2.1.4. Bucknell Road (southern arm), is on a straight alignment and there is a TRIEF kerbed traffic island approximately 40 m from the centre of the junction. There is a continuous hatched marking separator strip; the strip appears to have been highlighted with red surfacing in the past, although this is faded. The hatched area extends through the junction, to provide a narrow, 1 m wide, right turn area for users wishing to turn from Bucknell Road on to Howes Lane. This hatched area does not allow right turning vehicles to clear the through lane, and this led to some, minimal, queuing at the junction in the off-peak site visit period.

2.1.5. Bucknell Road (northern arm), is at the southbound exit from an adjacent small conventional roundabout; the junction of Bucknell Road with the A4095 Lords Lane, and the roundabout exit is approximately 40 m from the centre of the junction with Howes Lane. There is an uncontrolled pedestrian crossing, on Bucknell Road, just north of the Howes Lane junction; this crossing forms a link to the nearby footpath, which links with an adjacent residential development. There are map type direction signs on both the A4095 Lords Lane and Bucknell Road (N) approaches to the roundabout.



### 2.2. Traffic Flow Data

2.2.1. Peak hour traffic flow data has been provided to the assessment team, for both existing (2022) conditions and projected (2028) conditions, with possible development traffic added. This data is shown, in diagrammatic form below.

2.2.2. The traffic flow data indicates that the predominant traffic flows at the junction are:
> The left turn manoeuvre from Howes Lane to Bucknell Road, and
$>\quad$ The right turn manoeuvre from Bucknell Road to Howes Lane.
2.2.3. The traffic flow data also indicates that in the AM peak hour the increase in traffic at the junction will be $15.5 \%$ (from 1744 vehicles in 2022, to 2002 by 2028 with development) and $16.7 \%$ in the pm peak hours (from 1433 in 2022, to 1672 in 2028 with development).
2.2.4. Whilst capacity modelling information has not been provided to the assessment team, it can be seen that the turning traffic proportions would indicate that the current junction priorities do not reflect the predominant traffic movements and queuing at the junction (particularly for the right turn manoeuvre from Bucknell Road) is likely at peak times with increased traffic volumes associated with the proposed development.
2.2.5. No vehicle speed information has been made available to the assessment team, however, the proximity of the Lords Lane roundabout to the Howes Lane junction is likely to result in low approach vehicle speeds.

### 2.3. Road Traffic Collision History

2.3.1. Road traffic collision data has been provided to the assessment team for the five year period 01/01/2016 and 31/12/2021. This data indicates that there have been no reported injury collisions at the Howes Lane junction, nor the roundabout junction with Bucknell Road with the A4095 in that period.
2.3.2. One injury collision occurred on the A4095 Lords Lane, approximately 50 m from the roundabout junction. This collision appears to be related to a medical episode and not related to the highway layout at this location.

### 2.4. Road Safety Related Issues of the Existing Layout

2.4.1. Notwithstanding the absence of reported road traffic collisions, there are a number of potential road safety related issues associated with the existing layout; these are outlined below and are associated with both the existing traffic flow conditions and in future traffic flow scenarios with the proposed development.
2.4.2. On Bucknell Road ( N ), at the uncontrolled pedestrian crossing, inter-visibility between pedestrians crossing from the western footway and drivers turning left from Howes Lane is restricted by the railway bridge wing wall. At the time of the site visit traffic flows were such that it was difficult to assess a safe gap for pedestrians to make the crossing; it is likely that during peak traffic periods assessing safe gaps is likely to be more problematic. Additional traffic volumes associated with the proposed development is likely to exacerbate the issue.
2.4.3. On Bucknell Road $(\mathrm{N})$, the right turn manoeuvre to Howes Lane is the predominant traffic flow at present, this is reflected in the traffic flow data provide above. There is a short stacking space between the right turn area and the exit of the Lords Lane roundabout. It is likely that occasionally queuing vehicles may exceed this stacking space, which may lead to blocking of the roundabout junction. Queuing vehicles within the roundabout junction area may increase the risk of collisions involving unexpected lane change or filtering manoeuvres, particularly involving two-wheeled users. Additional traffic volumes associated with the proposed development is likely to exacerbate the issue.
2.4.4. With the current collision record, the apparent road safety issues have not led to reported road traffic collisions, however increased traffic volumes, and possible increases in pedestrian movements associated with the proposed development may increase the likelihood of the road safety related hazards maturing into reported collisions. The increase in traffic volumes will increase exposure to risk, however there is no clear calculable method of identifying whether the increase in exposure to risk will mature into injury collisions.

## 3. The Proposed Junction

### 3.1. Junction Layout

3.1.1. The proposal to convert the give way controlled tee junction has been triggered by Oxfordshire County Council's decision to redirect the previously agreed funding for the Approved A4095 Strategic Link Road (14/01968/F). As such, the proposed Interim Improvement (i.e. the conversion of the A4095 Howes Lane/Bucknell Road junction to a mini roundabout) is proposed to accommodate all of the development traffic associated with the full Firethorn Development prior to the implementation of the A4095 Strategic Link Road.
3.1.2. The proposed mini roundabout junction layout has been subject to a Stage 1 Road Safety Audit (RSA) (audit reference RSC/KS/EB/21093). This audit raised six road safety related issues, with associated recommendations to mitigate these issues. This report should be read in conjunction with the Stage 1 RSA report and the issues identified within the Stage 1 RSA will not be repeated within this report.

### 3.2. Mini Roundabout Road Safety

3.2.1. TRL research report TRL 281 - Accidents at Urban Mini Roundabouts indicates that three arm mini roundabouts have similar mean collision rates to three arm priority T-junctions and up to $30 \%$ fewer collisions than for signalled junctions. This research (confirmed by DfT Mini Roundabout Good Practice Guidance - 2006) also indicates that the severity of collisions (percentage of fatal and serious collisions to all injury accidents) at three arm mini-roundabout sites is lower than at three arm signalled junctions and considerably lower than at 30 mph T-junctions.
3.2.2. The same research also indicates that at three arm sites $39.9 \%$ of injury collisions involved two wheeled users; the majority of these were of the entering/circulating type. Research from TfL indicates, that in London, $37 \%$ of collisions at priority junctions involved two-wheeled users, compared to 33\% for mini roundabouts - "Levels of Risk in Greater London, issue 13, TfL 2012.

## 4. Discussion and Conclusions

### 4.1. Discussion

4.1.1. According to DfT / County Surveyors document "Mini Roundabout Good Practice Guidance" the introduction of a three arm mini roundabout can improve the operation of a junction by:

## $>\quad$ Reducing the dominance of one traffic flow

As the mini-roundabout works on the principle of 'priority to circulating traffic from the right,' a minor traffic flow can be given priority over a major traffic flow that would otherwise dominate the junction.
> Giving priority to right turners
Again the 'priority' principle of operation has been exploited for right-turning traffic, giving it priority over ahead movements from the opposing direction.
> Facilitating access and reducing delay at side roads
The 'priority to the right' rule effectively halves the traffic to which side road flow has to yield priority, making it easier for side road traffic to turn.
> Improving capacity at overloaded junctions
For a given road space, the mini-roundabout has a higher capacity than most alternatives and is very flexible in coping with variations in both volumes and proportions of traffic flow during the day.
4.1.2. Additionally, the injury collision rates for mini roundabouts are generally similar to urban T-junctions, and show lower severity of injury when compared with urban T-junctions. Mini roundabouts are generally believed to have high proportions of collisions involving two-wheeled users, although this is likely to be layout dependent and figures from TfL show mixed outcomes, and in Greater London the proportions of two-wheeled user involvement for the two junction types is similar.
4.1.3. At the specific location in question, i.e. the junction of A4095 Howes Lane, there have been no recorded injury collisions in the past five years. Whilst no vehicle speed information has been made available to the assessment team, the proximity of the Lords Lane roundabout to the study junction is likely to result in low approach vehicle speeds and this may be contributing to the good collision record history and continue to assist in reducing collision risk with the introduction of a mini roundabout.
4.1.4. From a road safety related point of view, there are potential road safety related issues associated with the proposed mini roundabout layout, as highlighted within the Stage 1 Road Safety Audit, although the design is likely to be amenable to amendment to overcome the issues directly related to the proposed junction conversion.
4.1.5. There are pedestrian safety issues associated with both the existing and proposed layouts, specifically, restricted inter-visibility at the uncontrolled crossing of the northern arm of Bucknell Road. The lack of any injury collisions involving pedestrians at this location at present, may be a result of low pedestrian crossing volumes. The proposed layout is unlikely to improve conditions for pedestrians at the junction, particularly with increased traffic volumes, as well as possible increased pedestrian activity. Any increase in traffic flows will increase the exposure to risk for
vulnerable users, therefore there may be a need to introduce measures to improve the pedestrian crossing environment; the Stage 1 RSA has recommended improvement measures.
4.1.6. At the Howes Lane junction, the predominant turning movement are the left turn from Howes Lane to Bucknell Road northern arm and the reverse right turn movement from Bucknell Road in to Howes Lane. The introduction of a mini roundabout junction would provide a level of priority for the right turn manoeuvre in to Howes Lane and this is likely to be beneficial in reducing the possibility of junction blocking at the adjacent Lords Lane roundabout.

### 4.2. Conclusions

4.2.1. The existing T-junction layout exhibits a good road safety record, with no reported road traffic collisions in the past five year period.
4.2.2. The conversion of the existing junction to a mini roundabout is unlikely to materially adversely affect road safety at the junction, with collision control data indicating similar collision rates between Tjunctions and mini roundabouts, and with the proportion of serious injuries being less with mini roundabouts.
4.2.3. Some research has indicated that mini roundabouts tend to have higher portions of collisions involving two-wheeled users than T-junctions, although control data from TfL shows similar proportions of two-wheeled users involvement with the different junction types.
4.2.4. With the absence of strong evidence to rule out the conversion of the junction to a mini roundabout, there are some benefits in such a conversion, and these are associated with traffic capacity improvements and introducing priority for right turning movements from Bucknell Road, which would assist in capacity improvement and play a part in reducing potential junction blocking at the Lords Lane roundabout, which would in turn reduce the likelihood of collisions associated with such junction blocking.
4.2.5. Overall, the conversion of the existing T-junction would provide positive impacts in terms of traffic capacity, to enable a level of residential development to be implemented. Any adverse effects that may be associated with such a conversion are questionable and appear to be able to be mitigated by a 'best practice' design of the three armed mini roundabout.
4.2.6. One issue that should be carefully considered when converting the junction form would be pedestrian safety and amenity at the junction. This is clearly an issue with the current T-junction layout and improved provision, as recommended with the Stage 1 RSA, would mitigate an existing issue and provide a more 'pedestrian friendly' crossing environment with the proposed converted layout.

## Safety Assessors

Kevin Seymour, B Sc, PG Dip TS, MCIHT, MSoRSA
Signed: .........feqpenen ...................... Date: $16^{\text {th }}$ March 2022
Elaine Bingham, BEng (Hons), MCIHT, MSoRSA

Signed: ............................................. Date: $17^{\text {th }}$ March 2022

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## APPENDIX 1: Existing and Proposed Junction Layouts

## Existing Layout



## Proposed Layout



## ATTACHMENT I

PROPOSED MINI-ROUNDABOUT JUNCTION PARAMETERS


## ATTACHMENT J

PROPOSED MINI-ROUNDABOUT JUNCTION - JUNCTIONS 10 OUTPUT FILES


Filename: 2022.03.14 - NW BICESTER - HOWES LANE (Mini RBt Mitigation).j10
Path: P:|Firethorn Trust_460011100 - NW Bicester|Analysis\ModellinglPicadylBTM 2026 FLOWS Report generation date: 23/03/2022 15:21:26
„BTM Base 2026, AM
„BTM Base 2026, PM
"BTM 2026 + Proposed Development, AM
"BTM 2026 + Proposed Development, PM

## Summary of junction performance

|  | AM |  |  |  |  | Junction <br> Delay (s) | PM |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Set ID | Queue (PCU) | Delay (s) | RFC | Los |  | Set ID | Queue (PCU) | Delay (s) | RFC | Los | Junction Delay (s) |
|  | BTM Base 2026 |  |  |  |  |  |  |  |  |  |  |  |
| AmA | D1 | 4.5 | 33.19 | 0.82 | D | 132.46 | D2 | 1.9 | 12.62 | 0.64 | B | 349.63 |
| Arm B |  | 3.5 | 22.05 | 0.77 | c |  |  | 55.8 | 222.96 | 1.12 | F |  |
| Arm C |  | 68.1 | 248.48 | 1.13 | F |  |  | 153.8 | 607.00 | 1.27 | F |  |
|  | BTM 2026 + Proposed Development |  |  |  |  |  |  |  |  |  |  |  |
| AmA | D3 | 5.0 | 37.25 | 0.84 | E | 309.47 | D4 | 1.9 | 12.20 | 0.63 | в | 527.20 |
| Arm B |  | 4.9 | 29.15 | 0.83 | D |  |  | 105.7 | 472.77 | 1.25 | F |  |
| Arm C |  | 149.5 | 59.54 | 1.27 | F |  |  | 208.4 | 807.01 | 1.34 | F |  |

Values shown are the highest values encountered over all time segments. Delay is the maximum value of average delay per arriving venicle. Junction LOS and Junction Delay re demand-weighted averages.

## File summary

File Description

| Titite | (untilled) |
| :--- | :--- |
| Location |  |
| Site number |  |
| Date | $02 / 11 / 2021$ |
| Version |  |
| Status | (new file) |
| Identifier |  |
| Client |  |
| Jobnumber |  |
| Enumerator | VTPICRicci |
| Description |  |

Units
 m kph PCU cu PCU perthour $\frac{\text { sinage delay }}{\mathrm{s}}$ s -Min perMin

## Analysis Options

| Mini-roundabout model | Calculate Queue Percentiles | Calculate residual capacity | RFC Threshold | Average Delay threshold (s) | Queue threshold (PCU) |
| :--- | :--- | :--- | :--- | :--- | :--- |
| JUNCTIONS 9 |  |  | 0.85 | 36 | 20.00 |

Demand Set Summary
(

| ID | Scenario name | Time Period name | Traffic profile type | Start time (HH:mm) | Finish time (HH:mm) | Time segment length (min) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| D1 | BTM Base 2026 | AM | ONE HOR | $07: 45$ | $09: 15$ | 15 |
| D2 | BTM Base 2026 | PM | ONE HOUR | $16: 45$ | $18: 15$ | 15 |
| D3 | BTM 2026 + Proposed Development | AM | ONE HOUR | $07: 45$ | $09: 15$ | 15 |
| D4 | BTM 2026 + Proposed Development | PM | ONE HOUR | $16: 45$ | $18: 15$ | 15 |

Analysis Set Details
ID Network flow scaling factor (\%)

| A1 | 100.000 |
| :--- | :--- |

## BTM Base 2026, AM

## Data Errors and Warnings

No errors or warrings

## Junction Network

unctions

| Junction | Name | Junction type | Use circulating lanes | Arm order | Junction Delay (s) | Junction Los |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | untitled | Miniriroundabout |  | A, B, C | 132.48 | F |

unction Network

| Driving side | Lighting | Road surface | In London | Network delay (s) | Network LOS |
| :---: | :---: | :---: | :---: | :---: | :---: |

## Arms

Arms

| Afm | Name | Description |
| :---: | :---: | :---: |
| A | untitled |  |
| B | untitled |  |
| C | untitled |  |

Mini Roundabout Geometry

| Am | Approach road half-width $(\mathrm{m})$ | Minimum approach road half-width (m) | $\begin{aligned} & \text { Entry } \\ & \text { width }(m) \end{aligned}$ | $\begin{aligned} & \text { Effectiviv flare } \\ & \text { length } \end{aligned}$ | Distance to next arm (m) | Entry corner kerb line <br> distance $(\mathrm{m})$ | Gradient over $50 \mathrm{~m}(\%)$ | Kerbed central island |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| A | 3.10 | 3.10 | 4.00 | 6.9 | 12.80 | 11.60 | 0.0 |  |
| B | 3.00 | 3.00 | 3.90 | 30.0 | 7.18 | 4.60 | 0.0 |  |
| c | 3.50 | 3.50 | 3.60 | 1.5 | 12.50 | 12.90 | 0.0 |  |

Slope / Intercept / Capacity
oundabout Slope and Intercept used in model

| Am | Final slope | Final intercept (PCU/hr) |
| :---: | :---: | :---: |
| A | 0.622 | 1078 |
| B | 0.621 | 972 |
| C | 0.621 | 904 |

The slope and intercept shown above include any corrections and adiustments.

## Traffic Demanc

Demand Set Details



| Vehicle mix source | PCU Factor for a HV (PCU) |
| :--- | :--- |
| HV Percentages | 200 |



## Origin-Destination Data

Demand (PCU/hr)

|  | To |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| From |  | A | B | c |
|  | A | 0 | 174 | 296 |
|  | B | 13 | 0 | 526 |
|  | c | 180 | 735 | 0 |

## Vehicle Mix

Heavy Vehicle Percentages


## Results



## Main Results for each time segment

| Arm | Total Demand <br> $($ PCU/hr) | Circulating flow <br> (PCU/hr) | Capacity <br> (PCU/hr) | RFC | Throughput <br> $(\mathbf{P C U} / \mathrm{hr})$ | End queue (PCU) | Delay (s) | Unsignalised <br> level of service |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| A | 354 | 543 | 740 | 0.478 | 350 | 1.0 | 10.044 | B |
| B | 406 | 220 | 836 | 0.486 | 402 | 1.0 | 9.047 | A |
| C | 689 | 10 | 898 | 0.767 | 676 | 3.3 | 16.914 | C |


| Arm | Total Demand $(\mathrm{PCU} / \mathrm{hr})$ | Circulating flow (PCU/hr) | Capacity (PCU/tr) | RFC | Throughput (PCU/hr) | End queue (PCU) | Delay (s) | Unsignalised level of service |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| A | 423 | 645 | 677 | 0.624 | 419 | 1.8 | 15.204 | c |
| B | 485 | 264 | 808 | 0.599 | 482 | 1.6 | 12.056 | в |
| c | 823 | 12 | 897 | 0.917 | 803 | 8.3 | 35.901 | E |

08:15-08:30

| Afm | Total Demand <br> (PCU/hr) | Circulating flow <br> (PCUG/hr) | Capaity <br> (PCC/hr) | RFC | Throughput <br> (PCUUhr) | End queue (PCU) | Delay (s) | Unsignalised <br> level of service |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| A | 517 | 709 | 636 | 0.813 | 508 | 4.1 | 28.893 | D |
| B | 593 | 320 | 774 | 0.767 | 587 | 3.3 | 20.438 | C |
| C | 1007 | 14 | 895 | 1.125 | 883 | 39.4 | 112.013 | F |


| Am | Total Demand (PCU/hr) | Circulating flow (PCU/hr) | Capacity (PCUlhr) | RFC | Throughput (PCU/hr) | End queue (PCU) | Delay (s) | Unsignalised level of service |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| A | 517 | 717 | 632 | 0.819 | 516 | 4.5 | 33.193 | D |
| B | 593 | 325 | 771 | 0.770 | 593 | 3.5 | 22.050 | c |
| c | 1007 | 14 | 895 | 1.125 | 893 | 68.1 | 227.823 | F |


| Am | Total Demand (PCU/hr) | Circulating flow (PCU/hr) | Capacity (PCU/hr) | RFC | Throughput (PCU/hr) | End queue (PCU) | Delay (s) | Unsignalised Level |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| A | 423 | 709 | 637 | 0.663 | 431 | 2.3 | 20.012 | c |
| B | 485 | 272 | 804 | 0.603 | 492 | 1.7 | 12.964 | в |
|  |  |  | 897 | 0.917 | 882 | 53.1 | 248.4 |  |


| Am | Total Demand (PCU/hr) | Circulating flow (PCU/hr) | Capacity (PCU/hr) | RFC | Throughput (PCU/hr) | End queue (PCU) | Delay (s) | Unsignalised level of service |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| A | 354 | 706 | 639 | 0.554 | 357 | 1.4 | 14.242 | B |
| B | 406 | 225 | 833 | 0.487 | 408 | 1.1 | 9.392 | A |
| c | 689 | 10 | 898 | 0.767 | 879 | 5.7 | 128.057 |  |

## BTM Base 2026, PM

## Data Errors and Warnings

No errors or warnings

## Junction Network

Junctions

| Junction | Name | Junction type | Use circulating lanes | Arm order | Junction Delay (s) | Junction Los |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | untitled | Mini-roundabout |  | A, B, C | 349.63 | F |

## Junction Network

| Driving side | Lighting | Road surface | In London | Network delay (s) | Network LOs |
| :--- | :--- | :--- | :--- | :--- | :--- |

## Traffic Demand

## Demand Set Details



| D2 | Scenario name | Time Period |
| :---: | :---: | :---: |
|  | BTM Base 2026 | PM |


| Vehicle mix source | PCU Factor for a HV (PCU) |
| :---: | :---: |
| HV Percentages | 2.00 |

Demand overview (Traffic)


## Origin-Destination Data

Demand (PCU/hr)


Vehicle Mix
Heavy Vehicle Percentages

|  | To |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
|  |  | A | B | C |
|  | From | 0 | 10 | 10 |
|  | B | 10 | 0 | 10 |
|  | C | 10 | 10 | 0 |

## Results

Results Summary for whole modelled period

| Am | Max RFC | Max Delay (s) | Max Queue (PCU) | Max Los |
| :---: | :---: | :---: | :---: | :---: |
| A | 0.64 | 12.62 | 1.9 | B |
| B | 1.12 | 222.96 | 55.8 | F |
| C | 1.27 | 607.00 | 153.8 | F |

## Main Results for each time segment

| Am | Total Demand <br> (PCU/hr) | Circulating flow <br> (PCU/hr) | Capacity <br> (PCU/hr) | RFC | Throughput <br> (PCU/hr) | End queue (PCU) | Delay (s) | Unsignalised <br> level of service |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| A | 379 | 285 | 901 | 0.421 | 376 | 0.8 | 7.504 | A |
| B | 575 | 243 | 821 | 0.700 | 565 | 2.4 | 14.969 | B |
| C | 780 | 10 | 898 | 0.868 | 756 | 5.9 | 24.902 | C |

## 17:00-17:15

| Am | Total Demand $(\mathrm{PCU} / \mathrm{hr})$ | Circulating flow (PCU/hr) | Capacity (PCU/hr) <br> (PCU/hr) | RFC | Throughput (PCU/hr) | End queue (PCU) | Delay (s) | Unsignalised level of service |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| A | 453 | 326 | 875 | 0.5 | 452 | 1.2 | 321 | A |
| в | 687 | 292 | 791 | 0.868 | 673 | 5.8 | 30.559 | D |
| c | 931 | 11 | 897 | 1.03 | 867 | 21.9 | 72.622 |  |

## 7:15-17:30

| Am | Total Demand (PCU/hr) | Circulating flow (PCU/hr) | Capacity (PCU/hr) | RFC | Throughput (PCU/hr) | End queue (PCU) | Delay (s) | Unsignalised level of service |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| A | 555 | 337 | 869 | 0.639 | 552 | 1.9 | 12.393 | в |
| B | 841 | 357 | 751 | 1.12 | 736 | 32.1 | 109.222 | F |
| c | 1141 | 13 | 896 | 1.273 | 894 | 83.5 | 223.987 | F |

17:30-17:45

| Am | Total Demand <br> (PCU/hr) | Circulating flow <br> (PCU/hr) | Capacity <br> (PCU/hr) | RFC | Throughput <br> (PCU/hr) | End queue (PCU) | Delay (s) | Unsignalised <br> level of service |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| A | 555 | 337 | 868 | 0.639 | 555 | 1.9 | 12.618 | B |
| B | 841 | 359 | 749 | 1.122 | 746 | 56.8 | 222.63 | F |
| C | 1141 | 13 | 896 | 1.273 | 896 | 144.8 | 467.059 | F |


| Am | Total Demand (PCU/hr) | Circulating flow (PCU/hr) | $\underset{\text { Capacity }}{\text { (PCU/hr) }}$ | RFC | Throughput (PCU/hr) | End queue (PCU) | Delay (s) | Unsignalised level of service |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| A | 453 | 337 | 868 | 0.522 | 456 | 1.2 | 9.662 | A |
| в | 687 | 295 | 789 | 0.870 | 774 | 34.0 | 211.055 | F |
| c | 931 | 13 | 896 | 1.040 | 895 | 153.8 | 607.003 |  |


| Am | Total Demand (PCU/hr) | Circulating flow (PCU/hr) | Capacity (PCU/hr) | RFC | Throughput (PCU/hr) | End queue (PCO) | Delay (s) | Unsignalised level of service |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| A | 379 | 335 | 870 | 0.436 | 381 | 0.9 | 8.128 | A |
| B | 575 | 246 | 819 | 0.702 | 700 | 2.9 | 57.886 | F |
| c | 780 | 12 | 897 | 0.870 | 890 | 126.3 | 567.040 | F |

## BTM 2026 + Proposed Development, AM

## Data Errors and Warnings

No errors or warnings

## Junction Network

Junctions

| Junction | Name | Junction type | Use circulating lanes | Arm order | Junction Delay (s) | Junction Los |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| 1 | untited | Miniriroundabout |  | A, B, C | 309.47 | F |

## Junction Network

| Driving side | Lighting | Road surface | In London | Network delay (s) | Network Los |
| :--- | :--- | :--- | :--- | :--- | :--- |

## Traffic Demand

## Demand Set Details



| D 3 | BTM $2026+$ Proposed Development | AM |
| :--- | :--- | :--- | :--- |


| Vehicle mix source | PCU Factor for a HV (PCU) |
| :---: | :---: |

## Demand overview (Traffic)



## Origin-Destination Data



Vehicle Mix
Heavy Vehicle Percentages


## Results

Results Summary for whole modelled period

| Am | Max RFC | Max Delay (s) | Max Queue (PCU) | Max Los |
| :---: | :---: | :---: | :---: | :---: |
| A | 0.84 | 37.25 | 5.0 | E |
| B | 0.83 | 29.15 | 4.9 | D |
| C | 1.27 | 591.54 | 149.5 | F |

## Main Results for each time segment

| Am | Total Demand <br> (PCU/hr) | Circulating flow <br> (PCU/hr) | Capacity <br> (PCU/hr) | RFC | Throughput <br> (PCU/hr) | End queue (PCU) | Delay (s) | Unsignalised <br> (level of service |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| A | 354 | 622 | 691 | 0.512 | 349 | 1.1 | 11.45 | B |
| B | 437 | 220 | 836 | 0.523 | 433 | 1.2 | 9.715 | A |
| C | 776 | 10 | 898 | 0.864 | 753 | 5.7 | 24.464 | C |

## 08:00-08:15

| Am | Total Demand (PCU/hr) | Circulating flow (PCU/hr) | $\begin{aligned} & \text { Capacity } \\ & \text { (PCU/hr) } \end{aligned}$ | RFc | Throughput (PCU/hr) | End queue (PCU) | Delay (s) | Unsignalised level of service |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| A | 423 | 714 | 633 | 0.667 | 419 | 2.1 | 18.113 | c |
| B | 522 | 264 | 809 | 0.646 | 519 | 1.9 | 13.544 | B |
| c | 927 | 12 | 897 | 1.03 | 865 | 21.1 | 70.606 | F |

08:15-08:30

| Am | $\underset{\substack{\text { Total Demand } \\(\text { PCU/hr })}}{ }$ | Circulating flow (PCU/hr) | Capacity (PCU/hr) | RFC | Throughput (PCU/hr) | End queue (PCU) | Delay (s) | Unsignalised level of service |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| A | 517 | 737 | 619 | 0.836 | 507 | 4.6 | 32.653 | D |
| в | 640 | 320 | 774 | 0.827 | 629 | 4.5 | 25.684 | D |
| c | 1135 | 14 | 895 | 1.268 | 893 | 81.6 | 218.800 | F |


| Am | Total Demand <br> (PCCULr) | Circulating flow <br> (PCU/hr) | Capacity <br> (PCUUTh) | RFC | Throughput <br> (PCU/hr) | End queue (PCU) | Delay (s) | Unsignalised <br> (eveel of service |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| A | 517 | 739 | 618 | 0.837 | 516 | 5.0 | 37.250 | E |
| B | 640 | 325 | 771 | 0.830 | 638 | 4.9 | 29.150 | D |
| C | 1135 | 14 | 895 | 1.268 | 895 | 141.7 | 457.317 | F |


| Am | Total Demand (PCU/hr) | Circulating flow (PCU/hr) | Capacity (PCU/hr) | RFC | Throughput (PCU/hr) | End queue (PCU) | Delay (s) | Unsignalised level of service |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| A | 423 | 739 | 618 | 684 | 433 | 2.5 | 22.363 | c |
| в | 522 | 272 | 803 | 0.650 | 533 | 2.1 | 15.231 | c |
| c |  |  |  | 1.034 |  | 149.5 | 591.53 |  |


| Am | $\underset{\substack{\text { Total Demand } \\ \text { (PCU/hr) }}}{\text { 为 }}$ | $\underset{(\text { PCU/hr) }}{\text { Circulating flow }}$ | Capacity (PCulhr) | RFC | Throughput (PCU/hr) | End queue (PCU) | Delay (s) | Unsignalised level of service |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| A | 354 | 736 | 620 | 0.571 | 358 | 1.5 | 15.325 | c |
| в | 437 | 225 | 832 | 0.525 | 441 | 1.2 | 10.205 | в |
| c | 776 | 10 | 898 | 0.864 | 891 | 120.7 | 546.457 | F |



## BTM 2026 + Proposed Development, PM

## Data Errors and Warnings

No errors or warnings

## Junction Network

Junctions

| Junction | Name | Junction type | Use circulating lanes | Arm order | Junction Delay (s) | Junction Los |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| $\mathbf{1}$ | untited | Miniriroundabout |  | A, B, C | 527.20 | F |

## Junction Network

| Driving side | Lighting | Road surface | In London | Network delay (s) | Network Los |
| :--- | :--- | :--- | :--- | :--- | :--- |

## Traffic Demand

Demand Set Details

D4 4 BTM $2026+$ Proposed Development
ONE HOUR
16:45
18:15
15

| Vehicle mix source | PCU Factor for a HV (PCU) |
| :---: | :---: |
| HV Percent | 2.00 |

Demand overview (Traffic)


## Origin-Destination Data



Vehicle Mix
Heavy Vehicle Percentages


## Results

Results Summary for whole modelled period

| Am | Max RFC | Max Delay (s) | Max Queue (PCU) | Max Los |
| :---: | :---: | :---: | :---: | :---: |
| A | 0.63 | 12.20 | 1.9 | B |
| B | 1.25 | 472.77 | 105.7 | F |
| C | 1.34 | 807.01 | 208.4 | F |

## Main Results for each time segment

| Am | Total Demand <br> (PCU/hr) | Circulating flow <br> (PCU/hr) | Capacity <br> (PCU/hr) | RFC | Throughput <br> (PCU/hr) | End queue (PCU) | Delay (s) | Unsignalised <br> level of service |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| A | 379 | 282 | 903 | 0.420 | 376 | 0.8 | 7.478 | A |
| B | 640 | 243 | 821 | 0.779 | 626 | 3.5 | 19.99 | C |
| C | 823 | 10 | 898 | 0.916 | 791 | 8.1 | 30.907 | D |

17:00-17:15

| Amm | Total Demand <br> (PCU/hr) | Circulating flow <br> (PCU/hr) | Capacity <br> (PCU/hr) | RFC | Throughput <br> (PCU/hr) | End queue (PCU) | Delay (s) | Unsignalised <br> level of service |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| A | 453 | 315 | 882 | 0.514 | 452 | 1.1 | 9.164 | A |
| B | 764 | 292 | 791 | 0.966 | 732 | 11.5 | 50.457 | F |
| C | 983 | 11 | 897 | 1.095 | 882 | 33.3 | 99.667 | F |


| Am | Total Demand (PCU/hr) | Circulating flow (PCU/hr) | Capacity (PCU/hr) | RFC | Throughput (PCU/hr) | End queue (PCU) | Delay (s) | Unsignalised level of service |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| A | 555 | 320 | 879 | 0.631 | 552 | 1.8 | 12.008 | B |
| в | 936 | 357 | 751 | 1.247 | 746 | 58.9 | 184.471 | F |
| c | 1203 | 11 | 897 | 1.342 | 896 | 110.2 | 29.553 | F |

17:30-17:45

| Am | Total Demand <br> (PCU/hr) | Circulating flow <br> (PCU/hr) | Capacity <br> (PCU/hr) | RFC | Throughput <br> (PCU/hr) | End queue (PCU) | Delay (s) | Unsignalised <br> (level of service |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| A | 555 | 320 | 879 | 0.631 | 555 | 1.9 | 12.203 | B |
| B | 936 | 359 | 749 | 1.249 | 749 | 105.7 | 399.949 | F |
| C | 1203 | 11 | 897 | 1.342 | 897 | 186.8 | 603.780 | F |


| Am | Total Demand $(\mathrm{PCU} / \mathrm{hr})$ | Circulating flow (PCU/hr) | Capacity (PCU/hr) | RFC | Throughput (PCU/hr) | End queue (PCU) | Delay (s) | Unsignalised Ievel of service |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| A | 453 | 320 | 879 | 0.515 | 456 | 1.2 | 9.413 | A |
| B | 764 | 295 | 789 | 0.968 | 781 | 101.4 | 472.771 | F |
| c | 983 | 12 | 897 | 1.096 | 896 | 208.4 | ${ }^{800.353}$ | F |


| Am | Total Demand $(\mathrm{PCU} / \mathrm{hr})$ | Circulating flow (PCU/hr) | Capacity (PCU/hr) | RFC | Throughput (PCU/hr) | End queue (PCO) | Delay (s) | Unsignalised level of service |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| A | 379 | 318 | 880 | 0.431 | 381 | 0.8 | 7.953 | A |
| B | 640 | 246 | 819 | 0.781 | 811 | 58.7 | 357.827 | F |
| c | 823 | 12 | 896 | 0.918 | 892 | 191.2 | 807.011 | F |

# ATTACHMENT B 

VTP DRAWINGS


Firetharn Trust

ElMSBROGK SPINE
RロAD Ag马E马SMENT






FIRETHIRN Trust

## ATTACHMENT C <br> APPLICATION DRAWINGS




## ATTACHMENT D

FOOTBRIDGE INFORMATION

