



ENVIRONMENTAL STATEMENT
VOLUME 2
APPENDIX 4.2 – OUTLINE CONSTRUCTION
PROGRAMME

PROPOSED GREAT WOLF LODGE CHESTERTON, BICESTER

Outline Construction Programme

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VERSION CONTROL

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1 Introduction

This document provides Construction Methodology and Programme of Works for the development of Great Wolf Lodge ('the proposed development') on part of the existing golf course at Bicester Hotel Golf and Spa, Chesterton, Bicester, Oxfordshire.

The scheme comprises the reconfiguration of the existing golf course (back nine) for the inclusion of the hotel, leisure facilities and associated car parking. The existing hotel (and leisure / spa facilities) and the front nine holes will remain.

This document provides details of how we envisage construction of the works will proceed in the relevant sequence all of which is supported by the programme contained under Appendix A.

2 Programme Assumptions

The construction works will commence with setting out ahead of undertaking the reduced level excavation. Due to the extent of area this will most likely be undertaken using heavy excavation equipment such as a Grader/ bulldozer to form spoil heaps which will then be loaded into tipper wagons using 360 excavators and relocated/ transported off site. Once the appropriate levels are achieved the piling mat will be imported across the site again using tipper wagons and excavation equipment to roughly level the area ahead of compacting with a vibrating roller.

A dewatering strategy and equipment will be employed across the site to account for the existing water table (circa 250mm below ground level) during the excavation and piling activities and until the ground floor structure is erected. Some elements of this strategy may remain in place until the permanent drainage is installed, however, this is subject to the build-up of external finishes.

As the piling mat becomes established piling rigs will be delivered to undertake the bored piles, we have assumed up to twelve rigs on site. Pile caps will be excavated and cast thereafter along with installing any under-slab drainage and service ducts before setting up the formation level and pouring the ground floor slab areas. Once suitable areas of ground floor slab are available the superstructure frame will commence based on a mixture of concrete and steelwork. With regards to craneage for the Accommodation wing, the Family Entertainment Centre (FEC) and the Waterpark (WP) the most suitable craneage requirements for the erection of the various elements and parts of the project will be considered with the Main Contractor.

The envelope construction is based on a Structural Frame System (SFS) with prefabricated brick slip panels and windows, we expect this to be installed using a mixture of external scaffolds and mast climbers. Roof panels will be distributed using suitable cranes and accessed from the same mast climber and scaffold arrangement.

As areas become weathertight fit out works will commence across the differing building areas across the development. We anticipate up to 4Nr twin goods/ passenger hoists for the accommodation block. As the FEC and Waterpark areas are single storey it is unlikely they will require external hoist access, although machinery including forklifts, access platforms and cherry pickers will be used to access the double height areas internally

External works will be conducted using similar excavation and compaction equipment, with forklifts distributing materials across the areas. Access roads and parking areas are to be tarmac covered and this will be undertaken using an asphalt paver and compact roller ahead of applying road markings.

3 Programme Outputs for Groundworks and Superstructure Concrete

Below we have identified the typical programme output for the groundworks and the concrete works associated with the superstructure

- Excavation; Up to 925m³ per day
- Imported fill; Up to 415m³ per day
- Piles; 5Nr per rig day
- Concrete slabs; 500m² per week (per area).

4 Delivery Expectations

In order to fully appreciate the number of deliveries we need to fully understand the quantum of works which will influence this. As such we will need to first establish a potential average across the on-site duration and then delve deeper to understand the potential peaks and troughs during the construction period. The overall quantum of work appears to include;

4.1 Reduced Level Excavation

Without the benefit of a precise cut and fill exercise we have looked to make an assumption that removing 2m of earth across the site will encompass all made ground and take the site to below the 650mm deep piling mat level, which would result in the following;

$$25,408\text{m}^2 \times 2\text{m} = 50,816\text{m}^3 / 15\text{m}^3 = 3,388\text{Nr wagon loads}$$

4.2 Piling Mat

We have assumed the piling mat will be 650mm deep, therefore;

$$25,408\text{m}^2 \times 0.65\text{m} = 16,516\text{m}^3 / 15\text{m}^3 = 1,102\text{Nr wagon loads} +25\% \text{ perimeter crane hard standings and compaction } (4,129\text{m}^3) = 20,645\text{m}^3 / 15\text{m}^3 = 1,377\text{Nr wagon loads}$$

4.3 Piling

a. Accommodation areas

Typically show 14Nr piles across an area of 47.88m², based on 450mm diameter and 12m deep. With pile caps 1m deep and approx. 16m³ per area of 47.88m². Therefore;

$$\text{Pile numbers} = 1,775\text{Nr based on a typical floor area of } 6,066\text{m}^2$$

$$\text{Pile Arisings} = 1,775 \times (12 \times 0.45) = 9,585\text{m}^3 / 15\text{m}^3 = 639\text{Nr loads}$$

$$\text{Concrete piles} = 9,585\text{m}^3 / 6\text{m}^3 = 1598\text{Nr loads}$$

$$\text{Pile Cap Arisings} = 6,066\text{m}^2 / 47.88\text{m}^2 = 127\text{Nr} \times 16\text{m}^3 = 2,032\text{m}^3 / 15\text{m}^3 = 136\text{Nr loads}$$

$$\text{Concrete Pile Caps} = 127\text{Nr} \times 16\text{m}^3 = 2032\text{m}^3 / 6\text{m}^3 = 339\text{Nr loads}$$

$$\text{Excavation total} - 775\text{Nr loads}; \text{Concrete total} - 1937\text{Nr loads}$$

b. FEC areas (BoH/ FoH)

Typically show 28Nr piles across an area of 225m², based on 450mm diameter and 12m deep. With pile caps 1m deep and approx. 17.5m³ per area of 225m². Therefore;

$$\text{Floor Area} = 14,229\text{m}^2 - (6,066\text{m}^2 - 4,338\text{m}^2) = 12,501\text{m}^2 / 225\text{m}^2 = 56\text{Nr}$$

$$\text{Pile numbers} = 56 \times 28 = 1,568\text{Nr}$$

$$\text{Pile Arisings} = 1,568 \times (12 \times 0.45) = 8,468\text{m}^3 / 15\text{m}^3 = 565\text{Nr loads}$$

Concrete piles = $8,468\text{m}^3 / 6\text{m}^3 = 1412\text{Nr}$ loads

Pile Cap Arisings = $56\text{Nr} \times 17.5\text{m}^3 = 980\text{m}^3 / 15\text{m}^3 = 66\text{Nr}$ loads

Concrete Pile Caps = $56\text{Nr} \times 17.5\text{m}^3 = 980\text{m}^3 / 6\text{m}^3 = 164\text{Nr}$ loads

Excavation total – 631Nr loads; Concrete total – 1576Nr loads

c. Waterpark areas

Typically show 72Nr piles across an area of 900m², based on 450mm diameter and 12m deep. With pile caps 1m deep and approx. 35m³ per area of 900m². Therefore;

Floor Area = $7,198\text{m}^2 / 900\text{m}^2 = 8\text{Nr}$

Pile numbers = $8 \times 72 = 576\text{Nr}$

Pile Arisings = $576 \times (12 \times 0.45) = 3,111\text{m}^3 / 15\text{m}^3 = 208\text{Nr}$ loads

Concrete piles = $3,111\text{m}^3 / 6\text{m}^3 = 519\text{Nr}$ loads

Pile Cap Arisings = $8\text{Nr} \times 35\text{m}^3 = 280\text{m}^3 / 15\text{m}^3 = 19\text{Nr}$ loads

Concrete Pile Caps = $8\text{Nr} \times 35\text{m}^3 = 280\text{m}^3 / 6\text{m}^3 = 47\text{Nr}$ loads

Excavation total – 227Nr loads; Concrete total – 566Nr loads

4.4 Basement Excavation

These works consider the basement bunker and pool areas associated with the Waterpark building

Bunker Building – $871\text{m}^2 \times 5\text{m} = 4,355\text{m}^3 / 15\text{m}^3 = 291\text{Nr}$ loads

Pool works – $287\text{m}^2 \times 1.5\text{m} = 430.5 + 1,901\text{m}^3 = 2,331.5\text{m}^3 / 15\text{m}^3 = 156\text{Nr}$ loads

Total loads 447Nr

4.5 Ground Floor Concrete Slab

The ground floor slab to all areas of the development are noted as follows:

Accommodation – $6,066\text{m}^2 \times 0.3\text{m} = 1820\text{m}^3 / 6\text{m}^3 = 304\text{Nr}$ loads

FEC – $12,501\text{m}^2 \times 0.3\text{m} = 3751\text{m}^3 / 6\text{m}^3 = 626\text{Nr}$ loads

Waterpark – $7,198\text{m}^2 \times 0.3\text{m} = 2160\text{m}^3 / 6\text{m}^3 = 360\text{Nr}$ loads

E/o Pool Walls say $300\text{m}^3 / 6\text{m}^3 = 50\text{Nr}$ loads

Total loads 1040Nr

4.6 Substructure Totals

The total loads for the substructure works are noted as follows:

- Excavation – $3,338\text{Nr} + 775\text{Nr} + 631\text{Nr} + 227\text{Nr} + 447\text{Nr} = 5,418\text{Nr}$ loads
- Concrete – $1,937\text{Nr} + 1,576\text{Nr} + 566\text{Nr} + 1,040\text{Nr} = 5,119\text{Nr}$ loads

- Reinforcement – Say 25% of concrete; $5,119 \times 25\% = 1,280$ Nr Loads
- Piling mat – 1,377Nr loads

Overall loads = 13,194Nr loads

4.7 Superstructure

4.7.1 Concrete slabs

Accommodation – $6,066\text{m}^2 \times 0.3\text{m} = 1,820\text{m}^3 / 6\text{m}^3 = 304$ Nr loads $\times 4$ levels = 1216Nr loads

Reinforcement & decking – say 25% of concrete; $1216 \times 25\% = 304$ Nr loads

Total loads 1,520Nr Loads

4.7.2 Steelwork

Allowance 1 tonne per m^2 based onloads of 30 tonnes

Accommodation = $6,066$ tonnes/ $30 = 203$ Nr Loads

FEC = $12,501$ tonnes/ $30 = 417$ Nr Loads

Waterpark = $7,198$ tonnes/ 30 tonnes = 240 Nr loads

Total Structural Steelwork $203 + 417 + 240 = 860$ Nr Loads

5 Delivery Numbers

Overall number of substructure & superstructure frame loads equates to $13194 + 1520 + 860 = 15,574$.

Therefore, if we have assumed that the envelope and fit out will produce the same amount of deliveries including waste removal as substructure and frame, equating to approx. 31,150 deliveries overall.

As such if we divide this by the number of working days per year (240 days) based on an overall programme of 24 months (480 working days) we see an average of 65 deliveries per day which when considered over an 8 hour day equates to 8 deliveries per hour (1 delivery every 7 minutes); or when split over up to 3 delivery/ site locations across the site this becomes approx. 2- 3 deliveries per hour.

5.1 Delivery Peak

Based on the programme output the initial peak will involve earthworks, piling mat and commencement of piling to the accommodation wings. Based on typical outputs of;

- Excavation 925m³ per day
- Piling mat 415m³ per day
- Piling 5 piles per rig day (using 6Nr rigs) 162m³ per day (excavation and reinforced concrete)

This initial peak will therefore be;

- Excavation 62Nr loads per day
- Pile Mat 28Nr loads per day
- Piling 36Nr loads per day

Overall 126Nr loads per day; say 130Nr

During Substructure works associated with Accommodation FEC and Waterpark this becomes;

Accommodation Wings

Piling – 27m³/ Rig day x 6 Rigs = 162m³/ 15m³ = 11 Loads spoil; 108/ 6m³ = 20 Loads concrete (+ 25% Reinforcement) = 25 Loads; 36 Loads overall per day

Pile caps – 2,032m³/ 50 days = 40.7m³ per day / 15m³ = 3 loads spoil; 19.6m³/ 6m³ = 7 loads concrete + 25% RF = 9 loads; 12 loads overall

GF Slab – 500m² per week/ 5 days = 100m² x 0.3m = 30m³/ 6m³ = 5 loads (+25% RF) = 6.25 loads

Overall 36 + 12 + 6 = 54 loads

FEC Area

Piling – 27m³/ Rig day x 4 Rigs = 108m³/ 15m³ = 8 Loads spoil; 108/ 6m³ = 18 Loads concrete (+ 25% Reinforcement) = 23 Loads; 31 Loads overall per day

Pile caps – 980m³/ 85 days = 11.6m³ per day / 15m³ = 1 load spoil; 11.6m³/ 6m³ = 2 loads concrete + 25% RF = 2.5 loads; 7 loads overall

GF Slab – 500m² per week/ 5 days = 100m² x 0.3m = 30m³/ 6m³ = 5 loads (+25% RF) = 6.25 loads

Overall 31 + 7 + 6 = 44 loads

Waterpark

Piling – 27m^3 / Rig day x 2 Rig = 54m^3 / 15m^3 = 4 Loads spoil; $54/ 6\text{m}^3$ = 9 Loads concrete (+ 25% Reinforcement) = 11.25 Loads; 21 Loads overall per day

Pile caps – 280m^3 / 60 days = 4.7m^3 per day / 15m^3 = 0.33 loads spoil; 4.7m^3 / 6m^3 = 0.8 loads concrete + 25% RF = 1 load; 2 loads overall

Basement/ Pool Excavation – $6,666.5\text{m}^3$ / 35 days = 190.5m^3 per Day/ 15m^3 = 13 Loads spoil

Overall = $11 + 2 + 13 = 26$ loads per day

Combine substructure Accommodation, FEC & Waterpark

$54 + 44 + 26 = 124$ loads per day

5.2 Delivery Range

Based on the above we would suggest the following delivery movements are anticipated across the project duration;

- Enabling; 10 – 130Nr Loads per day
- Substructure works; 60 – 130Nr loads per day
- Superstructure/ Envelope Works; 40 – 80Nr loads per day
- Fit Out; 10 – 65Nr Loads per day

These numbers may reduce if excavated material is retained/ reused on site to circa;

- Enabling; 10 – 80Nr Loads per day
- Substructure works; 60 – 130Nr loads per day

The numbers across superstructure and fit out could reduce significantly (approx. 50%) if modular units are used for the accommodation block;

- Superstructure/ Envelope Works; 20 – 40Nr loads per day
- Fit Out; 10 – 30Nr Loads per day

5.3 Delivery Vehicles

We anticipate the following vehicles will be undertaking deliveries to and from site during these periods;

- Tipper wagon (approx. 15m^3)
- Concrete wagon (approx. 6m^3)
- Skip Wagon
- HGV with flatbed trailer (approx. 30- 40t)
- HGV with curtained trailer (approx. 30- 40t)
- Rigid trucks
- Vans

5.4 Plant & Machinery

Other plant machinery would include;

- Grading excavator/ Bulldozer
- 360 Excavator
- Compact roller
- Piling rig
- Mobile or tower crane
- Concrete pump
- Mast climbers
- Forklift
- Goods Hoist
- Access platform/ cherry picker
- Asphalt paver

5.5 Construction Operatives

Although the number of Construction operatives on-site cannot be ascertained prior to appointment of a Principal Contractor and their Sub-Contractors, we are assuming the following average number of operatives will be present on site during the listed phases:

- Enabling; **10 - 50 Men on site**
- Substructure; **50 - 100 Men on Site**
- Superstructure & Envelope; **100 - 150 Men on Site**
- Envelope & Fit Out; **150 - 400 Men on Site**

In order to minimise the number of operatives' journeys to site, we would expect the Principal Contractor to consider utilising a shuttle bus from the local train station. The proposed times of a shuttle bus service can be timed around peak rush hour; however, this will be further clarified in the Construction Management Plan as it is developed.

Conclusion

Based on the above we would envisage that the delivery ranges will include;

- Enabling; 10 – 130Nr Loads per day
- Substructure works; 60 – 130Nr loads per day
- Superstructure/ Envelope Works; 40 – 80Nr loads per day
- Fit Out; 10 – 65Nr Loads per day

The level of design information currently available for this project means these figures are undoubtedly subjective, while we have looked to focus on the key activities which can be measured or defined, we have not recorded outputs against all programme activities. Elements such as;

- Below slab drainage and services
- External works including parking areas and landscaping
- Temporary site access roads and site accommodation/ welfare areas
- Secant and sheet piles
- Machinery deliveries

have not been considered because it is difficult to fully comprehend the scope, measurable areas and extent of deliveries associated with these works until the design has been fully developed. Facades, Roof, MEP Plant areas and fit out activities have not been considered beyond the assumption of the x2 multiplier referenced above for this same reason mixed with the inability to understand the volume of material which can be transported to site on one vehicle, and full deliveries may not be solely for this project.

As such we have looked to offer an informed 'best guess' based on the things we can measure at this stage to inform the EIA submission associated with the planning application. By basing these figures on the assumptions associated with reduced level (2m across the building area), and that all excavated material will be removed from site including piling and basement/ pool and attempting calculations to include reinforcement and steel frames we have looked to address a worst-case scenario.

As we have previously noted there may be trade off's which look to reduce the scope of delivery movements (retain spoil on site and use of modular buildings for accommodation wings) but at such an early stage it makes sense to offer a higher number rather than lower as any penalties will be associated with exceeding delivery numbers rather than achieving lower and we would welcome a view point from WSP based on past experience for similar size and value developments of this nature to benchmark our figures against.

APPENDIX A

Construction Programme

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