2021



REA01_01068
CHAINAGE_1067.971
458094.52E 225061.17N BEARING 324
Culvert Entrance
29/07/2021



REA01_01073
CHAINAGE_1072.775
458098.86E 225062.89N BEARING 326
Open Channel
29/07/2021

CROSS SECTION KEY:
WATER LEVEL
BED LEVEL
SILT LEVEL

NOTES:

SYMBOL	DESCRIPTION
1	Proposed
2	Existing
3	As Shown
4	Not Shown
5	Not to Scale
6	Not to Scale
7	Not to Scale
8	Not to Scale
9	Not to Scale
10	Not to Scale
11	Not to Scale
12	Not to Scale
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14	Not to Scale
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98	Not to Scale
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CONTROL USED:
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CLIENT: Vectos

PROJECT: NW Bicester Survey

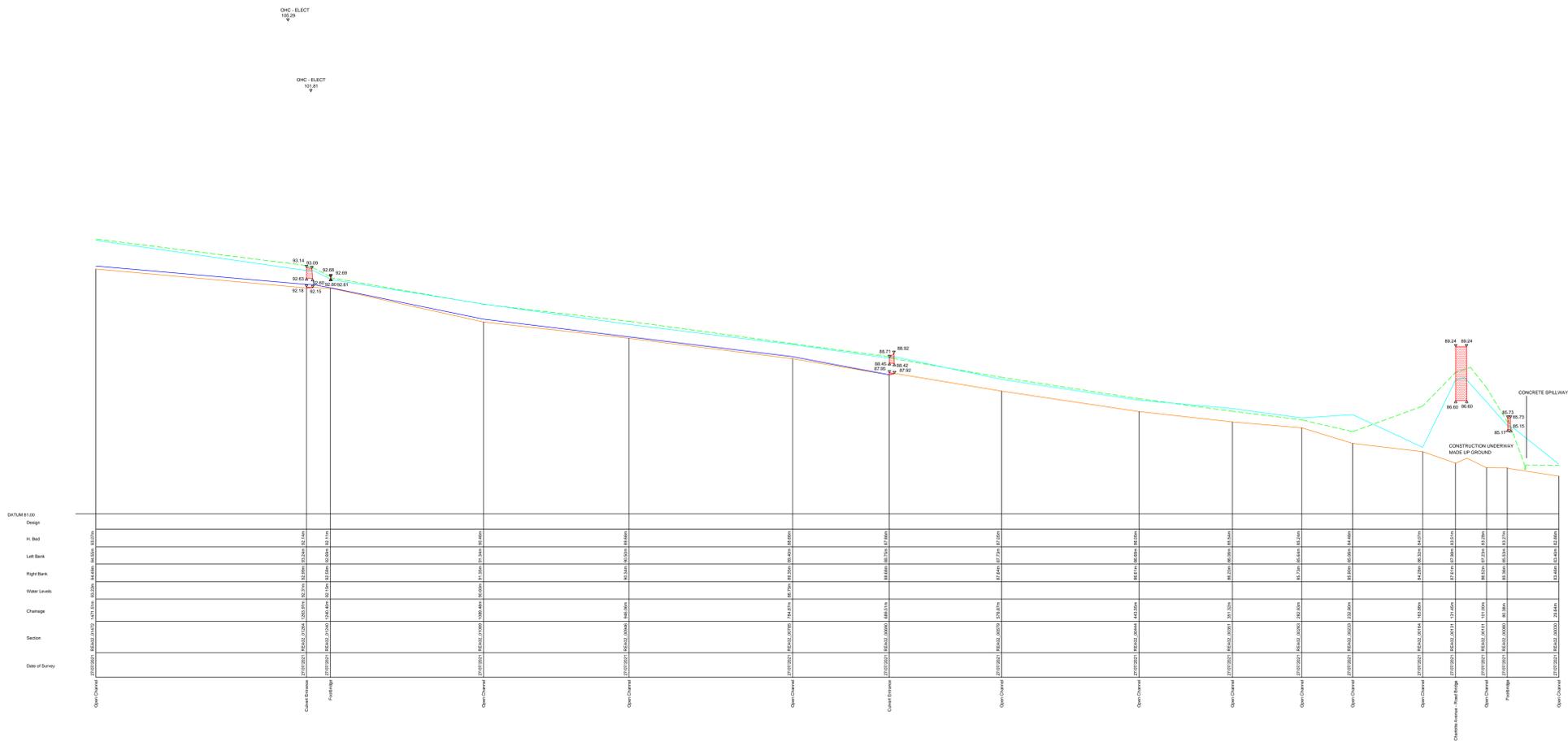
SITE: Reach 02
Charlotte Ave, Bicester OX27 8BL
REA01_00725 to REA01_01073
Sheet 2 of 3

SURVEYED BY: Storm Geomatics Ltd
SURVEY DATE: July 2021
SCALE: 1:100 (A0 Sheet)
DATUM: OS Newlyn (15)
GRID: OS Grid (15)

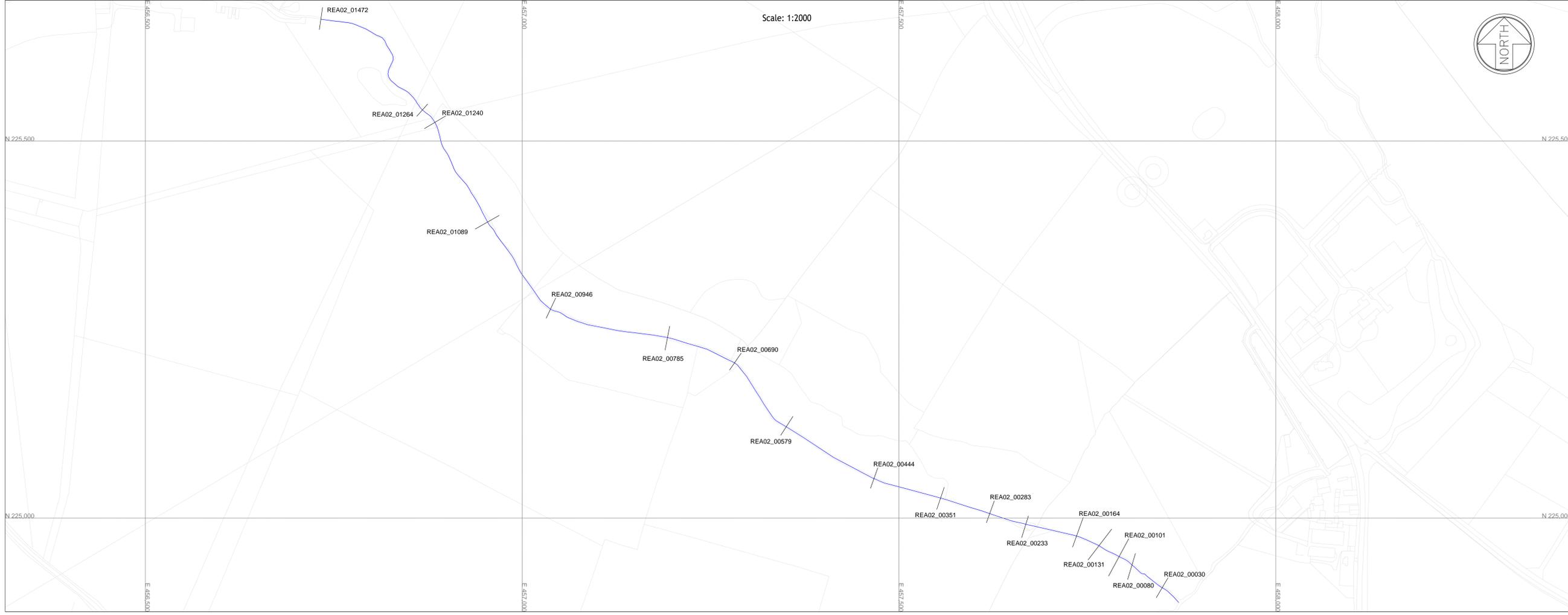
PROJECT NO: 21FD1391/02

DATE: 06/08/2021
DRAWING NO: 21FD1391/02

DATE: 12/08/2021
REV: 1



Scale: 1:2000H 1:100V



LONG SECTION KEY:

WATER LEVEL	—
BED LEVEL	—
SILT LEVEL	—
STRUCTURES	—
RIGHT BANK	—
LEFT BANK	—

NOTES:

ALL LEVELS ARE METERS UNLESS OTHERWISE STATED.
ALL DIMENSIONS ARE METERS UNLESS OTHERWISE STATED.

SYMBOL LEGEND:

AW	AW AREA	AW	AW AREA
B	BANK	B	BANK
BW	BANK WIDTH	BW	BANK WIDTH
CL	CANAL	CL	CANAL
CR	CREEK	CR	CREEK
DR	DRAINAGE	DR	DRAINAGE
ER	ELEVATION	ER	ELEVATION
FR	FENCE	FR	FENCE
GR	GRASS	GR	GRASS
HR	HIGHWAY	HR	HIGHWAY
IR	IRREGULAR	IR	IRREGULAR
LR	LINE	LR	LINE
MR	MOUND	MR	MOUND
NR	NORWAY	NR	NORWAY
OR	OPEN	OR	OPEN
PR	PROPOSED	PR	PROPOSED
QR	QUARRY	QR	QUARRY
RR	RIVER	RR	RIVER
SR	STRUCTURE	SR	STRUCTURE
TR	TERRACE	TR	TERRACE
UR	UNDEVELOPED	UR	UNDEVELOPED
VR	VEGETATION	VR	VEGETATION
WR	WATER	WR	WATER
XR	X-SECTION	XR	X-SECTION
YR	Y-SECTION	YR	Y-SECTION
ZR	Z-SECTION	ZR	Z-SECTION
AR	AREA	AR	AREA
BR	BANK	BR	BANK
CR	CREEK	CR	CREEK
DR	DRAINAGE	DR	DRAINAGE
ER	ELEVATION	ER	ELEVATION
FR	FENCE	FR	FENCE
GR	GRASS	GR	GRASS
HR	HIGHWAY	HR	HIGHWAY
IR	IRREGULAR	IR	IRREGULAR
LR	LINE	LR	LINE
MR	MOUND	MR	MOUND
NR	NORWAY	NR	NORWAY
OR	OPEN	OR	OPEN
PR	PROPOSED	PR	PROPOSED
QR	QUARRY	QR	QUARRY
RR	RIVER	RR	RIVER
SR	STRUCTURE	SR	STRUCTURE
TR	TERRACE	TR	TERRACE
UR	UNDEVELOPED	UR	UNDEVELOPED
VR	VEGETATION	VR	VEGETATION
WR	WATER	WR	WATER
XR	X-SECTION	XR	X-SECTION
YR	Y-SECTION	YR	Y-SECTION
ZR	Z-SECTION	ZR	Z-SECTION

CONTROL USED:

EG-1 PR N21 437834.006 224786.156 84.964

STORM GEOMATICS

RICs

ICCS

Client: Vectos

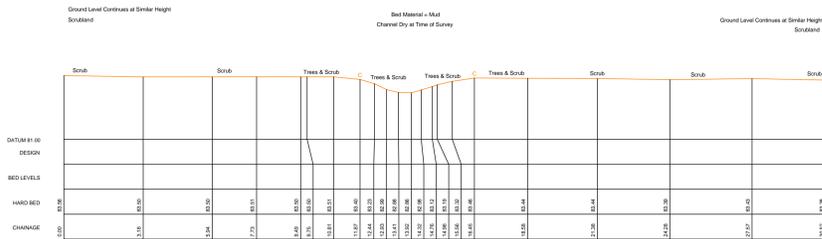
Project: NW Bicester Survey

Reach 02
Charlotte Ave, Bicester OX27 8BL
Long Section & Location Plan
Sheet 1 of 1

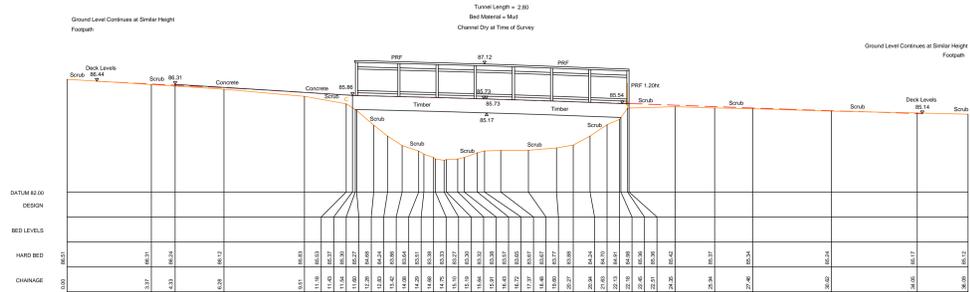
Surveyed by: Storm Geomatics Ltd
Survey Date: July 2021
Scale: As Shown (A0 Sheet)
Datum: OS Newlyn (15)
Grid: OS Grid (15)

DRN: M8M
CHD: SRD
Date: 06/08/2021
Date: 12/08/2021

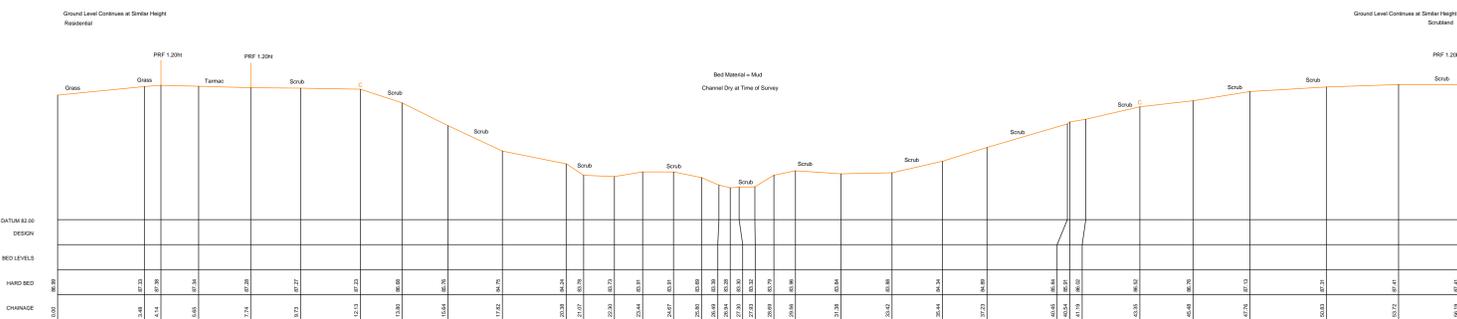
Drawing No: 21FD1391-07



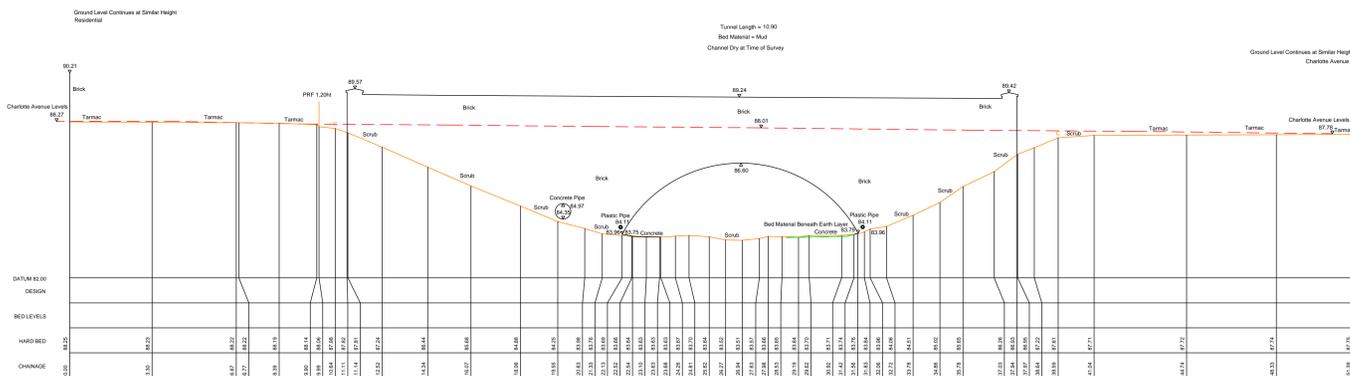
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457866.97E 224920.72N BEARING 211
Open Channel
27/07/2021



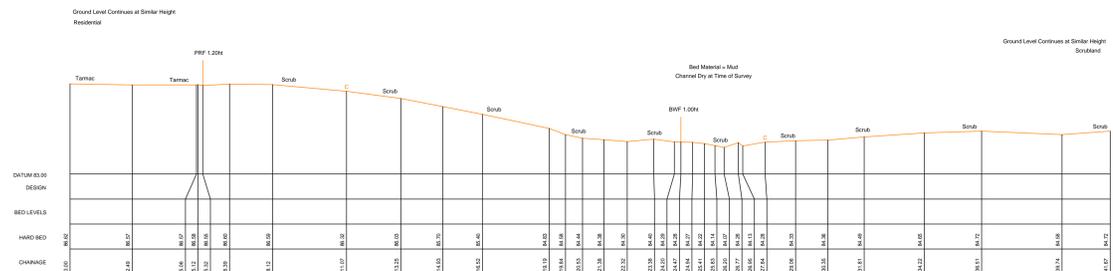
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Footbridge
27/07/2021



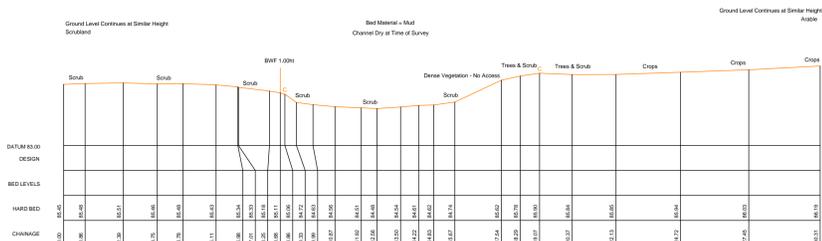
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27/07/2021



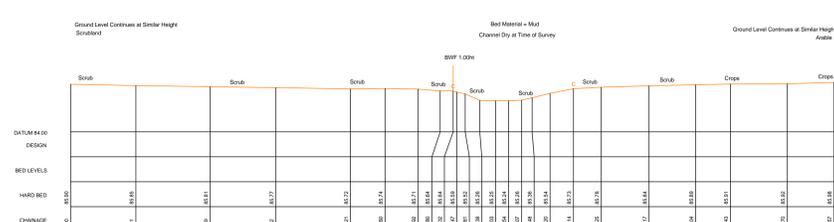
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457782.19E 224985.33N BEARING 218
Charlotte Avenue - Road Bridge
27/07/2021



REA02_00164
CHAINAGE_163.879
457744.39E 225000.78N BEARING 200
Open Channel
27/07/2021



REA02_00233
CHAINAGE_232.895
457671.61E 225002.97N BEARING 197
Open Channel
27/07/2021



REA02_00283
CHAINAGE_282.932
457626.51E 225022.79N BEARING 200
Open Channel
27/07/2021

CROSS SECTION KEY:
WATER LEVEL
BED LEVEL
SILT LEVEL

NOTES:
ALL CROSS SECTIONS SHOWN IN A DIRECTION FROM THE HIGHEST TO THE LOWEST POINTS.
ALL LEVELS ARE MEAN SEA LEVEL UNLESS OTHERWISE STATED.
ALL DIMENSIONS & CHAINAGE ARE IN METRES UNLESS OTHERWISE STATED.

SURVEY LEGEND	
AP	APRIL 2021
BP	BANK PROTECTION
CP	CONCRETE PIPE
DP	DRAINAGE
EP	EXISTING
FP	FOOTING
GP	GRASS
HP	HARD BED
IP	IRON PIPE
JP	JUNCTION
KP	KERB
LP	LANDSCAPE
MP	MUD
NP	NATURAL
OP	OPEN CHANNEL
QP	QUARRY
RP	ROAD
SP	SCRUB
TP	TREES
UP	UNDERPASS
VP	VERTICAL CURVE
WP	WATER LEVEL
XP	EXISTING
YP	YARD
ZP	ZONE

REV.	AMENDMENT	DRN	CHKD	DATE

CONTROL USED:
E6-1 PR N21 43783.606 Northing
E6-1 PR N21 224786.156 Easting



Tel: 01608 664910
mail@storm-geomatics.com
www.storm-geomatics.com

CLIENT: Vectos

PROJECT: NW Bicester Survey

SITE: Reach 02
Charlotte Ave, Bicester OX27 8BL
REA02_00030 to REA02_00283
Sheet 1 of 2

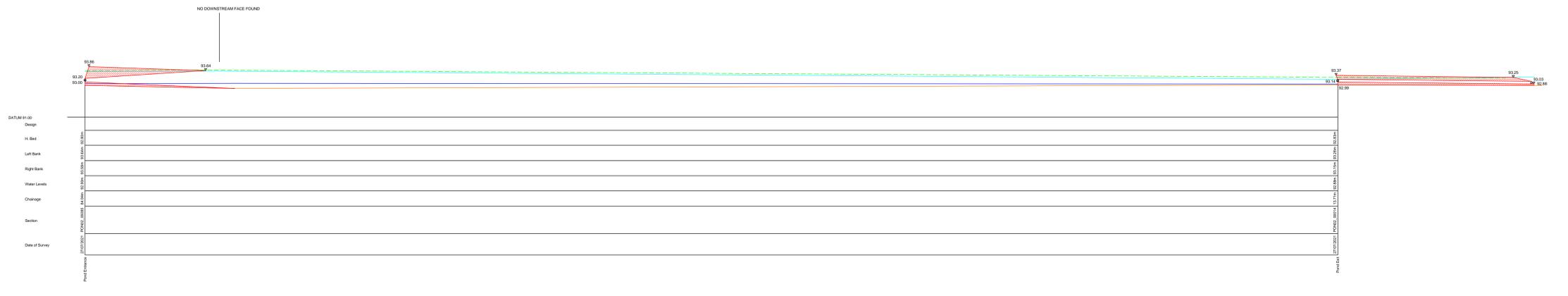
SURVEYED BY: Storm Geomatics Ltd Project Ref: 21FD1391

SCALE: 1:100 (A3 Sheet) DRN: MBM CHKD: SRD

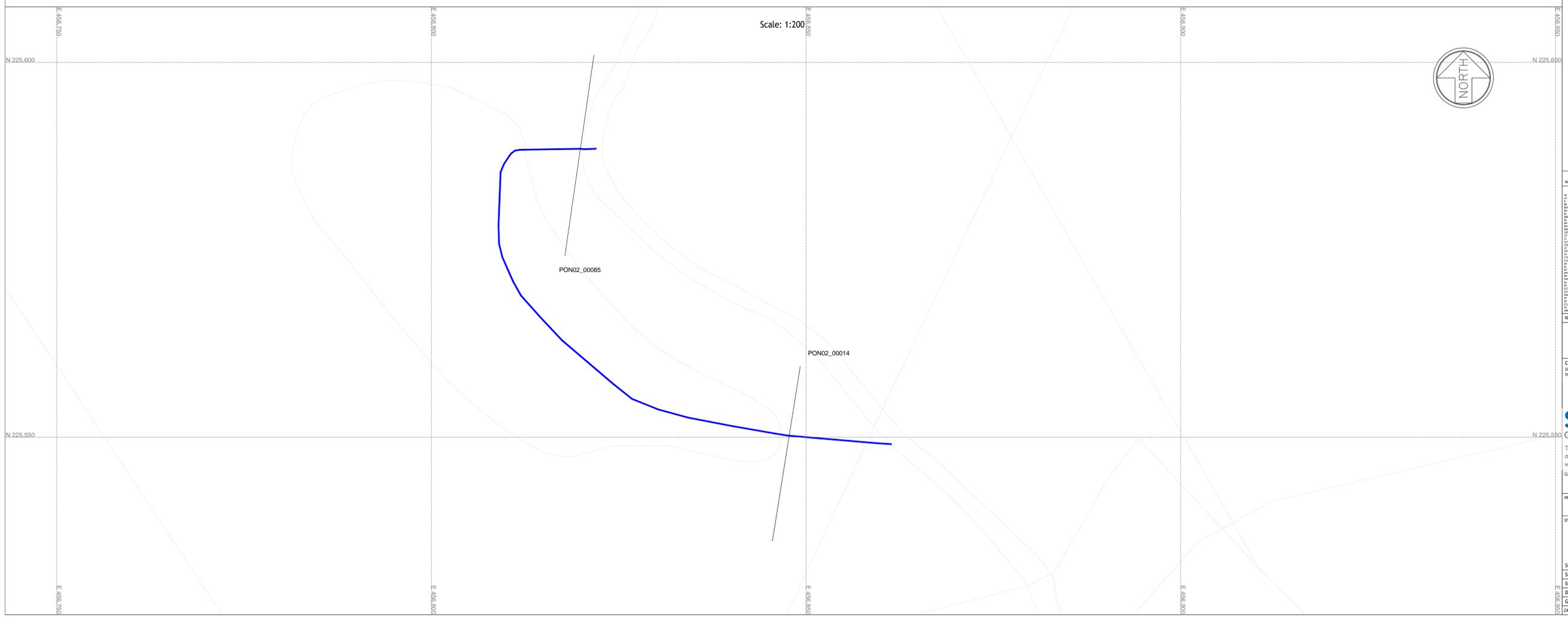
DATUM: OS Newlyn (15) DATE: 06/08/2021 DATE: 12/08/2021

GRID: OS Grid (15) DRAWING NO: 21FD1391/05 REV: -

CAD FILENAME: REA02_VS.dwg



Scale: 1:100H 1:100V



LONG SECTION KEY:

- WATER LEVEL
- BED LEVEL
- SILT LEVEL
- STRUCTURES
- RIGHT BANK
- LEFT BANK

POINTS LOCATED DOWNSTREAM
POINTS LOCATED BY THE
POINTS BETWEEN SECTIONS

NOTES:

ALL LEVELS ARE REFERRED TO DATUM UNLESS OTHERWISE STATED.
ALL DIMENSIONS & DIMENSIONS ARE IN METRES UNLESS OTHERWISE STATED.

SURVEY LEGEND

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B100	BENCH	B100	BENCH	B100	BENCH

REV.	AMENDMENT	DRN	CHKD	DATE

CONTROL USED:

ID	Description	Easting	Northing	Height
E-1	PR N11	437834.606	224768.158	84.964

STORM GEOMATICS

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 www.storm-geomatics.com

RICS **ICCS**

MEMBER OF THE ASSOCIATION OF SURVEYING CONTRACTORS

CLIENT: Vectos

PROJECT: NW Bicester Survey

SITE: Pond 02
 Charlotte Ave, Bicester OX27 8BL
 Long Section & Location Plan
 Sheet 1 of 1

SURVEYED BY: Storm Geomatics Ltd Project Ref: 21FD1391

SURVEY DATE: July 2021

SCALE: As Shown (A0 Sheet) DRN: M8M CHKD: SRD

DATUM: OS Newlyn (15) DATE: 06/08/2021 DATE: 12/08/2021

GRID: OS Grid (15) DRAWING NO.: REV.:

CAD FILENAME: POND2_L54UP.dwg 21FD1391-11

Appendix 4

Flood Mapping











Contact

London

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97 Tottenham Court Road,
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Tel: 020 7580 7373

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Company no. 7591661