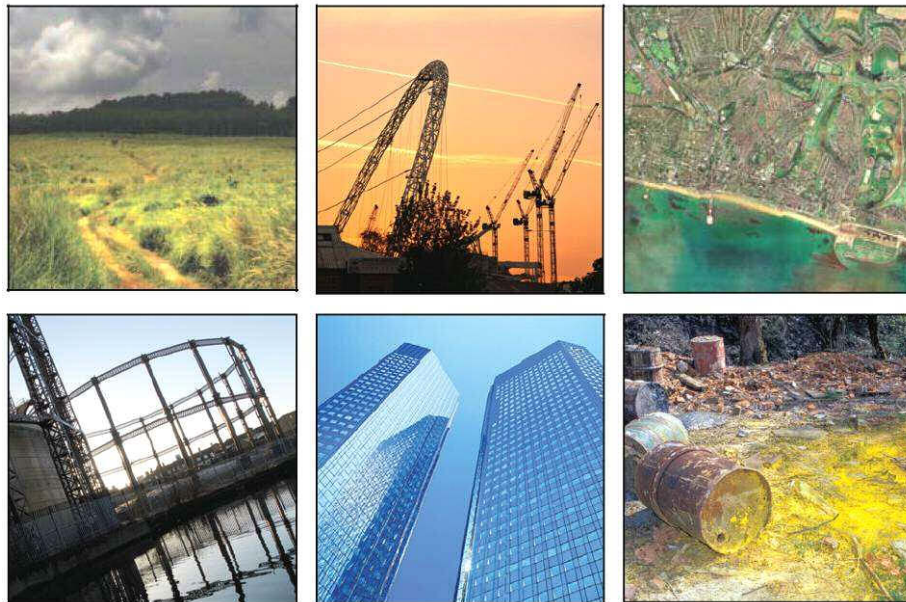




KINGSMERE PARCELS KM7 AND KM9

SURFACE WATER DRAINAGE DESIGN STRATEGY

JANUARY 2013



**KINGSMERE
PARCELS KM7 AND KM9
SURFACE WATER DRAINAGE
DESIGN STRATEGY**

RPS PROJECT No: JKK6647

ISSUE	DATE	REMARKS
A	08.01.13	First Issue

Our Ref: JKK6647

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QUALITY MANAGEMENT

<u>Author</u>	<u>Qualifications</u>	<u>Signature</u>
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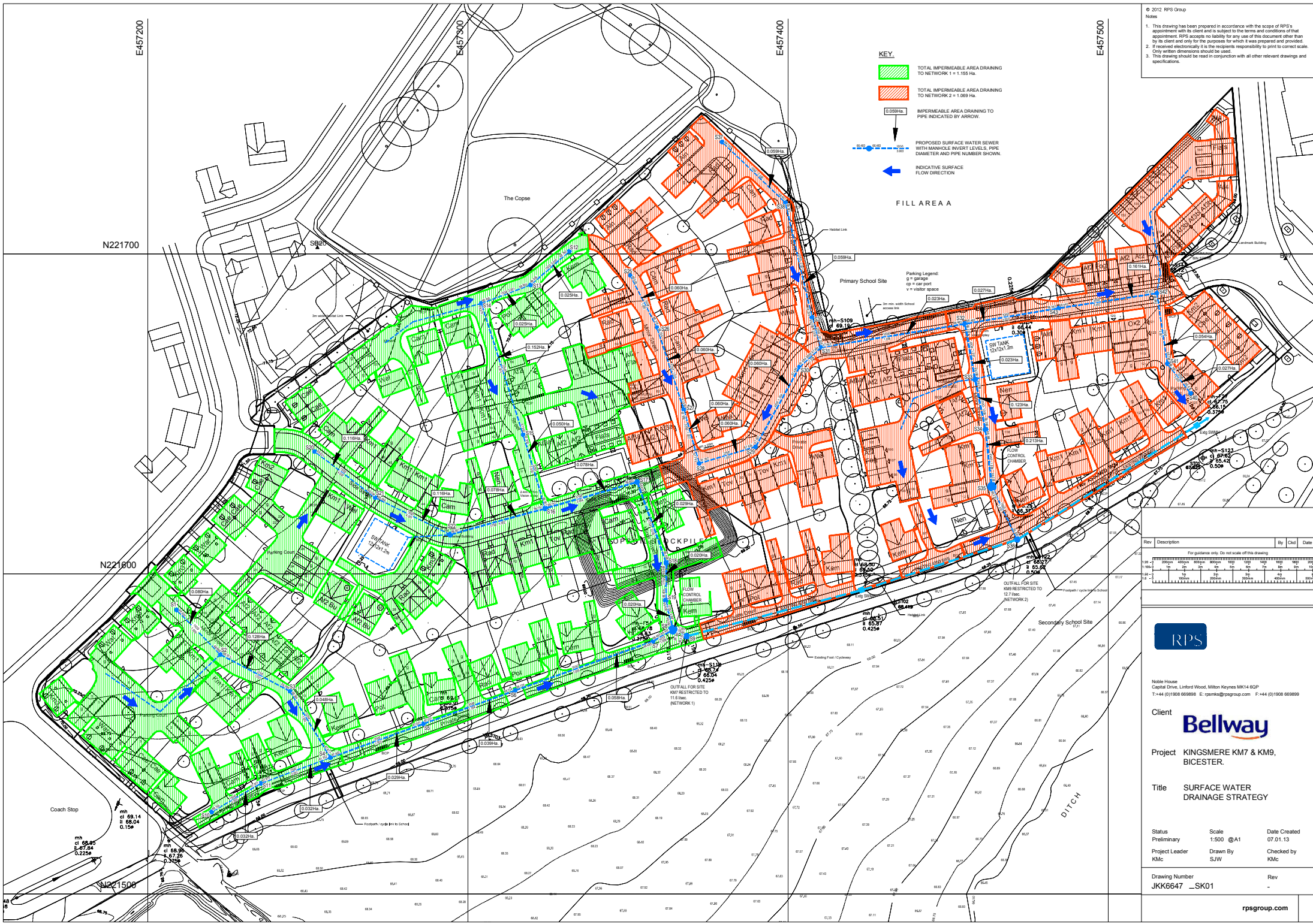
The material presented in this report is confidential. This report has been prepared for the exclusive use of the client and shall not be distributed or made available to any other company or person without the knowledge and written consent of the client or RPS.

1 SUMMARY

- 1.1 RPS Group has been commissioned to prepare a surface water drainage strategy for proposed residential parcels KM7 and KM9 of the Kingsmere development at Bicester. The proposed strategy is shown on the appended drawing JKK6647 SK01.
- 1.2 The overall surface water strategy for Kingsmere was prepared by WSP in consultation with Oxfordshire County Council and the Environment Agency. Each land parcel has been allocated a maximum surface water discharge into the infrastructure surface water system. Where the soil conditions allow, each parcel should utilise soakaways and permeable paving. Drawing 1903/D/006P states *'The on-plot storage equates to 100m³ per hectare of gross parcel area. This storage is designed to attenuate the 1 in 10 year event. The storage can include soakaways where viable'*.
- 1.3 A ground investigation for KM7 and KM9 and topographical survey has identified that the site has been filled with partly organic material by up to 2.3metres and also that part of the site is underlain clay. The report concludes that these two parcels are not suitable for the use of soakaways
- 1.4 Subsequently, the proposed surface water strategy is to provide a conventional piped surface water system with impermeable roads and paved areas. Attenuation will be provided in the form of below ground attenuation cellular tanks. The tanks have been sized to attenuate the 1 in 10 year rainfall event. Rainfall events in excess of the 1 in 10 event year are allowed to overflow into the infrastructure surface water system, with 100 year plus climate change attenuation provided within Detention Basin 4 in the south-west corner of The Development.
- 1.5 In accordance with the WSP strategy, the surface water discharge from KM7 and KM9 will be restricted to 11.6 and 12.7 litres per second respectively. Micro-drainage calculations appended to this report demonstrate that the system will not flood during a 1 in 10 year event.
- 1.6 The rate of surface water discharge will be restricted by a flow control device such as a hydrobrake at the proposed points of connection to the infrastructure surface water system. The control chambers will include an overflow that will prevent surface water flooding through the manhole covers. However, if there was to be a blocked pipe, there could be a situation where there will be surface water flows. Drawing SK01 shows indicative routes that surface flow would take demonstrating that there would be no risk of flooding to the proposed dwellings.
- 1.7 The proposed surface water drainage system including the attenuation tanks would be offered for adoption by Oxfordshire County Council or a Management Company.

APPENDIX A
PROPOSED SURFACE WATER DESIGN
DRAWING JKK6647 SK01

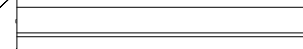
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 Notes
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 2. If received electronically it is the recipient's responsibility to print to correct scale. Only written dimensions should be used.
 3. This drawing should be read in conjunction with all other relevant drawings and specifications.



KEY

- TOTAL IMPERMEABLE AREA DRAINING TO NETWORK 1 = 1.155 Ha.
- TOTAL IMPERMEABLE AREA DRAINING TO NETWORK 2 = 1.069 Ha.
- IMPERMEABLE AREA DRAINING TO PIPE INDICATED BY ARROW.
- PROPOSED SURFACE WATER SEWER WITH MANHOLE INVERT LEVELS, PIPE DIAMETER AND PIPE NUMBER SHOWN.
- INDICATIVE SURFACE FLOW DIRECTION.

Rev	Description	By	Chd	Date
1	For guidance only. Do not scale off this drawing.			



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Client **Bellway**

Project **KINGSMERE KM7 & KM9, BICESTER.**

Title **SURFACE WATER DRAINAGE STRATEGY**

Status Preliminary	Scale 1:500 @A1	Date Created 07.01.13
Project Leader KMc	Drawn By SJW	Checked by KMc
Drawing Number JKK6647 _SK01	Rev -	

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APPENDIX B

WSP DRAINAGE STRATEGY

DRAWING 1903/D/006P



PETROL INTERCEPTOR FOR NEW ROUNDABOUT

EXISTING HIGHWAY DITCH

PETROL INTERCEPTOR FOR NEW ROUNDABOUT

DITCH DIVERTED FOR ROUNDABOUT

PINGLE BROOK

EXISTING HIGHWAY DITCH SYSTEM TO BE RETAINED

PINGLE BROOK DIVERSION

SW ATTENUATION REQUIRED ON PLOT

PINGLE BROOK

WHITELANDS FARM

VILLAGE

Commercial Centre

WHITELANDS FARM

WHITELANDS

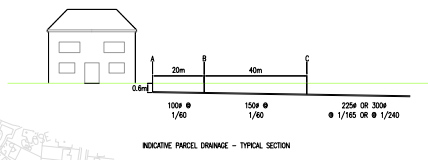
A41 ROADSIDE DITCH

EXISTING HIGHWAY DITCH SYSTEM TO BE RETAINED

PROPOSED DITCH WITHIN FENCED BUFFER ZONE OUTSIDE SCHOOL SITE

GARDEN CENTRE

PETROL INTERCEPTORS FOR NEW ROUNDABOUT



NORTH CATCHMENT	
GROSS CATCHMENT AREA =	38.3 HA
ENGINEERED CATCHMENT =	25.6 HA
HARD STANDING =	19.4 HA
ALLOWABLE DISCHARGES FROM ENGINEERED CATCHMENT:	
RETURN PERIOD	FLOW (l/s)
2	82
10	151
30	212
100+30%	388

DO NOT SCALE

LEGEND

- SURFACE WATER FLOW DIRECTION
- INDICATE PARCEL CRITICAL DRAIN ROUTE
- RETAINED EXISTING DITCHES
- NEW OR RELOCATED DITCHES
- INDICATE SURFACE WATER DRAIN ROUTES
- NORTHERN GROSS CATCHMENT BOUNDARY
- NORTHERN ENGINEERED CATCHMENT AREA DRAWING TO BASIN
- SOUTHERN GROSS CATCHMENT BOUNDARY
- SOUTHERN ENGINEERED CATCHMENT AREA DRAWING TO BASIN
- DETENTION BASINS
- PARCEL CONNECTION MANHOLE
- PARCEL BOUNDARY

NOTE 1: THE ON-PLLOT STORAGE VOLUME EQUATES TO 1000L PER HECTARE OF GROSS PARCEL AREA. THIS STORAGE IS DESIGNED TO ATTENUATE THE 1 IN 10 YEAR DESIGN FLOOD. THE STORAGE ON INCLUDES SUMMITS WHERE VISIBLE.

NOTE 2: INTERFLOW AND STORAGE FOR EACH PLOT SHALL ACCOMMODATE AT LEAST THE 1 IN 10 YEAR DESIGN FLOOD IN EXCESS OF THE 1 IN 10 YEAR RAIN OVERFLOW TO THE STRATEGIC SURFACE WATER NETWORK AND BE ATTENUATED WITHIN PROPOSED BASINS LOCATED AT THE OUTFALLS.

CHANGES: SINCE THE FIRST PARCEL LAYOUTS AND SIZES HAVE CHANGED SINCE THE FIRST PARCEL LAYOUTS, THIS HAS THEREFORE CHANGED THE ENGINEERED CATCHMENT AREAS AND SUBSEQUENTLY THE ALLOWABLE DISCHARGE RATES FOR THE CATCHMENTS.

Northern Catchment Details

Sub-Parcel	Gross Area (ha)	Imp Factor	Note 2				Pre-development Max Permitted Flow Rates per Parcel							
			Q1 1% Storage Vol (m³)	Q1 10% Connection (L/s)	Q1 30% Downstream Pipe No.	Q1 100% Downstream Pipe Size (mm)	Q2 1% (l/s)	Q2 10% (l/s)	Q2 30% (l/s)	Q2 100% (l/s)				
RM1	0.260	1.00	1.307	84.9	0.120	76.256	19.023	480	53	10.8	12.2	27.76		
RM2	0.090	1.70	0.662	36.1	0.060	21.961	5.028	71.961	12.028	480	2.9	6.1	7.1	23.06
RM3	1.052	1.20	1.207	145.2	0.110	124.015	23.029	309	9.7	10.9	14.9	26.16		
RM4	2.820	1.70	2.116	182.0	0.230	123.777	23.027	326	9.0	10.5	25.1	42.34		
RM5	3.082	1.70	2.312	206.2	0.250	131.368	18.028	480	9.9	10.1	20.3	46.28		
RM6	2.356	1.70	1.743	132.4	0.17	107.024	18.027	380	7.4	10.4	10.1	34.66		
RM7a	1.027	1.70	1.226	63.7	0.090	27.144	11.026	480	3.2	9.6	13.4	24.99		
RM7b	1.143	1.70	1.367	74.2	0.090	29.146	11.026	480	3.8	9.7	13.4	27.76		
RM7c	1.090	1.70	1.316	68.8	0.090	27.144	11.026	480	3.5	9.4	13.4	25.11		
RM7d	1.026	1.20	0.989	52.9	0.020	10.422	11.024	380	2.3	6.0	8.4	15.98		
RM7e	0.871	1.70	1.063	67.1	0.12	68.485	11.027	480	2.8	5.1	7.1	23.06		
RM7f	0.990	1.70	1.166	68.1	0.10	69.746	11.026	480	2.9	6.3	7.6	23.06		
RM7g	0.000	1.70	0.400	0.00	0.00	0.00	0.00	375	1.9	3.5	4.9	8.0		
RM7h	0.788	1.00	0.788	0.00	0.00	0.00	0.00	375	1.9	3.5	4.9	8.0		
Highway	16.762	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.0	0.0	0.0	0.0		
Total	38.302								93.1	126.2	132.2	216.6		

Engineered Catchment Drawing Directly to Pingle Brook

RM20	1.272	1.70	1.561	91.1	0.10	81.100	11.026	480	2.1	7.3	10.6	19.1
RM21	0.961	1.70	1.248	62.1	0.10	62.100	11.026	480	2.1	7.3	10.6	19.1
RM22	2.996	1.70	2.140	141.1	0.10	141.100	11.026	480	9.1	10.3	21.5	42.0
Total	5.229								13.3	25.2	32.2	78.2

NOTE: KM21 and KM22 are to the east of the northern catchment and will need to be dealt with separately to Pingle Brook and the storm water attenuation on plot storage.

Northern Catchment Totals

Engineered Catchment	25.56	Yes	16.583	On Plot	—	—	—	—	82.3	15.4	21.8	267.5
Retained Open Space	12.72	—	0.900	Yes	—	—	—	—	0.0	0.0	0.0	0.0
Total	38.282		19.283						82.3	15.4	21.8	267.5

Southern Catchment Details

Sub-Parcel	Gross Area (ha)	Imp Factor	Note 2				Pre-development Max Permitted Flow Rates per Parcel					
			Q1 1% Storage Vol (m³)	Q1 10% Connection (L/s)	Q1 30% Downstream Pipe No.	Q1 100% Downstream Pipe Size (mm)	Q2 1% (l/s)	Q2 10% (l/s)	Q2 30% (l/s)	Q2 100% (l/s)		
RM8	2.081	1.70	1.916	128.1	0.17	121.234	19.026	325	6.9	10.8	21.1	36.06
RM9	1.990	1.70	1.727	106.8	0.19	106.800	19.026	480	6.3	11.6	16.2	36.06
RM10	0.931	1.70	1.061	60.1	0.09	60.303	7.024	300	2.9	5.4	7.6	14.0
RM11	1.500	1.70	1.121	70.1	0.13	69.643	19.026	480	4.8	8.8	12.5	29.1
RM12	0.660	1.70	0.900	56.1	0.12	56.388	19.026	325	2.1	3.9	5.5	12.0
RM13	0.860	1.70	1.060	58.0	0.10	57.922	19.026	325	2.8	5.1	7.1	13.0
RM14	0.960	1.70	1.160	60.0	0.11	60.700	19.026	480	2.9	6.2	7.3	13.0
RM15	1.700	1.70	1.340	78.8	0.16	79.267	19.026	480	5.7	10.5	14.7	27.0
RM16	4.700	1.70	3.860	240.0	0.20	240.000	19.026	480	11.8	20.1	28.8	53.8
RM17	3.347	1.70	2.510	154.7	0.17	154.700	11.026	480	10.7	18.6	27.5	50.0
RM18	2.007	1.70	1.908	120.7	0.18	120.700	11.026	480	6.4	11.8	16.5	30.0
RM19	0.487	1.70	0.343	20.4	0.05	20.400	11.026	480	1.5	2.7	3.6	7.1
RM20	2.044	1.70	1.935	124.4	0.19	124.400	11.026	480	6.5	11.6	16.2	30.0
RM21	3.106	1.70	2.336	131.8	0.18	131.800	19.026	325	9.9	18.2	26.6	47.0
RM22	2.000	1.70	1.890	120.0	0.19	120.000	19.026	480	7.0	12.9	18.1	33.0
RM23 (St-Broad Site)	1.085	1.70	1.271	68.5	0.14	70.664	0.00	0.00	5.4	9.9	13.9	25.0
RM24	1.000	1.00	1.000	0.00	0.00	0.00	0.00	0.00	0.0	0.0	0.0	0.0
Primary School	2.470	1.30	1.816	111.9	0.10	111.900	23.026	300	5.3	6.4	6.9	19.0
Secondary School Site	3.140	1.30	1.990	114.9	0.10	114.900	23.026	300	4.4	6.1	11.3	21.0
Commercial Site	1.141	1.70	1.177	69.7	0.10	69.700	19.026	480	4.3	7.9	10.4	18.0
Community School	1.271	1.70	1.100	67.1	0.10	67.000	19.026	480	4.7	6.6	12.1	22.0
Local Centre	0.092	1.70	0.707	43.2	0.05	43.200	19.026	480	2.3	6.0	8.4	15.0
Parking area	0.737	1.00	0.737	0.00	0.00	0.00	0.00	0.00	1.7	3.3	3.0	8.0
Total	38.282		28.981						114.7	216.5	296.8	538.8

Southern Catchment Totals

Engineered Catchment	38.282	—	28.981	Yes	—	—	—	—	114.7	216.5	296.8	538.8
Retained Open Space	0.00	—	0.000	—	—	—	—	—	0.0	0.0	0.0	0.0
Total	38.282		28.981						114.7	216.5	296.8	538.8

SOUTH CATCHMENT	
GROSS CATCHMENT AREA =	81.1 HA
ENGINEERED CATCHMENT =	38.3 HA
HARD STANDING =	27.0 HA
ALLOWABLE DISCHARGES FROM ENGINEERED CATCHMENT:	
RETURN PERIOD	FLOW (l/s)
2	115
10	212
30	296
100+30%	542

FOR TECHNICAL APPROVAL

WSP
WSP Group plc
11011903 19030006

COUNTRYWIDE PROPERTIES

SOUTH WEST DISTRICT
DISTRICT OFFICE

SURFACE WATER STRUCTURE AND CATCHMENT LAYOUT

DATE: 19/01/2012
BY: [Signature]
CHECKED BY: [Signature]
APPROVED BY: [Signature]

W:\Projects\11011903\19030006\19030006.dwg 19/01/2012 11:52:21

APPENDIX C
EXTRACTS FROM HYDROCK
GROUND INVESTIGATION REPORT



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Spratton, Northampton, NN6 8LD.

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E-mail: northampton@hydrock.com
www.hydrock.com

**Desk Study and Ground
Investigation at
KM7 & KM9,
Kingsmere, Bicester**

Final Report

Prepared by

G Jenkins

&

C Vincett

for

Bellway (Northern Home Counties) Ltd

Hydrock Ref: R/12460/001/Rev002

November 2012

Soakaways & Drainage	<p>Hydrock do not believe soakaways are suitable on K7 & K9 due to:</p> <ul style="list-style-type: none"> • the presence of clay soils; • the soil infiltration rate testing indicated highly variable infiltration rates; • there is a significant thickness of Made Ground, part organic and placed by others, across the site; and • the groundwater at the site is shallow in the Cornbrash Formation. <p>In addition, anecdotal evidence from the local farmer indicates standing water levels at approximately 0.50m bgl in some land drainage ditches, precludes the use of soakaways.</p>
---------------------------------	--

Hydrock Site Works	<p>The initial Hydrock Ground investigation undertaken in September 2012 comprised:</p> <ul style="list-style-type: none"> • 30 trial pits to a maximum depth of 3.90m bgl; • 2 dynamic percussive sampling/rotary open holed boreholes to 12.00m bgl; • 24 TRL Dynamic Continuous Probe tests; • 5 soakaway tests; • installation of monitoring wells; • monitoring of ground gas concentrations and groundwater levels; • chemical testing of soils; and • geotechnical testing of soils.
---------------------------	---

Soakaway Potential	
<p>The Kellaways Clay Member is not considered suitable for conventional soakaways.</p> <p>The results of soakaway trials undertaken in the weathered Cornbrash Formation indicate infiltration rates in the order of between 10^{-4} and 10^{-5}. In less weathered Cornbrash Formation strata similar results were achieved although in two of the trials it was not possible to calculate an infiltration rate due to the relatively slow information.</p> <p>Although the Cornbrash Formation is considered suitable for soakaway design the presence of shallow groundwater (as identified by the WSP 2007 report) may preclude such design. The Pell Frischmann report also indicates anecdotal evidence from the local farmer which indicates standing water levels at approximately 0.50m bgl in some land drainage ditches.</p>	

5.5.1 Infiltration Tests

The results of soakaway testing are summarised in Table 5.10. The results sheets are given in Appendix D.

Table 5.10: Infiltration Test Results

Stratum	Trial Pit No.	Depth	Infiltration Rate (m/s)		
			Test 1	Test 2	Test 3
Cornbrash Formation	SA1	2.40	1.62×10^{-5}	n/a	n/a
Cornbrash Formation	SA2	1.90	1.38×10^{-4}	n/a	n/a
Kellaways Clay Member/Cornbrash Formation	SA3	2.00	Non calculable	n/a	n/a
Kellaways Clay Member/Cornbrash Formation	SA4	2.20	Non calculable	n/a	n/a
Cornbrash Formation	SA5	2.60	Non calculable	n/a	n/a

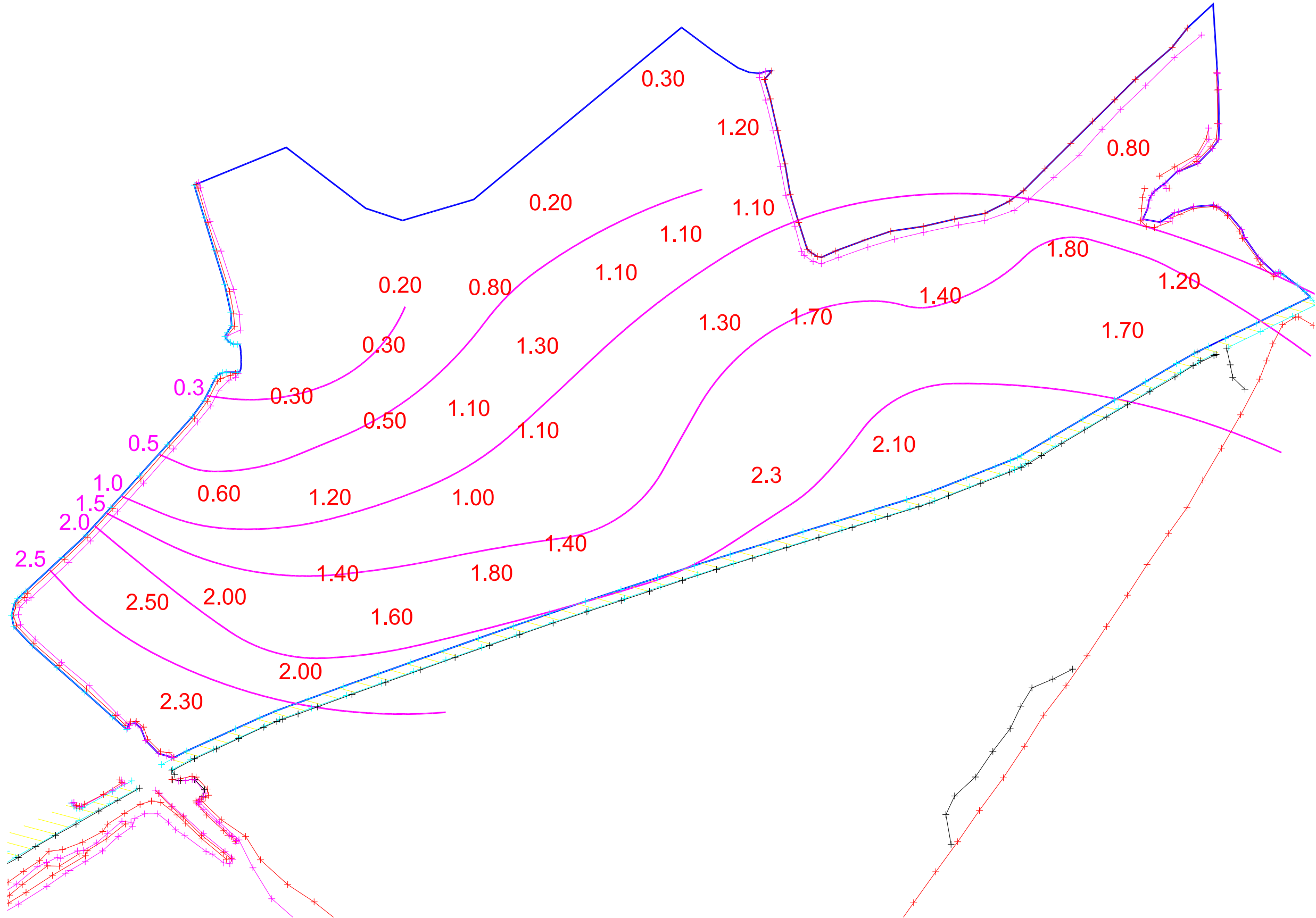
9.7 Soakaways and Drainage

Infiltration rate testing indicates that the Made Ground and Kellaways Clay Member are not suitable for soil infiltration.

With regards to the Cornbrash Formation, Hydrock do not believe soakaways are suitable on K7 & K9 as:

- the soil infiltration rate testing indicated highly variable infiltration rates;
- there is a significant thickness of Made Ground, part organic and placed by others, across the site;
- only 1 run could be undertaken, so test results are not in accordance with BRE 364; and
- the groundwater at the site is shallow in the Cornbrash Formation.

In addition, anecdotal evidence from the local farmer indicates standing water levels at approximately 0.50m bgl in some land drainage ditches, precludes the use of soakaways.



Notes:

1. This drawing has been produced using a topographical survey, drawing no. WSP0503 dated January 2006 by Nationwide Surveys.
2. DO NOT SCALE from this drawing.
3. This drawing is copyright.

Legend

- Site Boundary (Indicative Only)
- Interpolated Contours of Depth of Made Ground
- + 1.00 Made Ground Depths

Rev	Date	Description	Ckd	By

Hydrock

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Client

Bellway

Project

**KM7 & KM9 KINGSMERE,
BICESTER**

Title

**Depth of Made Ground
Contour Plan**

Drawing Status

INFORMATION

Job No.

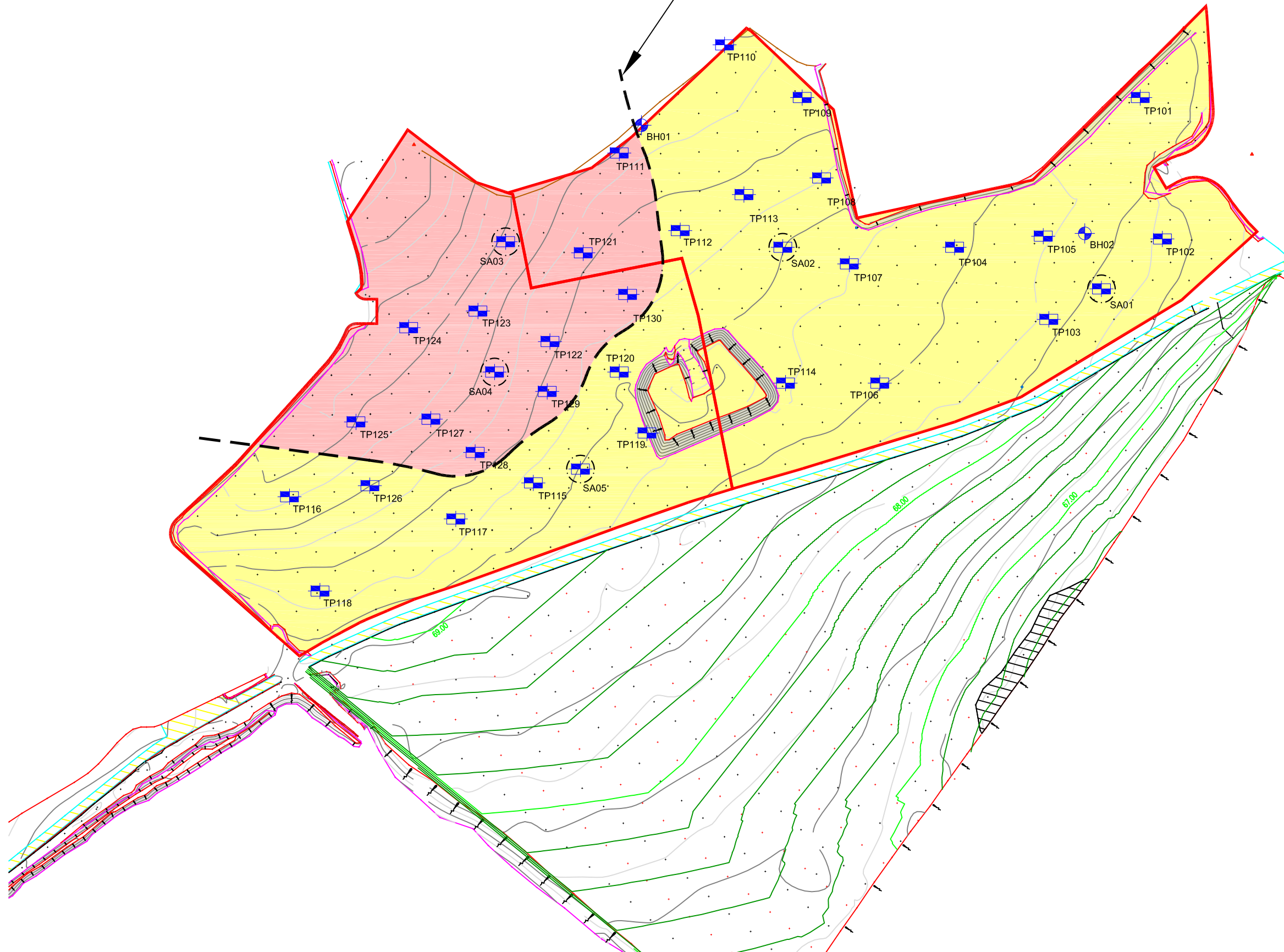
C/12460

Drawn	Checked	Scale at A3	Date	Issue Date
GJ	AB	1:1250	14/11/12	15/11/12

Drawing No.	Revision
12460/D005	-



Geology Boundary (Approximate Only)



Notes:

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Legend

- Parcel Boundary
- Trial Pit Location
TP101
- Borehole Location
BH01
- Soakaway Location
SA01
- Combrash Formation
- Kellaways Clay Member

Rev	Date	Description	Ckd	By

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Client

Bellway

Project

**KM7 & KM9, KINGSMERE,
BICESTER**

Title

Underlying Geology Plan

Drawing Status


INFORMATION

Job No. **C/12460**

Drawn	Checked	Scale at A3	Date	Issue Date
GJ	AB	1:1500	14/11/12	15/11/12

Drawing No.	Revision
12460/D006	-


APPENDIX D
MICRO DRAINAGE CALCULATIONS

RPS Design		Page 1
Noble House Capital Drive, Linfor... Milton Keynes MK14 6QP	JKK6647 - KINGSMEAD SITE KM7 10 YEAR SIMULATION	
Date 08/01/2013 File NETWORK 1.mdx	Designed by TD. Checked by	
Elstree Computing Ltd		Network W.12.6

Existing Network Details for NETWORK 1.SWS

* - Indicates pipe has been modified outside of System 1

PN	Length (m)	Fall (m)	Slope (1:X)	I.Area (ha)	T.E. (mins)	k (mm)	HYD SECT	DIA (mm)	
* 1.000	19.534	0.512	38.2	0.080	5.00	0.600	o	150	
* 1.001	24.578	0.529	46.5	0.128	0.00	0.600	o	225	
* 1.002	24.333	0.707	34.4	0.048	0.00	0.600	o	225	
* 2.000	17.775	0.173	102.7	0.032	5.00	0.600	o	150	
* 2.001	23.184	0.225	103.0	0.032	0.00	0.600	o	150	
* 1.003	30.868	0.581	53.1	0.029	0.00	0.600	o	300	
1.004	30.550	0.297	102.9	0.039	0.00	0.600	o	300	
1.005	45.636	0.239	190.9	0.058	0.00	0.600	o	300	
1.006	7.216	0.048	150.3	0.000	0.00	0.600	o	300	
* 3.000	15.990	0.155	103.2	0.025	5.00	0.600	o	150	
* 3.001	16.468	0.160	102.9	0.025	0.00	0.600	o	150	
3.002	43.038	0.418	103.0	0.152	0.00	0.600	o	225	
3.003	24.065	1.051	22.9	0.050	0.00	0.600	o	225	
* 4.000	23.771	0.567	41.9	0.116	5.00	0.600	o	150	
* 4.001	26.133	1.928	13.6	0.116	0.00	0.600	o	150	
* 4.002	30.782	0.150	205.2	0.078	0.00	0.600	o	375	
* 3.004	29.890	0.149	200.6	0.078	0.00	0.600	o	375	
PN	US/MH Name	US/CL (m)	US/IL (m)	US C.Depth (m)	DS/CL (m)	DS/IL (m)	DS C.Depth (m)	Ctrl	US/MH (mm)
* 1.000	1	70.850	69.500	1.200	70.338	68.988	1.200		1200
* 1.001	2	70.338	68.913	1.200	69.809	68.384	1.200		1200
* 1.002	3	69.809	68.384	1.200	69.312	67.677	1.410		1200
* 2.000	10	69.500	68.150	1.200	69.346	67.977	1.219		1200
* 2.001	11	69.346	67.977	1.219	69.312	67.752	1.410		1200
* 1.003	4	69.312	67.602	1.410	69.158	67.021	1.837		1200
1.004	5	69.158	67.021	1.837	69.048	66.724	2.024		1200
1.005	6	69.048	66.724	2.024	68.813	66.485	2.028		1200
1.006	7	68.813	66.485	2.028	68.802	66.437	2.065		1200
* 3.000	12	70.119	68.760	1.209	70.102	68.605	1.347		1200
* 3.001	13	70.102	68.605	1.347	70.354	68.445	1.759		1200
3.002	14	70.354	68.370	1.759	69.678	67.952	1.501		1200
3.003	15	69.678	67.952	1.501	69.393	66.901	2.267		1200
* 4.000	20	70.971	69.621	1.200	70.404	69.054	1.200		1200
* 4.001	21	70.404	69.054	1.200	69.827	67.126	2.551		1200
* 4.002	22	69.827	66.901	2.551	69.393	66.751	2.267		1500
* 3.004	16	69.393	66.751	2.267	69.329	66.602	2.352		1500

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Elstree Computing Ltd		Network W.12.6

Existing Network Details for NETWORK 1.SWS

PN	Length (m)	Fall (m)	Slope (1:X)	I.Area (ha)	T.E. (mins)	k (mm)	HYD SECT	DIA (mm)
* 3.005	26.813	0.134	200.1	0.029	0.00	0.600	o	375
* 3.006	11.732	0.059	198.8	0.020	0.00	0.600	o	375
* 3.007	9.449	0.047	201.0	0.020	0.00	0.600	o	375
* 1.007	4.798	0.047	102.1	0.000	0.00	0.600	o	150

PN	US/MH Name	US/CL (m)	US/IL (m)	US C.Depth (m)	DS/CL (m)	DS/IL (m)	DS C.Depth (m)	Ctrl	US/MH (mm)
* 3.005	17	69.329	66.602	2.352	69.007	66.468	2.164		1500
* 3.006	18	69.007	66.468	2.164	68.892	66.409	2.108		1500
* 3.007	19	68.892	66.409	2.108	68.802	66.362	2.065		1500
* 1.007	8	68.802	66.362	2.290	68.739	66.315	2.274	Hydro-Brake®	2700

Free Flowing Outfall Details for NETWORK 1.SWS

Outfall Pipe Number	Outfall Name	C. Level (m)	I. Level (m)	Min I. Level (m)	D,L (mm)	W (mm)
1.007	9	68.739	66.315	66.040	1800	0


Simulation Criteria for NETWORK 1.SWS

Volumetric Runoff Coeff	0.750	Additional Flow - % of Total Flow	0.000
Areal Reduction Factor	1.000	MADD Factor * 10m ³ /ha Storage	1.000
Hot Start (mins)	0	Inlet Coefficient	0.800
Hot Start Level (mm)	0	Flow per Person per Day (l/per/day)	0.000
Manhole Headloss Coeff (Global)	0.500	Run Time (mins)	60
Foul Sewage per hectare (l/s)	0.000	Output Interval (mins)	1

Number of Input Hydrographs	0	Number of Storage Structures	1
Number of Online Controls	1	Number of Time/Area Diagrams	0
Number of Offline Controls	0	Number of Real Time Controls	0

Synthetic Rainfall Details

Rainfall Model	FSR	Profile Type	Summer
Return Period (years)	2	Cv (Summer)	0.750
Region	England and Wales	Cv (Winter)	0.840
M5-60 (mm)	20.000	Storm Duration (mins)	30
Ratio R	0.400		


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Online Controls for NETWORK 1.SWS

Hydro-Brake® Manhole: 8, DS/PN: 1.007, Volume (m³): 15.2

Design Head (m) 1.800 Hydro-Brake® Type Md4 Invert Level (m) 66.362
Design Flow (l/s) 11.6 Diameter (mm) 105

Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)
0.100	3.1	1.200	9.4	3.000	14.9	7.000	22.7
0.200	7.0	1.400	10.2	3.500	16.1	7.500	23.5
0.300	6.4	1.600	10.9	4.000	17.2	8.000	24.3
0.400	5.9	1.800	11.5	4.500	18.2	8.500	25.0
0.500	6.2	2.000	12.2	5.000	19.2	9.000	25.8
0.600	6.7	2.200	12.7	5.500	20.1	9.500	26.5
0.800	7.7	2.400	13.3	6.000	21.0		
1.000	8.6	2.600	13.9	6.500	21.9		


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Storage Structures for NETWORK 1.SWS

Cellular Storage Manhole: 22, DS/PN: 4.002

Invert Level (m) 67.000 Safety Factor 2.0
 Infiltration Coefficient Base (m/hr) 0.00000 Porosity 0.95
 Infiltration Coefficient Side (m/hr) 0.00000

Depth (m)	Area (m ²)	Inf. Area (m ²)	Depth (m)	Area (m ²)	Inf. Area (m ²)
0.000	144.0	144.0	1.300	0.0	201.6
1.200	144.0	201.6			

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Summary of Critical Results by Maximum Level (Rank 1) for NETWORK 1.SWS

Margin for Flood Risk Warning (mm) 300.0
 Analysis Timestep 2.5 Second Increment (Extended)
 DTS Status ON
 DVD Status OFF
 Inertia Status OFF

Profile(s) Summer and Winter
 Duration(s) (mins) 15, 30, 60, 120, 240, 360, 480, 960, 1440
 Return Period(s) (years) 10
 Climate Change (%) 0


PN	Storm	Return Period	Climate Change	First X Surcharge	First Y Flood	First Z Overflow	O/F Act.	Lvl Exc.
1.000	15 Winter	10	0%					
1.001	15 Winter	10	0%					
1.002	15 Winter	10	0%					
2.000	15 Winter	10	0%					
2.001	15 Winter	10	0%					
1.003	15 Winter	10	0%					
1.004	15 Winter	10	0%					
1.005	15 Winter	10	0%					
1.006	120 Winter	10	0%					
3.000	15 Winter	10	0%					
3.001	15 Winter	10	0%					
3.002	15 Winter	10	0%					
3.003	120 Winter	10	0%	10/120 Winter				
4.000	15 Winter	10	0%					
4.001	15 Winter	10	0%					
4.002	120 Winter	10	0%					
3.004	120 Winter	10	0%					
3.005	120 Winter	10	0%					
3.006	120 Winter	10	0%					
3.007	120 Winter	10	0%					
1.007	120 Winter	10	0%					

PN	US/MH Name	Water		Flooded		Pipe		Status
		Level (m)	Surch'd Depth (m)	Volume (m³)	Flow / O'flow Cap. (l/s)	Flow (l/s)		
1.000	1	69.602	-0.048	0.000	0.79	0.0	21.5	OK
1.001	2	69.130	-0.008	0.000	0.82	0.0	58.1	OK
1.002	3	68.896	0.287	0.000	0.81	0.0	65.9	SURCHARGED
2.000	10	68.720	0.420	0.000	0.46	0.0	7.6	SURCHARGED
2.001	11	68.694	0.567	0.000	0.90	0.0	14.9	SURCHARGED
1.003	4	68.606	0.704	0.000	0.55	0.0	75.9	SURCHARGED
1.004	5	68.470	1.149	0.000	0.73	0.0	72.9	SURCHARGED
1.005	6	68.316	1.292	0.000	1.08	0.0	80.9	SURCHARGED
1.006	7	68.181	1.396	0.000	0.54	0.0	33.2	SURCHARGED
3.000	12	68.828	-0.082	0.000	0.41	0.0	6.6	OK
3.001	13	68.769	0.014	0.000	0.84	0.0	13.7	SURCHARGED
3.002	14	68.681	0.086	0.000	1.09	0.0	53.0	SURCHARGED

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Summary of Critical Results by Maximum Level (Rank 1) for NETWORK 1.SWS


PN	US/MH Name	Water		Flooded		Pipe		Status
		Level (m)	Surch'd Depth (m)	Volume (m ³)	Flow / O'flow Cap. (1/s)	Flow (1/s)		
3.003	15	68.196	0.019	0.000	0.21	0.0	21.4	SURCHARGED
4.000	20	70.199	0.428	0.000	1.00	0.0	26.1	SURCHARGED
4.001	21	69.641	0.437	0.000	1.09	0.0	50.7	SURCHARGED
4.002	22	68.190	0.914	0.000	0.10	0.0	11.8	SURCHARGED
3.004	16	68.189	1.063	0.000	0.09	0.0	11.4	SURCHARGED
3.005	17	68.186	1.209	0.000	0.09	0.0	11.3	SURCHARGED
3.006	18	68.183	1.340	0.000	0.10	0.0	11.1	SURCHARGED
3.007	19	68.180	1.396	0.000	0.11	0.0	10.9	SURCHARGED
1.007	8	68.178	1.666	0.000	0.85	0.0	11.6	SURCHARGED

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Existing Network Details for NETWORK 2.SWS

* - Indicates pipe has been modified outside of System 1

PN	Length (m)	Fall (m)	Slope (1:X)	I.Area (ha)	T.E. (mins)	k (mm)	HYD SECT	DIA (mm)	
* 1.000	18.276	0.195	93.7	0.060	5.00	0.600	o	150	
* 1.001	26.979	0.300	89.9	0.060	0.00	0.600	o	225	
* 1.002	17.538	0.177	99.1	0.060	0.00	0.600	o	225	
* 1.003	18.372	0.136	135.1	0.060	0.00	0.600	o	225	
* 1.004	28.481	0.067	425.1	0.060	0.00	0.600	o	300	
* 1.005	9.072	0.021	432.0	0.000	0.00	0.600	o	300	
* 2.000	27.386	0.283	96.8	0.059	5.00	0.600	o	150	
* 2.001	36.754	0.124	296.4	0.059	0.00	0.600	o	225	
* 2.002	9.849	0.720	13.7	0.000	0.00	0.600	o	225	
1.006	45.567	0.980	46.5	0.023	0.00	0.600	o	300	
* 3.000	11.788	0.065	181.4	0.027	5.00	0.600	o	225	
* 3.001	22.236	0.075	296.5	0.054	0.00	0.600	o	225	
3.002	31.033	0.073	426.5	0.161	0.00	0.600	o	300	
* 3.003	29.647	0.059	502.5	0.027	0.00	0.600	o	300	
* 1.007	17.617	0.035	503.3	0.023	0.00	0.600	o	450	
* 1.008	16.006	0.032	500.2	0.123	0.00	0.600	o	450	
* 1.009	17.944	0.036	498.4	0.213	0.00	0.600	o	450	
* 1.010	18.330	0.260	70.5	0.000	0.00	0.600	o	150	
PN	US/MH Name	US/CL (m)	US/IL (m)	US C.Depth (m)	DS/CL (m)	DS/IL (m)	DS C.Depth (m)	Ctrl	US/MH (mm)
* 1.000	25	69.859	68.509	1.200	69.664	68.314	1.200		1200
* 1.001	26	69.664	68.239	1.200	69.364	67.939	1.200		1200
* 1.002	27	69.364	67.939	1.200	69.187	67.762	1.200		1200
* 1.003	28	69.187	67.762	1.200	69.051	67.626	1.200		1200
* 1.004	29	69.051	67.551	1.200	69.142	67.484	1.358		1200
* 1.005	30	69.142	67.484	1.358	69.328	67.463	1.565		1200
* 2.000	37	70.090	68.740	1.200	69.807	68.457	1.200		1200
* 2.001	38	69.807	68.382	1.200	69.720	68.258	1.237		1200
* 2.002	39	69.720	68.258	1.237	69.328	67.538	1.565		1200
1.006	31	69.328	67.463	1.565	68.897	66.483	2.114		1200
* 3.000	40	67.829	66.830	0.774	68.003	66.765	1.013		1200
* 3.001	41	68.003	66.765	1.013	68.416	66.690	1.501		1200
3.002	42	68.416	66.615	1.501	68.550	66.542	1.708		1200
* 3.003	43	68.550	66.542	1.708	68.897	66.483	2.114		1200
* 1.007	32	68.897	66.333	2.114	68.690	66.298	1.942		1500
* 1.008	33	68.690	66.298	1.942	68.637	66.266	1.921		1500
* 1.009	34	68.637	66.266	1.921	68.533	66.230	1.853		1500
* 1.010	35	68.533	66.230	2.153	68.202	65.970	2.082	Hydro-Brake®	2700

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Free Flowing Outfall Details for NETWORK 2.SWS

Outfall Pipe Number	Outfall C. Level Name (m)	I. Level (m)	Min I. Level (m)	D,L (mm)	W (mm)
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1.010	36	68.202	65.970	65.620	1800 0
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Simulation Criteria for NETWORK 2.SWS

Volumetric Runoff Coeff	0.750	Additional Flow - % of Total Flow	0.000
Areal Reduction Factor	1.000	MADD Factor * 10m ³ /ha	Storage 1.000
Hot Start (mins)	0	Inlet Coefficient	0.800
Hot Start Level (mm)	0	Flow per Person per Day (l/per/day)	0.000
Manhole Headloss Coeff (Global)	0.500	Run Time (mins)	60
Foul Sewage per hectare (l/s)	0.000	Output Interval (mins)	1

Number of Input Hydrographs	0	Number of Storage Structures	1
Number of Online Controls	1	Number of Time/Area Diagrams	0
Number of Offline Controls	0	Number of Real Time Controls	0

Synthetic Rainfall Details

Rainfall Model	FSR	Profile Type	Summer
Return Period (years)	2	Cv (Summer)	0.750
Region	England and Wales	Cv (Winter)	0.840
M5-60 (mm)	20.000	Storm Duration (mins)	30
Ratio R	0.400		


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Online Controls for NETWORK 2.SWS

Hydro-Brake® Manhole: 35, DS/PN: 1.010, Volume (m³): 15.7

Design Head (m) 1.800 Hydro-Brake® Type Md4 Invert Level (m) 66.230
Design Flow (l/s) 12.7 Diameter (mm) 110

Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)
0.100	3.2	1.200	10.3	3.000	16.3	7.000	24.9
0.200	7.7	1.400	11.2	3.500	17.6	7.500	25.8
0.300	7.3	1.600	11.9	4.000	18.9	8.000	26.7
0.400	6.6	1.800	12.7	4.500	20.0	8.500	27.5
0.500	6.8	2.000	13.3	5.000	21.1	9.000	28.3
0.600	7.3	2.200	14.0	5.500	22.1	9.500	29.1
0.800	8.4	2.400	14.6	6.000	23.1		
1.000	9.4	2.600	15.2	6.500	24.0		


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Storage Structures for NETWORK 2.SWS

Cellular Storage Manhole: 33, DS/PN: 1.008

Invert Level (m) 66.400 Safety Factor 2.0
 Infiltration Coefficient Base (m/hr) 0.00000 Porosity 0.95
 Infiltration Coefficient Side (m/hr) 0.00000

Depth (m)	Area (m ²)	Inf. Area (m ²)	Depth (m)	Area (m ²)	Inf. Area (m ²)
0.000	144.0	144.0	1.300	0.0	201.6
1.200	144.0	201.6			

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Summary of Critical Results by Maximum Level (Rank 1) for NETWORK 2.SWS

Margin for Flood Risk Warning (mm) 300.0
 Analysis Timestep 2.5 Second Increment (Extended)
 DTS Status ON
 DVD Status ON
 Inertia Status OFF

Profile(s) Summer and Winter
 Duration(s) (mins) 15, 30, 60, 120, 240, 360, 480, 960, 1440
 Return Period(s) (years) 10
 Climate Change (%) 0

PN	Storm	Return Period	Climate Change	First X Surcharge	First Y Flood	First Z Overflow	O/F Act.	Lvl Exc.
1.000	15 Winter	10	0%					
1.001	15 Winter	10	0%	10/15 Winter				
1.002	15 Winter	10	0%	10/15 Summer				
1.003	15 Winter	10	0%	10/15 Summer				
1.004	15 Winter	10	0%	10/15 Summer				
1.005	15 Winter	10	0%	10/15 Summer				
2.000	15 Winter	10	0%					
2.001	15 Winter	10	0%	10/15 Summer				
2.002	15 Winter	10	0%					
1.006	15 Winter	10	0%					
3.000	120 Winter	10	0%	10/15 Summer				
3.001	120 Winter	10	0%	10/15 Summer				
3.002	120 Winter	10	0%	10/15 Summer				
3.003	120 Winter	10	0%	10/15 Summer				
1.007	120 Winter	10	0%	10/15 Summer				
1.008	120 Winter	10	0%	10/15 Summer				
1.009	120 Winter	10	0%	10/15 Summer				
1.010	120 Winter	10	0%	10/15 Summer				

PN	US/MH Name	Water		Flooded		Pipe		Status
		Level (m)	Surch'd Depth (m)	Volume (m³)	Flow / Cap.	O'flow (l/s)	Pipe Flow (l/s)	
1.000	25	68.629	-0.030	0.000	0.93	0.0	15.9	OK
1.001	26	68.483	0.019	0.000	0.60	0.0	30.3	SURCHARGED
1.002	27	68.387	0.223	0.000	0.91	0.0	42.7	SURCHARGED
1.003	28	68.240	0.253	0.000	1.39	0.0	55.7	SURCHARGED
1.004	29	67.969	0.118	0.000	1.42	0.0	68.7	SURCHARGED
1.005	30	67.825	0.041	0.000	2.03	0.0	69.0	SURCHARGED
2.000	37	68.863	-0.027	0.000	0.91	0.0	15.6	OK
2.001	38	68.630	0.023	0.000	1.07	0.0	30.5	SURCHARGED
2.002	39	68.336	-0.147	0.000	0.26	0.0	30.2	OK
1.006	31	67.646	-0.117	0.000	0.68	0.0	103.7	OK
3.000	40	67.594	0.539	0.000	0.06	0.0	1.9	FLOOD RISK
3.001	41	67.593	0.603	0.000	0.20	0.0	5.5	SURCHARGED
3.002	42	67.591	0.676	0.000	0.35	0.0	17.2	SURCHARGED
3.003	43	67.588	0.746	0.000	0.42	0.0	18.7	SURCHARGED
1.007	32	67.586	0.803	0.000	0.59	0.0	56.3	SURCHARGED

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Summary of Critical Results by Maximum Level (Rank 1) for NETWORK 2.SWS

PN	US/MH Name	Water		Flooded		Pipe		Status
		Level (m)	Surch'd Depth (m)	Volume (m ³)	Flow / O'flow Cap. (1/s)	Flow (1/s)		
1.008	33	67.583	0.835	0.000	0.44	0.0	38.9	SURCHARGED
1.009	34	67.597	0.881	0.000	0.33	0.0	32.2	SURCHARGED
1.010	35	67.599	1.219	0.000	0.55	0.0	11.0	SURCHARGED